

AN UPDATE ON THE 2004 PRE-FEASIBILITY STUDY FOR A FIXED LINK BETWEEN LABRADOR AND THE ISLAND OF NEWFOUNDLAND

MARCH 2018

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March 27, 2018

Mr. Ted Lomond
Deputy Minister of Tourism, Culture, Industry & Innovation
Government of Newfoundland and Labrador
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Dear Mr. Lomond:

An Update on the 2004 Pre-Feasibility Study for a Fixed Link Between Labrador and the Island of Newfoundland

On behalf of the Leslie Harris Centre of Regional Policy and Development, I am pleased to submit the final report of a study on a possible Fixed Link between Labrador and the Island of Newfoundland.

This study was intended as a follow-up to a study originally undertaken by the Harris Centre back in 2004. The purpose of this current new study is, first, to determine to what extent new geological research, innovations in tunneling technology, changes in labour costs, inflation or other factors may have an impact (positive or negative) on the original cost and time estimates. Secondly, the study aims to measure some possible impacts on the economy of the province overall and on those regions of the province that would be most affected by a change in traffic patterns.

The report is in four parts. The first consists of a 6-page executive summary of the entire study. The second consists of the detailed engineering and geotechnical study undertaken by Hatch, and runs to 397 pages, including eight appendices. The third part consists of the potential socio-economic impacts of a fixed link the Labrador Straits region and on Western Newfoundland undertaken by RAnLab, and runs to 27 pages. The final section consists of the terms of reference for the study, which runs to 20 pages.

Thank you for entrusting this project to the Harris Centre.

Sincerely,

A handwritten signature in blue ink that reads "Michael Clair".

Michael Clair, MBA
Associate Director (Public Policy)

An Update on the 2004 Pre-Feasibility Study for a Fixed Link Between Labrador and the Island of Newfoundland (2018)

Executive Summary

A Pre-Feasibility Study undertaken in 2004 by Hatch Mott MacDonald examined five different options for a fixed transportation link between Labrador and the Island of Newfoundland:

- An 18-km long suspension bridge.
- A combination of two 2-km long suspension bridges linked by three causeways.
- An immersed tube tunnel, consisting of prefabricated modules that are laid in a trench dug into the seabed.
- A tunnel excavated using drill and blast techniques.
- A tunnel excavated by a Tunnel Boring Machine (TBM).

The first three options were dismissed due to high construction costs and/or construction and operating risks. For the latter two options (the tunnels excavated in rock), the study investigated a road scenario (where vehicles are driven across individually) and a train option (where vehicles cross aboard a shuttle train). Both options included a single tunnel with a single-lane and a single-track respectively, and alternating traffic in each direction. A summary of the 2004 findings is provided in the following two tables. The financial information is provided in 2004 dollars.

Comparison of Fixed Link Road Options (2004 Pre-Feasibility Study)

| Option | Construction Cost (\$M-2004) | Annual Operating Cost (\$M-2004) | Risk Level | Project Duration (years) |
|--------------------------------|------------------------------|----------------------------------|------------|--------------------------|
| TBM Bored Tunnel ¹⁾ | 1,559 | 6.8 | Moderate | 12.2 |
| Drill & Blast Tunnel | 1,800 | 6.8 | High | 17.8 |
| Immersed Tube Tunnel | 4,810 | 6.8 | High | 14.7 |
| Bridge | 4,227 | 16.9 | Extreme | 15 |
| Causeway / Bridge | 10,123 | 4.3 | High | 18 |

1) TBM Bored Road Tunnel with 11 m inner diameter and 20 km length

Comparison of Fixed Link Rail Options (2004 Pre-Feasibility Study)

| Option | Construction Cost (\$M-2004) | Annual Operating Cost (\$M-2004) | Risk Level | Project Duration (years) |
|--------------------------------|------------------------------|----------------------------------|------------|--------------------------|
| TBM Bored Tunnel ¹⁾ | 1,144 | 7.64 | Moderate | 12.5 |
| Drill & Blast Tunnel | 2,272 | 7.64 | High | 23.8 |
| Immersed Tube Tunnel | 3,814 | 7.64 | High | 15 |

1) TBM Bored Rail Tunnel with 7.5 m inner diameter and 26.3 km length

The 2004 study concluded that the most viable option was an underground tunnel excavated by a Tunnel Boring Machine, with vehicles being transported on an electric shuttle train. The total development cost, including interest during construction and escalation, was estimated to be \$1.144 billion in 2004 dollars. The project was estimated to take 12½ years to complete.

Hatch Mott MacDonald has now split into two separate firms, Hatch and Mott MacDonald, with the former having the expertise in tunneling technology in Canada. To build upon the existing expertise, Hatch was retained as the engineering consultant for the current project to revise the 2004 Pre-Feasibility Study.

Hatch reviewed the geotechnical data that was available from various investigations carried out by others for Nalcor Energy between 2009 and 2012. Hatch also reviewed the HDD feasibility study and the conceptual design of a cable tunnel, prepared by Hatch Mott MacDonald that had been generated during the planning for the Strait of Belle Isle crossing of the hydroelectric cables from Muskrat Falls. As well, Hatch looked at advances in tunneling technology, and at tunnels constructed or under construction since 2004 in Turkey, Switzerland, Norway, Germany-Denmark, Korea and the United States.

Based on the review of assumptions and background information used in the Pre-Feasibility Study in 2004 and the new information available, a rail tunnel excavated by a single TBM is still considered the most technically and economically attractive option for a fixed transportation link between Labrador and Newfoundland.¹

A pressurized-face TBM is the best option to address the challenges of tunnelling through the different geological formations that will be encountered under the Strait of Belle Isle, comprising various rock types, varying from hard gneiss to weak shale. A pressurized-face TBM would also be capable of excavating through fault zones characterized by fractured rock and increased water inflows, as well as addressing high hydrostatic pressures that have to be expected at the estimated tunnel depth of 130 m below sea level.

The study was premised on the completion of Highway 138 on the Québec Lower North Shore and on all the Strait of Belle Isle traffic being diverted to the Fixed Link (i.e., abandonment of the Strait of Belle Isle ferry service). The analysis was further premised on the retention of the Gulf Ferry Service; it would be left to the market to determine which route to use to enter or leave Newfoundland (i.e., the Gulf Ferry Service or the Strait of Belle Isle Fixed Link). Currently, 60% of the traffic to the Island of Newfoundland is from Québec and points west; this percentage was therefore used to estimate the amount of traffic that might divert from the Gulf Ferry service to the Fixed Link in order to determine the capacity of the tunnel.

The study takes a long-term view of traffic volumes: starting from current levels, traffic volumes are projected 42 years into the future (including 12 years for planning, design and construction) with an estimated growth rate of 2.5% per year. Even at this rate of growth, this represents a relatively low projected traffic volume over the 42-year life of the project, for which a single tunnel would provide sufficient capacity.

Hatch updated the cost estimates for the construction phase and the ongoing annual operations. Two single-bore options were considered: a larger-diameter road tunnel with a single lane, and a smaller-diameter single-track rail tunnel for an electric shuttle train (like the Channel Tunnel between England and France, where cars and trucks cross aboard a train)². The train option would consist of three trains: one loading vehicles at one end, the second in transit through the tunnel, and a third unloading at the other end.

The estimated capital cost for the larger road tunnel is \$2.065 billion and for the smaller train tunnel \$1.675 billion (see the table next page). These estimates are based on a single TBM working 7 days a week during the construction phase.

¹ An option using two TBMs (one starting at each end, and meeting in the middle) is considered later in the report. While this would lead to a shorter construction period, it would also result in a higher construction cost.

² The larger tunnel diameter for the road option is required to accommodate an emergency passing lane and ventilation units in the tunnel to deal with vehicle exhaust fumes. An electric train would have no such requirements and could, therefore, operate in a smaller-diameter tunnel.

Comparison of Single-Bore Fixed Link Road and Rail Options (2004 vs 2017)

| Option | Tunnel Dimensions | Construction Costs ¹ | | Annual Operating Cost | | Project Duration (years) | |
|-------------|-------------------------|---------------------------------|----------|-----------------------|----------|--------------------------|------|
| | | \$M-2004 | \$M-2017 | \$M-2004 | \$M-2017 | 2004 | 2017 |
| Road option | 11 m Ø 21 km Length | 1,559 | 2,065 | 6.8 | 7.7 | 12.2 | 14.0 |
| Rail option | 7.6 m Ø 30 km Length | 1,144 | 1,675 | 7.6 | 8.7 | 12.5 | 15.5 |

¹ A more detailed assessment of the topography at the Labrador portal would likely enable a reduction in the rail tunnel length and an associated reduction in cost.

The 2017 capital costs are at a Class 4 level, meaning that the accuracy range for the estimates can vary from a low of -15% to -30%, to a high of +20% to +50%. For this study, a contingency of 40% has been added to the construction cost estimate.

A single-bore tunnel option means that traffic would only go in one direction at a time. Vehicles at either end would wait their turn to cross. For the road option, a travel speed of 80 km/h was assumed, resulting in a crossing time of approximately 20 minutes. It is estimated that the longest wait on shore would be about 60 minutes. The train was assumed to travel at 100 km/h and also take about 20 minutes to cross (the rail option requires a longer tunnel to account for a reduced slope at the exit ramps). The longest wait for the train option would be 60 minutes which would comprise 40 min for wait and loading of vehicles at one tunnel end and up to 20 minutes for extended unloading of vehicles at the opposite end.

In order to provide a more complete evaluation of the economic and financial options, the report looks at construction using 1 TBM and one using 2 TBMs.

The operating costs for the rail and road options over their operating periods are as follows:

Comparison of Operating Costs (\$ millions)

| | 1 TBM | 2 TBMs |
|-------------------------------------|-------|--------|
| <i>Number of years of operation</i> | 30 | 33 |
| Operating costs – rail option | 261 | 287 |
| Operating costs – road option | 229 | 252 |
| Difference | 32 | 35 |

The 2017 construction schedules for both the road and the rail options are longer than the 2004 schedules. The increased project duration is the result of the increased lengths of the tunnel exit ramps, which in turn are caused by the assumed greater depth below the sea bed of the tunnel, resulting in longer excavation times. The greater depth was assumed based on the available geotechnical information on ground conditions underneath the Strait and the associated risks during the tunnel excavation. As in 2004, the 2017 schedules include 3 years of planning (additional studies, field investigations, environmental assessment, etc.) and 2 years of detailed design, for a total of 5 years prior to start of construction.

Assuming that:

- 60% of the Gulf Ferry traffic decides to reroute to the Fixed Link;

- The Strait of Belle Isle Ferry service is abandoned and the annual subsidy reinvested into the Fixed Link; and
- The toll charged is equivalent to what is currently charged by the Strait of Belle Isle Ferry service;

the economic indicators are as shown in the table:

Economic Indicators

| | Rail Option | | Road Option | |
|---|-------------|--------|-------------|--------|
| | 1 TBM | 2 TBMs | 1 TBM | 2 TBMs |
| Net Present Value (NPV) of Cash Flows (\$ millions) | -593 | -674 | -829 | -928 |
| Internal Rate of Return (IRR) (%) | -0.16 | -0.34 | -0.79 | -0.56 |
| Benefit/Cost Ratio (BCR) | 1.03 | 1.08 | 0.84 | 0.88 |

The Net Present Value for each option is negative (ranging from -\$593 million to -\$928 million³). All IRRs are negative, rendering this indicator not useful for this economic evaluation. The Benefit Cost Ratio (the ratio of summed benefits to summed costs) is slightly above 1 for the rail option and slightly below 1 for the road option, which are both reasonable levels for public infrastructure. The NPV and BCR suggest that project justification, if pursued, would likely have to depend on public benefits that are external to this study.

Proceeding with the project will therefore require access to public funding. Various financing scenarios involving grants or contributions were considered to identify financial conditions under which the project could proceed. (To create a more robust projection, the table below assumes two scenarios: one where 60% of the Gulf Ferry service is rerouted to the Fixed Link and another where only 40% is redirected.) The main finding is that a total contribution in the order of \$1.37 billion to \$2.10 billion would be required for the project to provide a return of 8%. This is still below the private finance threshold considered currently applicable, except perhaps for long-term debt; however, it illustrates the level of public support that is required for advancing the project.

Project Costs in Current \$, IRRs and Suggested Grants Required

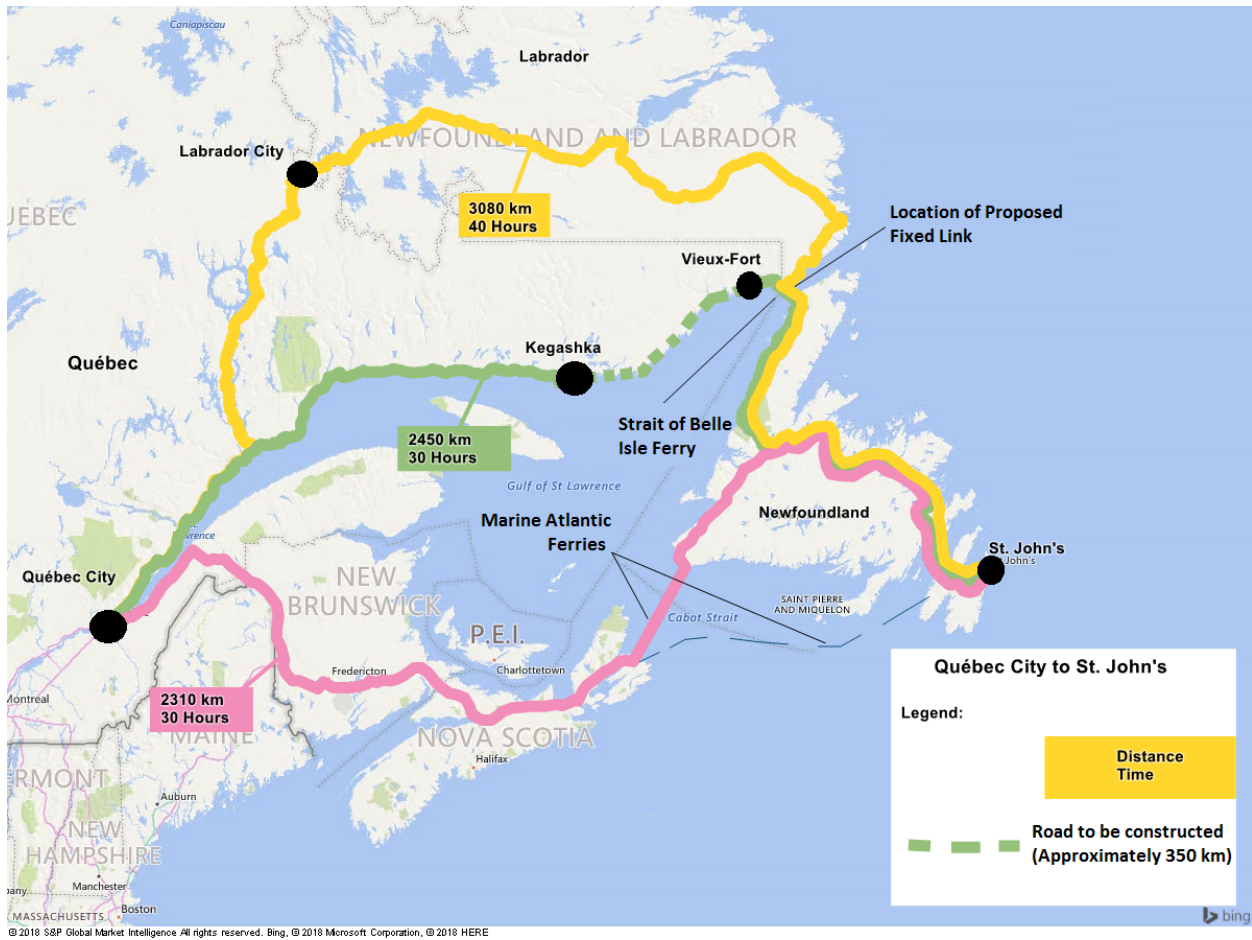
| | Rail Option | | Road Option | |
|--|-------------|--------|-------------|---------|
| | 1 TBM | 2 TBMs | 1 TBM | 2 TBMs |
| Total cost (\$Current) (millions) | 2,188 | 2,213 | 2,632 | 2,657 |
| Interest during construction (millions) | 578 | 398 | 692 | 451 |
| Total to finance (millions) | 2,767 | 2,611 | 3,324 | 3,107 |
| Assuming that 60% of the Gulf Ferry traffic is diverted to the Fixed Link | | | | |
| Internal rate of return (IRR) (%) | +1.97 | +2.20 | +1.04 | +1.28 |
| Grant required for an 8% IRR (millions) | 1,370 | 1,430 | 1,820 | 1,880 |
| Assuming that 40% of the Gulf Ferry traffic is diverted to the Fixed Link | | | | |
| Internal rate of return (IRR) (%) | +0.41 | +0.62 | +0.44 | +0.19 |
| Grant required for an 8% IRR (millions) | 1,595 | 1,655 | 2,041 | 2,104 |
| | Lowest | | | Highest |

Proceeding with this project as a Public-Private partnership (P3) appears feasible. It seems worthwhile to evaluate the potential of a Design-Build-Finance-Operate-Maintain (DBFOM) approach on the condition that the Federal and Provincial governments are willing to pay for the majority of the initial capital cost and they allow a private operator to charge tolls. The tolls would pay for the operating costs and for part of the capital costs.

³ The Net Present Value is based on a discount rate of 10% (a real social discount rate of 7.5% plus 2.5% annual inflation), the rate recommended by the Federal Treasury Board.

The socio-economic portion of the study was undertaken by the Harris Centre’s Regional Analytics Lab (RANLab), that uses advanced econometric models to estimate the impact of changes on the province’s economy.

As with the Hatch study, the RANLab study was premised on the fact that 60% of the traffic to the Island of Newfoundland is from Quebec and points west, and that the entirety of this traffic would reroute to the Fixed Link. The driving distance from Montreal to Deer Lake is longer by only about 120 km if travelling via the Fixed Link than via the Gulf Ferry service. When factors associated with “long-haul trips” (rest periods, time waiting on the ferry wharf and sailing time) are accounted for, the travel time for the Fixed Link route is on average 4.9 hours less than the road travel and Gulf Ferry crossing combined. (The map below is from the 2004 Hatch report and is included to provide a visual reference to the reader.)



The RANLab study looks at two potential impacts on the province. The first asks whether rerouting traffic to the Fixed Link would affect the cost of imports and/or the competitiveness of exports. It does this by taking commercial trucking as a proxy for the economic impact on the province as a whole. The model looks at four possible options: whether a truck travels to the Gulf ferry or to the Fixed Link (including the impact of mandatory rest stops), and whether the fee structure used is similar to the current Gulf ferry service or that of the Confederation Bridge to Prince Edward Island. (At 13 km, the Confederation Bridge is closer in scale to the proposed Strait of Belle Isle crossing, at approximately 30 km, vs. 180 km for the Gulf Ferry service.)

The Gulf ferry crossing costs nearly 10 times the toll on the Confederation Bridge, a reflection of the relative distances travelled. However, given that the ferry crossing constitutes a rest period for the driver, there are additional costs related to mandatory rests or to switching drivers for the option of driving to the Fixed Link via Highway 138.

Assuming that costs (or savings) are passed on to the consumer, there is a slight saving to residents of the Island if commercial traffic from Central Canada and points west uses the Fixed Link. The saving is higher if the trucker only has to pay the Confederation Bridge rate (a saving of \$13.1 million) rather than the Gulf ferry rate (a saving of \$3.2 million). Dividing the savings by the population of the Island, each Newfoundlander saves about \$27 per year if truckers pay the Confederation Bridge rate and \$7 if truckers pay the Gulf ferry rate.

The RAnLab study goes on to examine the intra-provincial changes that might be expected if 60% of the traffic is redirected from the Gulf ferry service to the Fixed Link. The study estimates that some economic activity will migrate from Southwestern Newfoundland to the Labrador Straits and the Northern Peninsula. (The Bay St. George and Bay of Islands regions would remain relatively unchanged. There would be no anticipated impacts east of Deer Lake.) It is estimated that Southwestern Newfoundland will lose between \$1.9 million and \$2.9 million worth of economic activity (direct and induced), resulting in a loss of between 40 and 83 full-time jobs. The lost jobs would be primarily in the travel and tourism sector: food and beverage, accommodations, and service stations; these would account for between 25 and 57 of the jobs. This economic activity and related jobs would migrate northwards.

The RAnLab study does not look at any socio-economic impacts during the construction phase, should the Fixed Link be constructed; this was outside the scope of the study. The findings related to intra-provincial transfers are only valid for the first year of the existence of the Fixed Link; future economic impacts are complicated by difficulties in estimating the induced impact of traffic in subsequent years.

The Leslie Harris Centre of Regional Policy and Development
Fixed Link between Labrador and Newfoundland
Update of 2004 Pre-Feasibility Study – FINAL REPORT



| Date | Rev. | Status | Prepared By | Checked By | Checked By | Approved By |
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1. Introduction

Hatch was retained by the Leslie Harris Centre of Regional Policy and Development (the Harris Centre), on behalf of the Government of Newfoundland and Labrador, to provide a revision of the Newfoundland and Labrador Fixed Link Pre-Feasibility Study which was prepared in 2004 by Hatch Mott MacDonald (HMM) [1]. The purpose of the current work is to update the previous study of fixed link infrastructure concepts with recent information and new data that was collected over the last years as well as with experience gained from the most recent similar fixed link projects worldwide.

The goal of the 2017 study is to determine to what extent new geotechnical and geophysical investigations and advancements in tunnelling technology as well as factors such as traffic forecasting, labour costs or inflation impact the previous assumptions and recommendations. As part of the current study, the Harris Centre is investigating the potential impact of a fixed link on the economy of the province.

1.1 Background

In 2004, HMM (now Hatch) carried out a Pre-Feasibility Study of options for a fixed transportation link between Labrador and Newfoundland across the Strait of Belle Isle with the objective to investigate economic and technical implications and the viability of a fixed link between the mainland of Labrador and the island of Newfoundland. The findings of the study were presented in HMM's final report 'Fixed Link between Labrador and Newfoundland Pre-Feasibility Study', dated November 2004 (refer to [1] and [2]).

Over the past 13 years, information pertaining to the surface and subsurface conditions along the proposed Strait of Belle Isle fixed link alignment was collected during a series of geotechnical and geophysical investigations, which were carried out for the Strait of Belle Isle High Voltage Direct Current (HVDC) cable crossing as part of Nalcor Energy's Lower Churchill Project (Nalcor Energy provided permission to Hatch to use this information in the current study). In addition, technological advances and experience was gained from other fixed link projects that were carried out worldwide, such as the Busan-Geoje Link in South Korea or the Gotthard Base Tunnel in Switzerland.

1.2 Understanding of the Work and Assumptions for this Study

The intent of this study is to update the background information and validate the assumptions applied in the 2004 Pre-Feasibility Study of Strait of Belle Isle fixed link alternatives. The main items included in the scope of work comprise the following (refer to Hatch's proposal dated March 06, 2017):

- Update of study background information based on new information available.
- Validation of previous assumptions made for the 2004 Pre-Feasibility Study.
- Update of experience gained from recent fixed link projects and technological advances worldwide.

- Update of comparison of fixed link alternatives and validation of previous recommendations for the preferred option.
- Update of cost estimate and schedule for the preferred option.
- A review of the financing considerations using public-private-partnerships (P3) is presented for a fixed link project in 2017.
- An update of the economic and financial analysis presented in the 2004 was added by addendum.

The updated fixed link recommendations are based on data review, revision of information and assumptions contained in the 2004 study, and discussions with the Harris Centre, thereby focussing on changes since 2004 that could impact the fixed link. Information or assessments contained in the 2004 Pre-Feasibility Study are recapped in the updated study only where deemed required for the understanding of the context or if considered to be otherwise relevant.

1.3 Study Approach

The following approach was used to carry out this updated study:

- A review of available information on worldwide fixed link projects that were carried out since 2004 or that are currently under construction or in planning or design phase was conducted. The review focussed on relevant projects and technological advances and innovations that can be related to the proposed Strait of Belle Isle fixed link. Project challenges, lessons learned and technological experience gained from these projects were evaluated and considered for the assessment of the Strait of Belle Isle fixed link options.
- A review of new background information was carried out, related to geotechnical, geological and environmental information, survey data, regulatory requirements as well as traffic volume data for the Gulf Ferries and the Strait of Belle Isle Ferry that was provided by the Harris Centre. Relevant changes to previous data used for the 2004 study were identified and the implications for the Strait of Belle Isle fixed link were defined.
- Previous assumptions included in the 2004 study regarding the utilization of the fixed link, the anticipated ridership and the design criteria were reviewed and updated where required based on the new information available and on discussions with the Harris Centre.
- The assessment of the Strait of Belle Isle fixed link options and the recommendations for the preferred option were reviewed and updated based on the updated information and assumptions.
- Cost estimate and schedule for construction were updated for the preferred option to reflect current market realities and to account for the recent technological advances.
- The economic and financial analysis included in the 2004 study was revised and updated.

- Financing considerations using P3 were reviewed and updated to reflect current trends in terms of delivery methods for major projects in North America.

2. Project Description and Location

The study area for the proposed fixed link is the Strait of Belle Isle, located between Newfoundland and Labrador (see **Figure 2-1**).

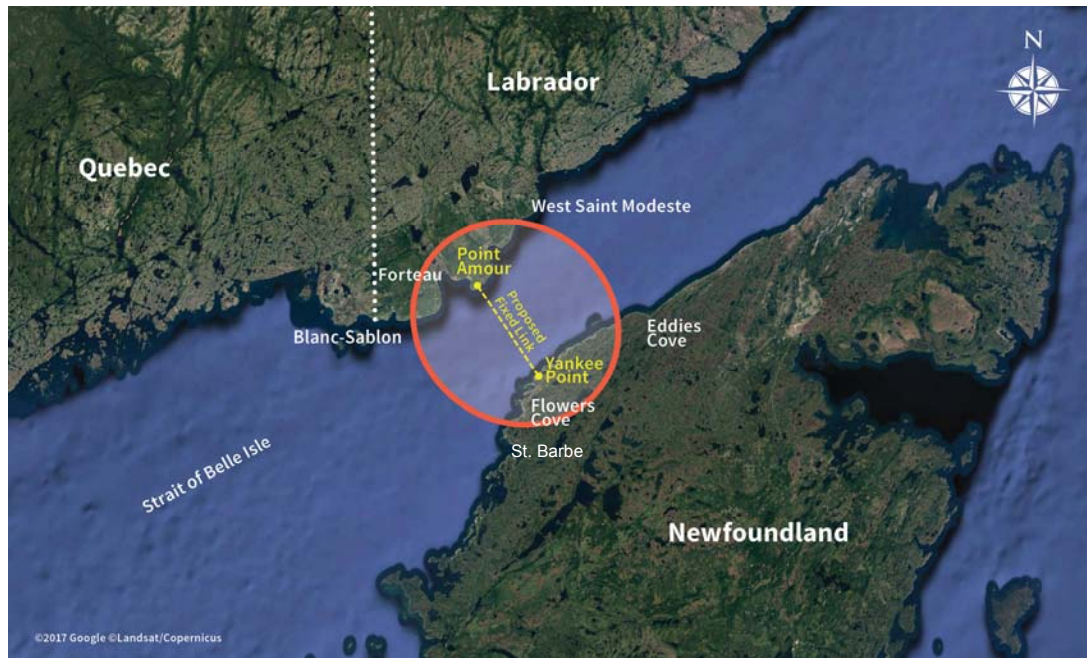


Figure 2-1: Proposed Project Location

Currently, the mainland of Labrador and the isle of Newfoundland are connected by ferry between Blanc-Sablon in Québec and St. Barbe in Newfoundland. Although the ferry connection is in operation throughout the year, it is subject to weather and ice conditions in the Strait of Belle Isle and interruptions during the winter months are common. The proposed fixed link project investigates a transportation link between Newfoundland and Labrador that would provide a permanent connection for vehicles between the isle and the mainland independently from weather and ice conditions.

The proposed fixed link is assumed to be located between Point Amour in Labrador (approximately 30 km east of Blanc-Sablon) and Yankee Point in Newfoundland (approximately 25 km north of St. Barbe), at the narrowest point of the Strait. The width across the Strait at this location is approximately 18 km. The water depth varies between shallower waters of approximately 25 m close to the Newfoundland shore and depths up to approximately 110 m towards the middle of the Strait and locally close to the Labrador shore.

The project location provides various challenges that impact the fixed link project. The great water depth, harsh environmental conditions, sea ice that covers the waters of the Strait for up to seven months each year and icebergs that float through the Strait, typically between the months of December to June, all have to be considered for any fixed link solution installed above surface, such as bridges or immersed tube tunnels. As a further constraint, marine traffic has to be maintained during construction and operation of surface fixed links. These factors will also impact pre-construction ground investigations that would need to be carried

out for potential subsurface fixed links such as bored tunnels. The complex geology underneath the Strait that includes fault zones and variable rock conditions will have to be considered for any subsurface construction.

Furthermore, Nalcor Energy's HVDC subsea cable that was installed in recent years on the sea bottom across the Strait of Belle Isle would have to be taken into account for the construction of all proposed subsurface fixed links at this location as well as for potential ground investigations carried out in the Strait.

3. Overview of 2004 Pre-Feasibility Study

Within the Pre-Feasibility Study carried out in 2004 [1], three basic fixed link concepts were investigated for the Strait of Belle Isle crossing. For each of these concepts, it was assumed that the crossing would be between Point Amour (Labrador) and Yankee Point (Newfoundland) which is the shortest distance between the two shores. The fixed link concepts included:

1. Bridge
2. Causeway with bridges
3. Tunnel, including:
 - i) Immersed Tube Tunnel (ITT)
 - ii) Bored tunnel
 - iii) Drill & blast tunnel

The assessment indicated that the projected traffic could be accommodated by a two lane above ground facility or with appropriate capacity by a single lane tunnel operated periodically in each direction for use by road vehicles or by roll-on/roll-off shuttle trains. All tunnel options included the integration of three high-voltage, direct current (HVDC) cables in the crossing.

3.1 Fixed Link Alternatives Considered in the 2004 Pre-Feasibility Study

Brief summaries of the fixed link concepts investigated during the 2004 study are summarized in the following sections. For more details about these options, the reader should refer to the 2004 Pre-Feasibility Study.

3.1.1 Bridge

A suspension bridge with two-kilometre spans crossing the approximately 18 km wide Strait of Belle Isle was considered the most economic bridge solution. The general concept is shown in **Figure 3-1**.

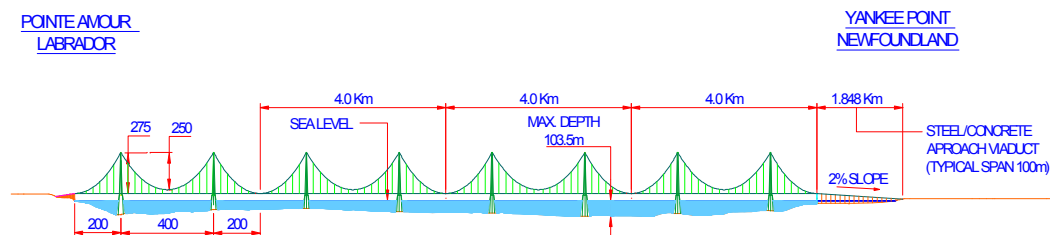


Figure 3-1: Bridge Concept (2004 Pre-Feasibility Study)

The wide spans would minimize the number of foundations to be installed in great water depth and the exposure of the structures to iceberg loadings. The bridge piers would have to be protected by berms to withstand impact from icebergs. A clearance of at least 50 m between the bridge deck and sea level was considered to accommodate marine traffic and moving icebergs. A bridge crossing of the Strait of Belle Isle was associated with very high

risks related to design, construction and operation resulting from the difficult environmental conditions at the site, including the location in an iceberg zone, the foundation installation in great water depths and the harsh weather conditions.

3.1.2 Causeway with Bridges

The causeway across the Strait of Belle Isle would consist of a rock berm placed on the sea floor and protected with armour stones on the outside to withstand the impact from icebergs. The general concept is shown in **Figure 3-2**.

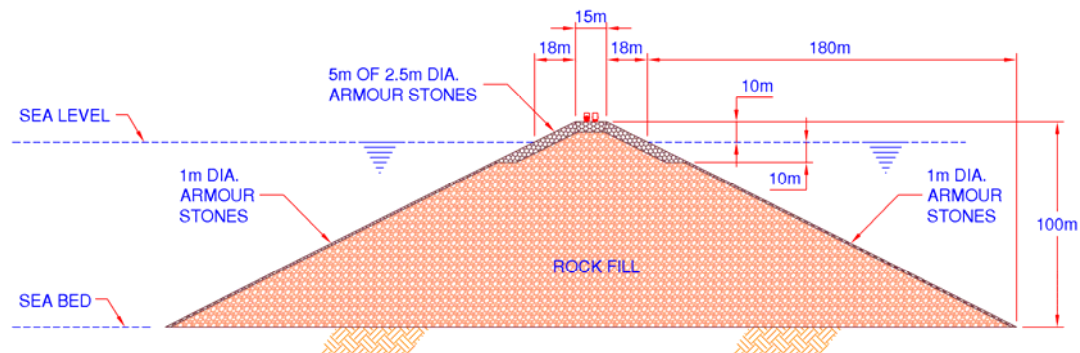


Figure 3-2: Causeway with Bridges Concept (2004 Pre-Feasibility Study)

To allow for marine traffic and moving icebergs, openings in the causeway would be required. For the 2004 study, it was assumed that two openings in the causeway would be needed, located at the position of the shipping lanes. Bridges would have to be integrated in the crossing to span the causeway openings. The bridge piers could be integrated into the causeway structure for protection against iceberg impact; however, foundations and anchoring of the bridges would still be challenging. The construction of a causeway across the Strait of Belle Isle was considered an ambitious undertaking and the risks associated with a causeway were considered high. A major environmental risk is the likely effect of a causeway on marine life and on the sea current regime in the area.

3.1.3 Immersed Tube Tunnel (ITT)

An ITT is a subsea tunnel that is installed on the sea bottom in a trench dredged into the sea floor. The ITT consists of pre-fabricated tunnel segments that are floated to their required location and sunk to their final position. The segments are connected and sealed under water, and backfilled to prevent uplift. The tunnel cross section suggested in 2004 is shown in **Figure 3-3**.

Since the ITT would be installed on the sea bed in varying water depths, there is a risk of iceberg impact on the tunnel where the depth is less than the submerged portion of the icebergs. The tunnel elements would have to be protected against iceberg impact and scour, either by burying the ITT elements below the depth of iceberg scour, or by having sufficient protection to absorb the energy of an iceberg. The risk associated with the construction of the ITT is considered high due to the depth of the water, the location of the site in an iceberg area and the harsh weather conditions.

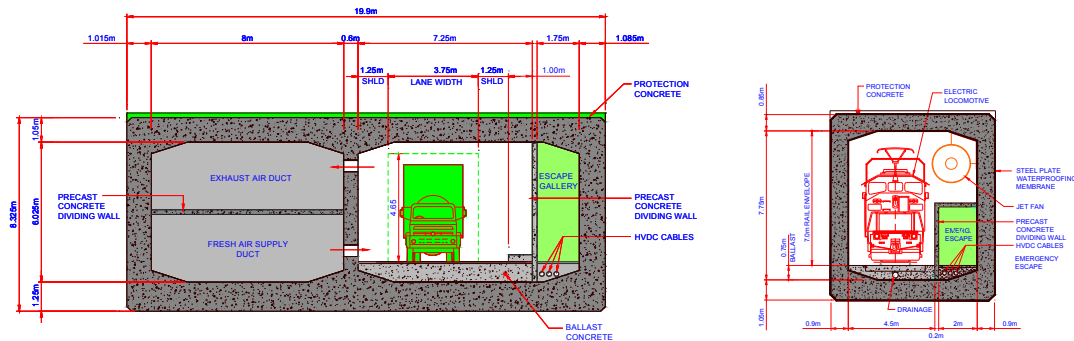


Figure 3-3: ITT Concept for Road and Rail (2004 Pre-Feasibility Study)

3.1.4 **Mined Tunnel**

Most of the risk factors associated with surface crossings of the Strait of Belle Isle (such as iceberg impact and construction in great water depth) would be eliminated with a tunnel that is constructed below the seabed. In the 2004 study, it was assumed that the tunnel would be excavated in the sedimentary rock layer below the seafloor. Water inflow into the tunnel would have to be considered, in particular in the many fault zones expected to be encountered along the alignment. A tunnel excavated in less permeable rock in greater depth of approximately 300 to 400 m below the sea bed (approximately 400 to 500 m below sea level) was considered not economically feasible. The tunnel excavation can be accomplished by either drill & blast excavation or by using a Tunnel Boring Machine (TBM).

3.1.4.1 **Bored Tunnel**

The general concept for a bored tunnel as suggested in the 2004 study is provided in **Figure 3-4**. Tunnel excavation was assumed to be carried out using a pressurized face TBM. This type of TBM is designed to apply pressure to the excavation face to support the ground and to counteract water pressure in fractures and fissures. It was assumed that the tunnel lining would be installed immediately as the tunnel is progressed to prevent water ingress. For the 2004 study, an alignment with approximately 10 m of minimum rock cover was assumed.

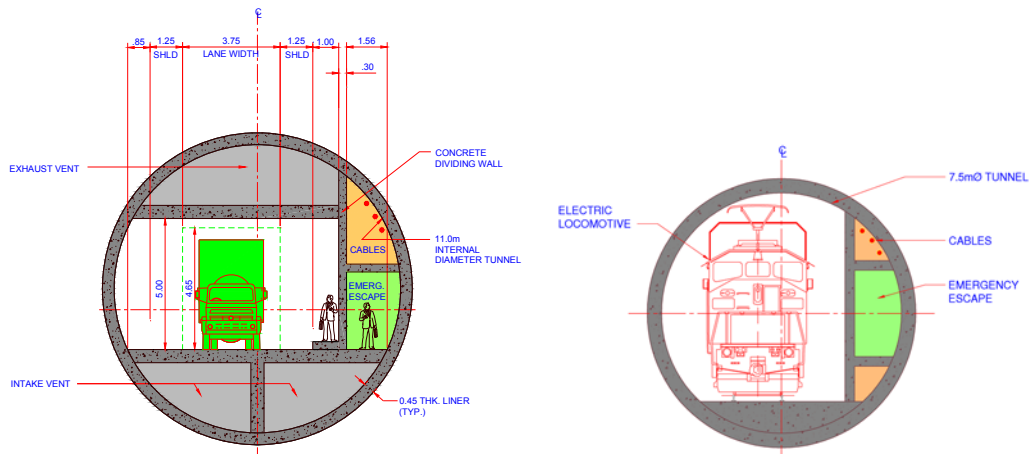


Figure 3-4: TBM Bored Tunnel Concept for Road and Rail (2004 Pre-Feasibility Study)

3.1.4.2 Drill & Blast Tunnel

The general concept for a tunnel excavated by drilling and blasting as suggested in the 2004 study is shown in **Figure 3-5**. A drill & blast excavation would require probe drilling and grouting ahead of the tunnel face to stabilize the rock in areas of poor rock quality and to seal water paths and reduce water ingress into the tunnel. Since grouting might not prevent water ingress entirely, a tunnel depth of approximately 60 m below the sea bed was assumed to separate the tunnel from the presumably more fractured rock in the upper rock layers closer to bedrock surface. Because of the greater depth, the drill & blast tunnels were assumed to be between 0.3 km (road tunnel) and 4.4 km (rail tunnel) longer than the respective TBM excavated tunnels.

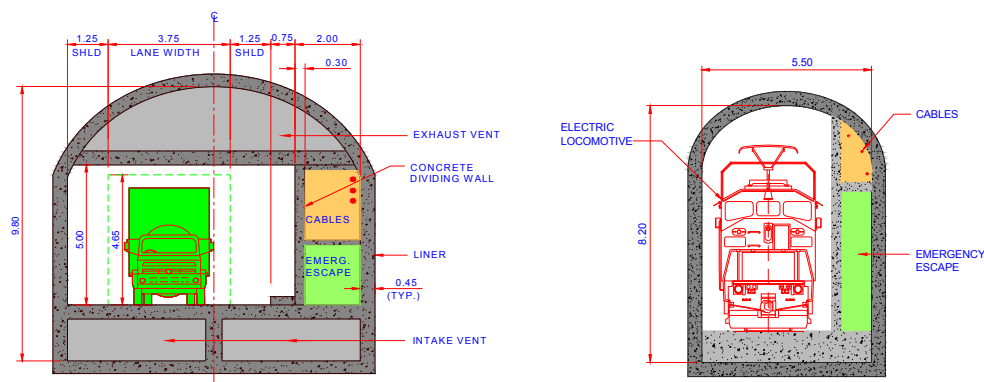


Figure 3-5: Drill & Blast Tunnel Concept for Road and Rail (2004 Pre-Feasibility Study)

3.2 Comparison of Alternatives (2004 Pre-Feasibility Study)

The comparison of the fixed link alternatives in the 2004 study was based on an assessment of the construction (short-term) risks and the operational (long-term) risks associated with each alternative.

3.2.1 Construction Risks

All surface fixed link options (bridge, causeway with bridges and ITT) would be exposed to construction risks related to the harsh weather and environmental conditions in the Strait of Belle Isle. Factors such as icebergs and sea ice conditions during the winter months, water depths up to 100 m, rough sea conditions and strong currents as well as the need to maintain heavy marine traffic in the navigation channel will impact the construction work and will likely prevent marine based construction during the winter period. These factors present a high level of risk that the construction would be interrupted and the schedule extended with consequent increases in cost.

For the immersed tube tunnel, the construction process is likely more sensitive to the environmental conditions due to some unprecedented aspects related to the Strait of Belle Isle project location, including depth of immersion, length of tunnel, excavation of rock at depth underwater, and substantial excavation volumes.

The mined tunnel fixed link options (TBM bored tunnel and drill & blast tunnel) are exposed to construction risks associated with subsea tunnelling at great depth. For the Strait of Belle Isle

project location, these include the occurrence of faults or fractured rock requiring immediate support or pre-excavation support systems as well as ground treatment to reduce groundwater inflows. For a bored tunnel, these geotechnical risks would be reduced with the use of an appropriate TBM such as a slurry or EPB machine together with a precast concrete tunnel lining. This type of machine provides the ability to pressurize and stabilize the excavation face to address poor rock conditions and seal off the workers from exposure to the ground conditions and associated water inflows. Drill & blast tunnelling would generally require a deeper tunnel alignment to increase the tunnel separation from the seabed and reduce water ingress. Water inflow would have to be addressed by grouting; however, the risk inherent with this method is considered high due to the high water pressures that have to be expected.

3.2.2 Operational Risks

The bridge option as well as the causeway with bridges option are associated with a considerable risk to the operation of the facility during the winter due to snow and icing, high winds and fog as well as iceberg interactions and ship impacts. The risk of having operational interruptions in the winter months was considered high.

The operational risks are similar for all tunnel options. The major risk of service interruption is associated with breakdowns or accidents within the tunnel. For the road tunnels, this risk is addressed with a passing lane and CCTV monitoring of the tunnel. For more substantial accidents and fires, an emergency egress passage has been provided and a fire suppressor system will likely be included to protect the structure and to reduce the closure time. Since the likelihood of fires in tunnels, especially in rail tunnels is low, the risk of operational interruptions in the tunnel options was considered low.

Specific for the ITT option is the risk of iceberg impact which could lead to an outage and to substantial protection work repairs or damage to the ITT elements themselves.

3.2.3 Summary

The following tables provide a comparative summary of the road (**Table 3-1**) and rail (**Table 3-2**) options investigated in the 2004 study, including estimated construction costs and project duration. The details regarding costs and schedule are not repeated in this report; for details refer to 2004 Pre-Feasibility Study [1] and [2].

Table 3-1: Comparison of Fixed Link Road Options (2004 Pre-Feasibility Study)

| Option | Construction Cost (\$M-2004) | Annual Operating Cost (\$M-2004) | Risk Level | Project Duration (years) |
|--------------------------------|------------------------------|----------------------------------|------------|--------------------------|
| TBM Bored Tunnel ¹⁾ | 1,559 | 6.8 | Moderate | 12.2 |
| Drill & Blast Tunnel | 1,800 | 6.8 | High | 17.8 |
| ITT | 4,810 | 6.8 | High | 14.7 |
| Bridge | 4,227 | 16.9 | Extreme | 15 |
| Causeway / Bridge | 10,123 | 4.3 | High | 18 |

1) TBM Bored Road Tunnel with 11 m inner diameter and 20 km length

Table 3-2: Comparison of Fixed Link Rail Options (2004 Pre-Feasibility Study)

| Option | Construction Cost (\$M-2004) | Annual Operating Cost (\$M-2004) | Risk Level | Project Duration (years) |
|--------------------------------|------------------------------|----------------------------------|------------|--------------------------|
| TBM Bored Tunnel ¹⁾ | 1,144 | 7.64 | Moderate | 12.5 |
| Drill & Blast Tunnel | 2,272 | 7.64 | High | 23.8 |
| ITT | 3,814 | 7.64 | High | 15 |

1) TBM Bored Rail Tunnel with 7.5 m inner diameter and 26.3 km length

3.3 Preferred Alternative (2004 Pre-Feasibility Study)

Based on the technical assessment and on the comparison of costs, risks and schedule, a TBM bored rail tunnel was considered the preferred alternative in the 2004 study.

4. Review of Recent Fixed Link Projects

As part of this study, a review of recent fixed link projects that were constructed since 2004 or that are currently under construction or in planning was carried out. Although various fixed link tunnel projects were realized over the past years, the focus of the review was on tunnel projects that are relevant to the proposed Strait of Belle Isle fixed link. The findings are summarized in the following sections. Experience from projects constructed up to 2004 are included in the previous Pre-Feasibility Study (refer to [1]).

4.1 Istanbul Strait Tunnel (Eurasia Tunnel)

The Istanbul Strait Tunnel project (Eurasia Tunnel) includes a 5.4 km long tunnel section that crosses underneath the Bosphorus Strait and connects the Asian and European sides of Istanbul. The tunnel section comprises an approximately 3.4 km long subsea tunnel excavated by TBM, a 1 km long drill & blast twin-tunnel on the Asian side as well as sections of cut-and-cover tunnel. The tunnel alignment and simplified geological profile are shown in **Figure 4-1** and **Figure 4-2**.



Figure 4-1: Eurasia Tunnel Location in Turkey

(<https://www.newcivilengineer.com/world-view/under-pressure-eurasia-tunnel/10001844.article>)

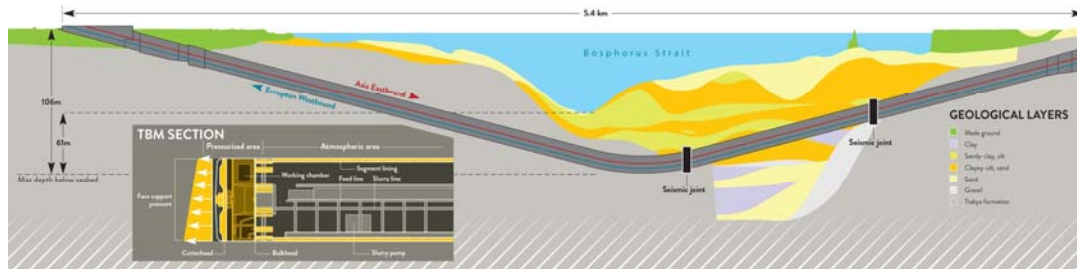


Figure 4-2: Simplified Geological Profile along Eurasia Tunnel Alignment
 (<https://www.newcivilengineer.com/world-view/under-pressure-eurasia-tunnel/10001844.article>)

The TBM tunnel, housing a stacked roadway for light vehicles only with two lanes in each direction (see **Figure 4-3**, left), was excavated with a 13.7 m diameter Mixed Shield TBM mostly in sedimentary rock; however, a significant section of the tunnel passes through soft alluvial deposits including sand and clay. Due to the depth of the tunnel alignment of up to 106 m below the water level, high hydraulic pressures of up to approximately 12 bar at the deepest point of the tunnel alignment were considered. The Mixed Shield TBM was specifically designed to address the various ground conditions including zones of heavily fractured rock as a result of tectonic events as well as mixed and soft ground conditions and to withstand the high hydraulic pressures. The 12.0 m diameter tunnel liner that was installed behind the machine consists of 600 mm thick precast concrete segments.



Figure 4-3: Eurasia Tunnel Cross Sections: TBM (left), Drill & Blast (right)
 (<https://www.newcivilengineer.com/world-view/under-pressure-eurasia-tunnel/10001844.article>); (Istanbul Strait Road Tube Crossing: Challenges, Risk and Mitigation Strategies, N. Munfah et al.)

To address poor ground conditions and high hydrostatic pressure, the drill & blast twin tunnels were excavated with a curvilinear cross section (see **Figure 4-3**, right) using the sequential excavation method including pre-support consisting of a continuous pipe umbrella support (forepoling). The initial support comprised lattice girders, rock bolts and shotcrete to address the poor ground conditions. 400 mm reinforced concrete over a waterproof membrane system was installed as the final liner in this tunnel section.

The project was executed under a Design-Build-Finance-Operate-Maintain (DBFOM) contract in a public-private-partnership delivery method. The tunnel opened in 2016 for an anticipated daily capacity of 100,000 light vehicles. The tunnel is open 24 hrs per day; the current toll for a one-way passage through the tunnel is approximately \$5.50 (16.60 TL) per car and \$8.30 (24.90 TL) per minibus (<https://www.avrasyatuneli.com/en/>). All other vehicles such as trucks, tractor trailers, motorcycles, busses, etc. are prohibited in the tunnel, A height and weight limit is in effect and the transport of hazardous materials is prohibited.

Challenges of the project included the large excavation diameter, the difficult geology that included full face rock sections, full face soft ground and mixed face transition zones, the high hydrostatic pressure and the susceptibility to seismic activity. The Mixed Shield TBM was capable of managing the ground challenges as well as the high water pressures that were encountered at the deepest point of the tunnel, which coincided with mixed face conditions. Cutterhead maintenance under the encountered high face pressure conditions required the machine to be equipped for saturation hyperbaric inspections. However, a non-exposure maintenance procedure was incorporated that included a remote monitoring system which allowed monitoring of the cutterhead conditions, disc cutter wear etc. using sensors and cameras installed at the cutterhead. Tool maintenance systems allowed the replacement of cutter tools under atmospheric conditions from inside the cutterhead spokes without exposure to the pressurized conditions.

4.2 Follo Tunnel

The Follo Tunnel is a rail tunnel project that is currently under construction in Norway. The 20 km long twin tunnels will provide a transportation link for high-speed passenger trains (with speeds up to 250 km/h) as well as for freight trains between Oslo Central Station and the new station in the city of Ski. The project location is shown in **Figure 4-4**.

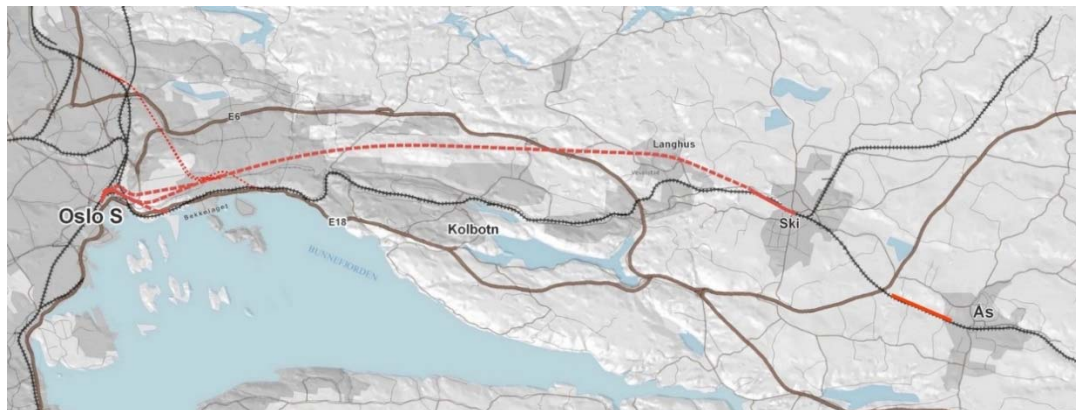


Figure 4-4: Follo Tunnel Location in Norway
(<http://www.northsouthraillink.org/follo-line-oslo/>)

The majority of the tunnel length (18.5 km) will be excavated by TBM, using four double-shield hard rock TBMs with a 10 m diameter that are designed for abrasive and strong rock. Starting from a launch area at mid-point of the alignment, two machines advance north towards Oslo and two machines south towards Ski, each excavating an approximately 9 km long section of the tunnels. Pre-cast concrete elements will be installed as final tunnel liner

behind the machines. Cross-passages between the two tunnels will be installed every 500 m along the alignment to provide escape routes. At the north end of the tunnel towards Oslo, where the new tunnels had to be excavated close to existing tunnels and sensitive installations, a 1.5 km long tunnel section was excavated by drill & split or drill & blast. The tunnel is Norway’s first long twin tube rail tunnel and one of the first tunnels in Norway excavated by TBM. A tunnel cross section is shown in **Figure 4-5**.

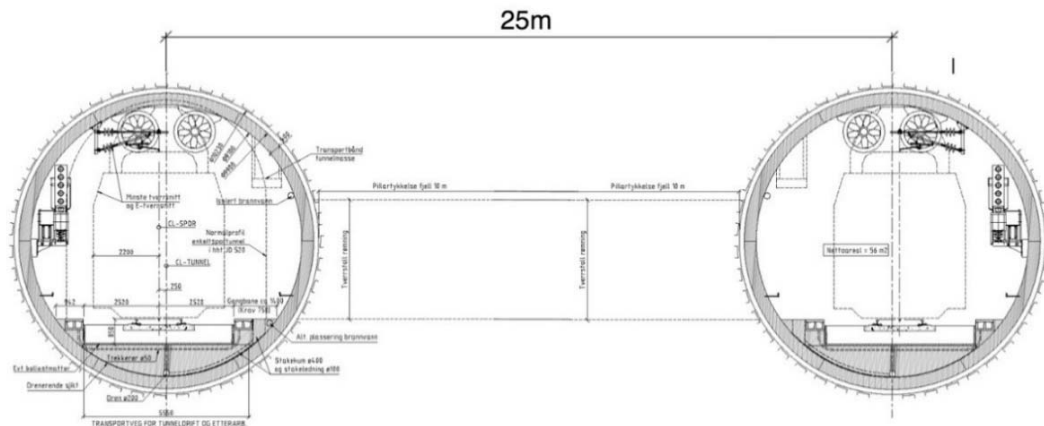


Figure 4-5: Cross Section of the Follo Tunnel Twin Tubes
 (<http://www.northsouthraillink.org/follo-line-oslo/>)

Available information on the timeline for planning and design indicates that the preliminary work for the tunnel commenced in 2008/2009 and final design started in 2012. Main construction work began in 2015 and TBM excavation started in September 2016. Beginning November 2017, 50% of the TBM tunnels were completed. It is expected that the TBMs will reach Oslo in the Summer of 2018. The scheduled construction time for the TBM tunnel excavation is 2 years with TBM advance rates anticipated to be between 12 m and 15 m per day. Project completion is scheduled for December 2021. As of early November 2017, all four machines were on schedule and no major difficulties were experienced so far.

4.3 Gotthard Base Tunnel

As part of the Alp Transit Project, the Gotthard Base Tunnel is a railway tunnel with two single track tubes that cross under the Alps in Switzerland and provide a high-speed rail link between southern and northern Europe. The tunnel alignment is shown in **Figure 4-6**. Both tunnel tubes have a length of approximately 57 km (total tunnelled length of 114 km) and were constructed at depths up to 2,450 m below ground surface. The tunnel opened in 2016.

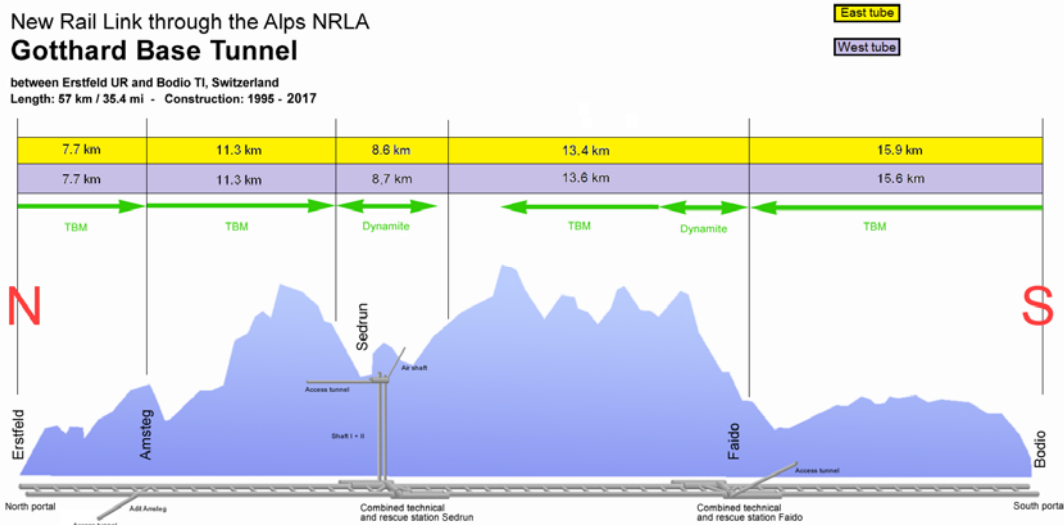


Figure 4-6: Profile along Gotthard Base Tunnel in Switzerland
 (https://en.wikipedia.org/wiki/Gotthard_Base_Tunnel)

The tunnel tubes were excavated mainly by four open face gripper TBMs with diameters between 8.8 m and 9.6 m (**Figure 4-7**). The average advance rates were approximately 10 to 14 m per work day with maximum performance up to 40 m per work day. The lengths of the individual sections excavated by TBM varied between 7 and 14 km.

Approximately 20% of the entire tunnel length as well as several access tunnels and cross passages were excavated by conventional drill & blast methods with excavation diameters between 8.8 m and 13.1 m and average advance rates of 1 m to 4.5 m per work day, depending on rock conditions. The maximum advance rate was 11.5 m per work day. Drill & blast excavation was chosen for tunnel sections that were expected to be in rather weak rock and that required a controlled approach of the expected unstable fault zones and prevention of uncontrolled water inflow.

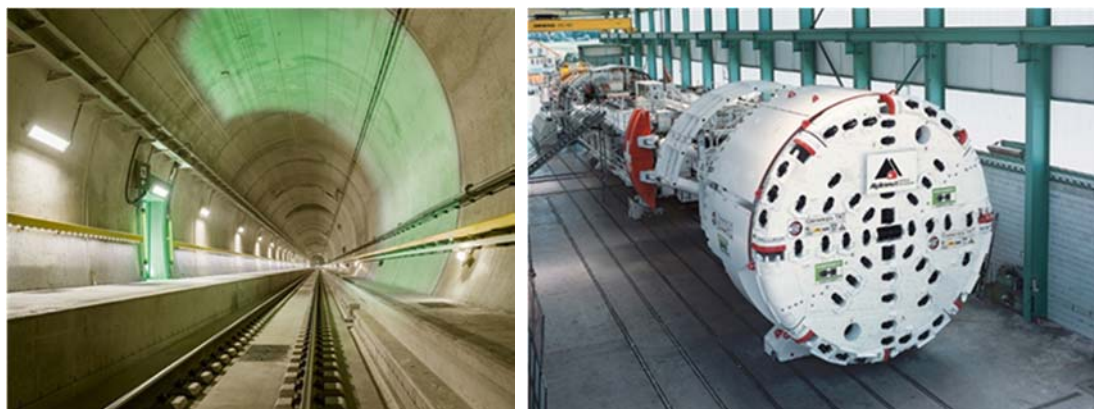


Figure 4-7: Gotthard Base Tunnel after Completion (left) and Gripper TBM (right)
 (<http://www.littlegatepublishing.com/tag/gotthard-base-tunnel/> and
<https://tunneltalk.com/Gotthard-TBM-safely-across-the-Piora-Mulda2.php>)

The rock support used to stabilize the excavated tunnel walls and crowns comprised typically rock bolts and shotcrete. In areas where large radial displacements were experienced, a yielding support including steel arches was used. In zones of loose, fractured rock, auxiliary measures including a pipe screen umbrella with extensive pre-excavation grouting were applied.

Lessons learned during TBM excavation were mostly related to difficult geological conditions. These included zones of soft ground that were too soft for the gripper TBM as well as areas of poor rock quality. An unexpected zone of unconsolidated rock resulted in the TBM being stuck due to the sudden inflow of loosened rock mixed with water into the TBM's cutterhead. To improve the stability of the rock ahead of the cutterhead, a cement/bentonite mixture was injected into the unconsolidated rock. In addition, a bypass gallery had to be advanced from the parallel tunnel tube to free the TBM cutterhead from in front.

Challenges at the intermediate station in Sedrun were caused by difficult geological conditions including an extensive fault zone consisting of steeply inclined sequences of soft and hard rock in which radial deformations up to 700 mm occurred. Large overbreak and a yielding tunnel lining that included steel arches were used to address the rock deformations.

Challenges resulted also from the high pressure due to the high rock overburden above the tunnel. To prevent extreme deformations of the tunnel, a new tunnel support concept of flexible steel arches was developed that allowed deformation of the ground around the over-excavated tunnel opening without damage to the final tunnel liner.

A further challenge was the management of the 28.2 million tons of excavated rock material that had to be processed for use as aggregate for concrete or for embankments, etc. Approximately 0.2 million tons were classified as hazardous waste.

4.4 Eiksund Tunnel

The Eiksund Tunnel in Norway is part of a fixed link project including three tunnels and a bridge between the Norwegian mainland and Hareidlandet Island (**Figure 4-8**). The 7.8 km long single tube road tunnel (**Figure 4-9**) has a width of 10 m and accommodates three traffic lanes, allowing for a crawler lane on each exit gradient to address slower traffic on the steep exit grades of up to 9.6%. The Eiksund Tunnel passes under the Vartdalsfjorden and is to date the world's deepest sub-sea rock tunnel with a depth of 287 m below sea level at the deepest point.

The tunnel was excavated in Precambrian gneiss with a minimum rock cover of approximately 50 m using traditional drill & blast technique and supported by rock bolts and shotcrete. The tunnel final lining system consist of reinforced patented fabric and steel arches. During construction, probe drilling from the tunnel face and pre-excavation grouting was used to control groundwater inflows and improve the ground conditions ahead of the excavation face.



Figure 4-8: Eiksund Tunnel Location in Norway
(<https://www.roadtraffic-technology.com/projects/eiksund/>)



Figure 4-9: Eiksund Tunnel after Completion
(<http://www.panoramio.com/user/80930>)

Opened in 2008, the projected usage was 1000 vehicles per day (vpd). However, due to ease of access actual traffic increased rapidly to 2200 vpd. The toll for a one-way passage was approximately \$13 (76 NOK) per car and \$40 (228 NOK) per truck. After six years in operation, the tunnel was fully financed and became toll free in 2014, resulting in a traffic increase to 2880 vpd (2014), half of which were trucks.

The Eiksund Tunnel is a typical example for tunnel constructions in Norway. The lessons learned from this project are that traditional drill & blast tunnels in deep undersea conditions are feasible and that significant water inflows can be addressed with appropriate probe drilling and pre-excitation grouting. Attention had to be paid to the issue of corrosion of the rock support and the tunnel liner as well as damage to electrical and other equipment installed in the tunnel due to salt water seepage into the tunnel. These problems would also need to be addressed in a drill & blast tunnel at the Strait of Belle Isle due to the expected high sea water inflow into the tunnel through the faults underneath the Strait. By choosing steep grades of up to 9.6%, the lengths of the Eiksund tunnel entrance/exit ramps were reduced.

4.5 Ceneri Base Tunnel

The Ceneri Base Tunnel was constructed as part of the Alp Transit Project in Switzerland (Figure 4-10). The tunnel consists of two single track tubes, each with a length of 15.4 km, that were excavated by drilling and blasting; excavation was completed between 2007 and early 2016. The rock overburden above the tunnels has a thickness between approximately 10 m and up to approximately 900 m. The tunnel tubes have cross-sections between 62 m² and 87 m² (approximately 8.7 m excavated width); a total bulked volume of 3.5 million m³ rock was excavated.

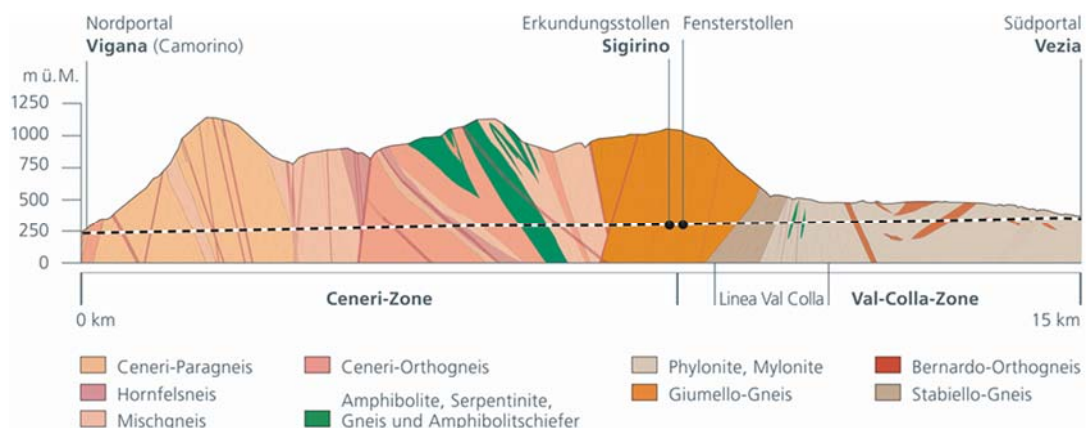


Figure 4-10: Geological Profile along Ceneri Base Tunnel in Switzerland
 (<https://www.alptransit-portal.ch/en/events/ereignis/construction-of-the-ceneri-tunnel/>)

The tunnel was excavated through varying geological conditions and fault zones. Rock support and excavation shape were adjusted depending on the conditions. In areas located in stable rock formations a flat tunnel invert with a few short radial rock bolts and a thin shotcrete lining were required. Sections with intermediate fault zones were advanced using curved inverts, steel ribs in the crown, rock bolts in the tunnel walls and thick shotcrete lining. In areas located in the most severe fault zones the tunnel was excavated with a circular invert

and supported with steel ribs in the crown and invert, with many long rock bolts and a thick shotcrete lining.

The lesson learned from the Ceneri Tunnel project is that drill & blast methods can be used for long tunnel excavations and these allow for flexibility regarding tunnel size, shape and support installation to address varying ground conditions and size requirements.

4.6 Busan-Geoje Link

The Busan-Geoje fixed link in South Korea connects the City of Busan on the south coast with the island of Geoje (**Figure 4-11**). The dual two-lane road link has a total length of 8.2 km and consists of a 3.2 km long ITT along with two cable-stayed bridges (total of 3.6 km) and short rock tunnels excavated on two islands along the alignment.



Figure 4-11: Busan-Geoje Fixed Link Location in South Korea
 (https://en.wikipedia.org/wiki/Busan%E2%80%93Geoje_Fixed_Link)

The immersed tunnel consists of 18 segments with an average length of 180 m. Crossing the strait at a depth of up to 48 m below mean sea level, the Busan-Geoje ITT was the world's deepest immersed vehicle tunnel when it opened in 2010, becoming the second deepest with the opening of the Marmaray rail tunnel (see section 4.7) in 2013.

Furthermore, two cable-stayed bridges are included in the Busan-Geoje fixed link (**Figure 4-12**). The main two pylon cable-stayed bridge has a 475 m central span and 230 m side spans; the pylons have a height of 156 m with 52 m of navigational clearance; the smaller three pylon cable-stayed bridge has two central spans of 230 m and 106 m side spans with pylons of 102 m height and 36 m clearance. Pre-cast caisson foundations were placed at water depths of 30 m.



Figure 4-12: Busan-Geoje Fixed Link Bridges
(<http://www.ipernity.com/doc/303473/41396014>)

Lessons learned from this project are that ITT constructions at greater water depths (>50 m) and in challenging environmental conditions are becoming more feasible. Project challenges included the extreme weather conditions along the strait such as typhoons and huge swell waves which can create fluctuating pressure around the immersed tunnel with hydrostatic pressure of 65 m. In addition, the region's high seismicity had to be addressed in the design of the tunnel element joints. To improve the soil conditions below the immersed tunnel, a mixture of ground replacement, sand compaction piles and cement deep mixing was used to stabilize the soil down to 65 m below sea level.

Innovations on the project included an External Positioning System (EPS) comprising two hydraulic sea-legs that can grip an immersed tunnel segment and 'walk' across the sea floor which allowed the operator to position the segment to an accuracy of a few millimetres. Steel bulkheads instead of concrete bulkheads were used to seal the tunnel segments which allowed for significant time savings during removal of the bulkheads.

4.7 Marmaray Tunnel

The Marmaray link is a rail transportation project in Turkey connecting Asia and Europe below the Bosphorus Strait (**Figure 4-13**).

The project includes 13.6 km of tunnel sections including a 1.4 km long ITT (see **Figure 4-14**). The two-tube rail tunnel was installed in water depths of up to 58 m below sea level, which to date makes the Marmaray ITT the world's deepest immersed tunnel. Project challenges included the high seismicity in the area.

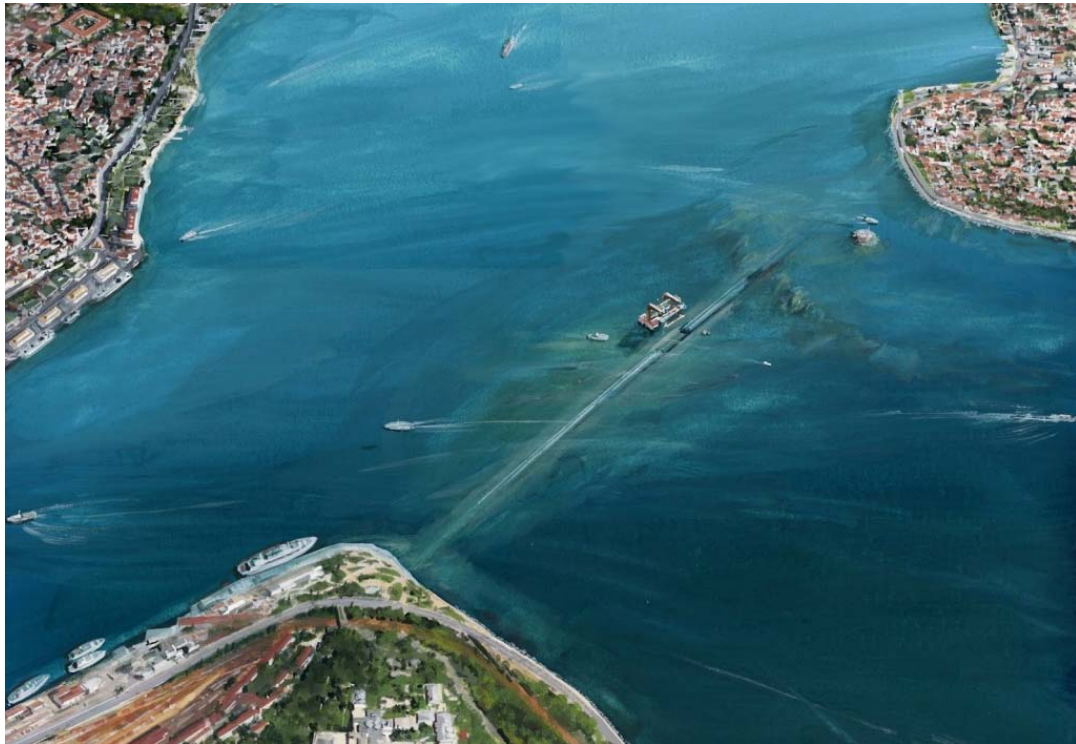


Figure 4-13: Marmaray ITT Location at the Bosphorus Strait in Turkey
(<http://howtoistanbul.com/en/opening-of-the-marmaray-project-on-the-republics-90th-anniversary/10398>)

As shown in **Figure 4-14**, the immersed tunnel consists of 11 prefabricated segments with lengths ranging between 100 m and 135 m. Due to the proximity of the project site to an active fault, the tunnel segments and the joints between the segments were designed and built to withstand high magnitude earthquakes.

The construction time for the ITT was approximately 4.5 years (2004 to 2008); archeological discoveries caused multiple project delays. The rail link was opened in 2013.

Lessons learned from this project are that ITT construction in difficult environments, including great water depths and high seismicity, are becoming technically feasible; however, they require costly construction measures. The chances that the new tunnel will be impacted by a 7.0 earthquake within the next 30 years were estimated to be 77%. The measures to address seismic impact included compaction grouting of the silty soil below the ITT down to 24 m below the sea bed to prevent liquefaction of the soil. In addition, each tunnel segment was designed to deform and the joints between rock tunnels and ITT were sealed with massive rubber and steel gaskets allowing the structure to shift without breaking. Floodgates at both ITT ends were installed to isolate the ITT section and prevent flooding of the rock tunnels during a major earthquake event.

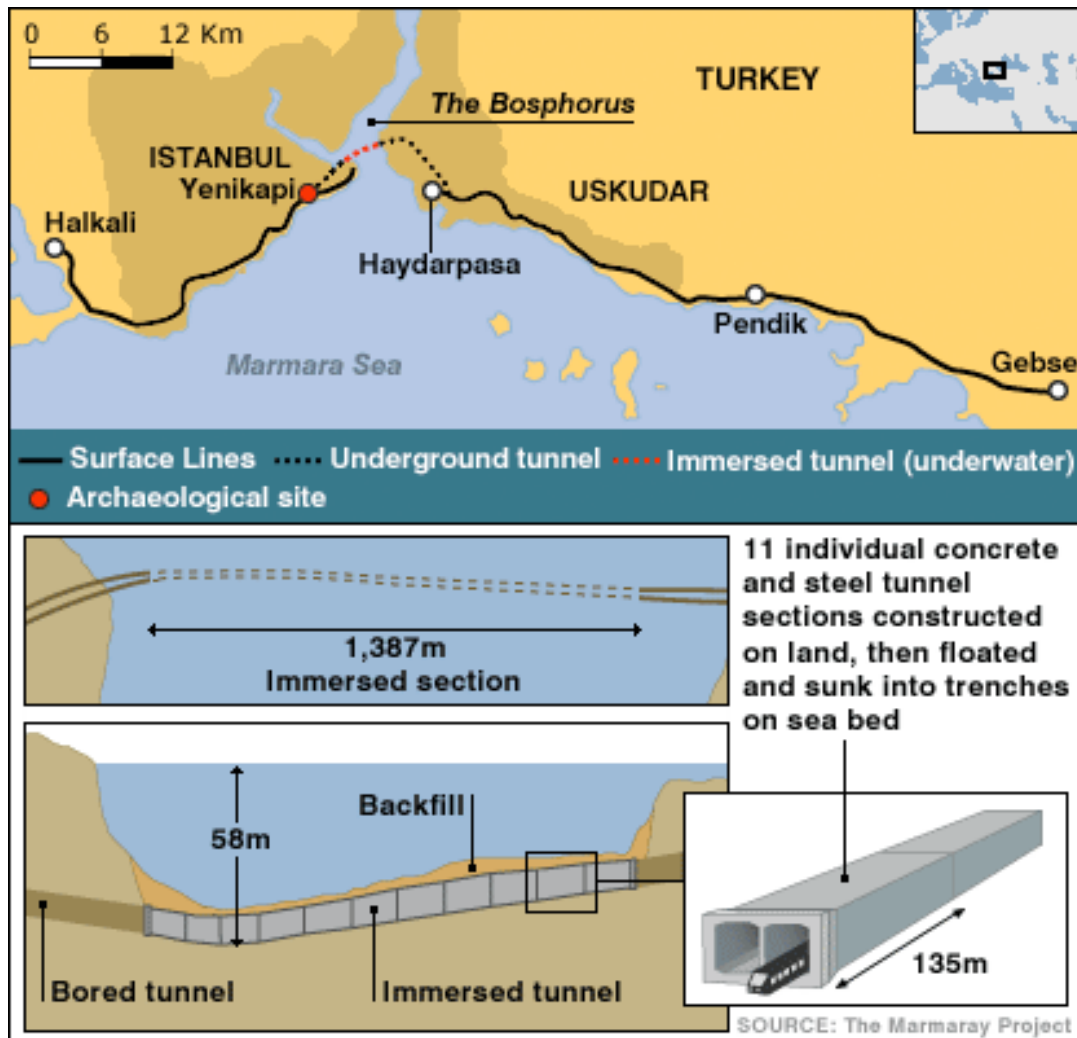


Figure 4-14: Marmaray ITT Fixed Link Construction Concept
 (<http://news.bbc.co.uk/2/hi/europe/4949862.stm>)

Other issues that had to be addressed during construction of the ITT included environmental issues (such as impact of dredge spill on fish migration through the Bosphorus Strait, noise impact on marine life and disposal of large amounts of contaminated material dredged from the sea floor for the ITT trench), archeological discoveries including numerous structures and artefacts, and the dense marine navigation in the Bosphorus Strait.

The tunnel costs increased due to several years of delays, largely related to archeological findings during excavation that required construction to stop for extended periods of time.

4.8 Fehmarnbelt Tunnel

The Fehmarnbelt fixed link (**Figure 4-15**) between Denmark and Germany is part of the highway and rail system connecting Denmark, Sweden and Germany. Construction start is currently scheduled for 2017 with anticipated completion of the project in 2026. The link will be constructed as an 18 km long ITT between the Danish island of Lolland and the German island of Fehmarn and will be the world's longest ITT.



Figure 4-15: Fehmarnbelt ITT Location between Denmark and Germany
(<https://spfaust.wordpress.com/2011/02/02/the-significance-of-another-big-tunnel/>)

The tunnel will be constructed using the same method as was used for the construction of the Øresund tunnel between Denmark and Sweden that was completed in 2000 (refer to 2004 Pre-Feasibility Study [1]) and will comprise a four-lane motorway and two electrified rail tracks (**Figure 4-16**). The roadway will allow a travel speed of 110 km/h while electric trains will be able to travel at 200 km/h, reducing the travel time between Denmark and Germany to 10 min by car and 7 minutes by train.

The immersed tunnel will consist of 79 standard elements, each with a length of 217 m, and 10 special elements with an additional lower floor for machinery. These special elements will be installed every 2 km along the alignment and allow for easier maintenance of the individual tunnel sections once the tunnel is under operation. Each element has a width of 42 m and comprises two tubes for the roadways as well as two tubes for the rail tracks. The tunnel elements will be installed in a 10 to 15 m deep trench with a maximum depth of 40 m below sea level.

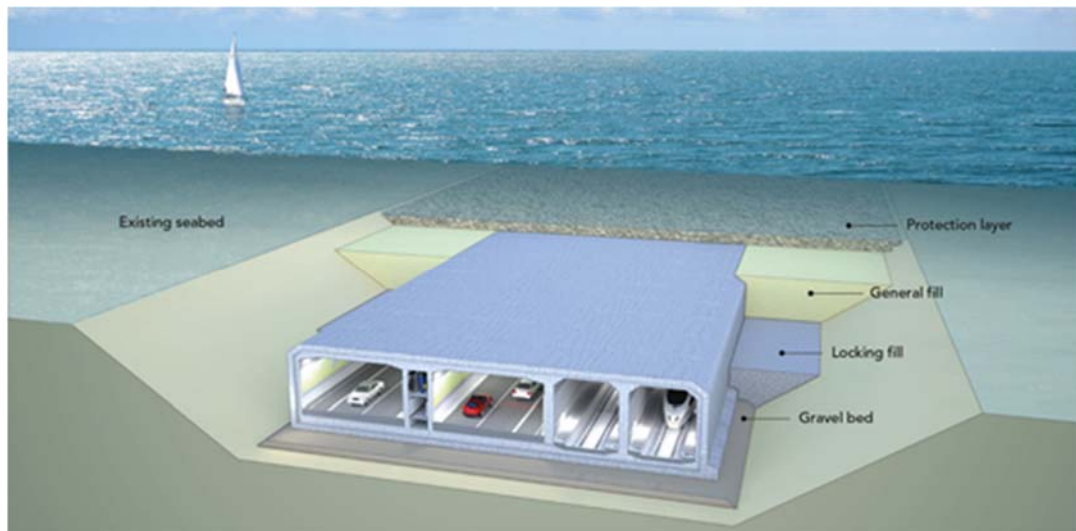


Figure 4-16: Fehmarnbelt ITT Construction Concept
 (<http://www.israscan.com/transport/>)

Although comparable in length with the Strait of Belle Isle fixed link, the water depth in the Fehmarn Strait is significantly lower and the environment less hostile compared to the conditions in the Strait of Belle Isle.

4.9 Other Fixed Link Projects

Currently under construction is the approximately 5.6 km long ITT that is part of the Hong Kong-Zhuhai-Macau Fixed Link project. The immersed tunnel comprises 33 elements installed in water depth of approximately 40 m.

Also under construction is the 36 km long Sheikh Jaber Al-Ahmad Al-Sabah Causeway which spans across Kuwait Bay between Kuwait City and the Subiyah area. The project includes the construction of a 27 km long low-level bridge across the bay, a main bridge with a span of 200 m, an elevated road, and a 5 km approach road onshore in Subiyah. The causeway crosses two artificial islands on its route. The project comprises over 1500 piles that are installed up to 72 m into the loose clay seabed in water depth between approximately 5 and 23 m. The bridges will be between 9 and 23 m above sea level.

Currently in the pre-construction stage is the Rogaland Fixed Link project in Norway (**Figure 4-17**). The proposed 27 km long Rogfast twin tube tunnel with an inner width of 8.5 m is planned to be excavated by traditional drill & blast technique at a record maximum depth below sea level of up to 390 m. The Ryfast tunnel system (**Figure 4-17**), which is currently under construction in Norway, is an approximately 20 km long subsea twin-tube tunnel system (including the Ryfylke Tunnel with 14.3 km and the Hundvåg Tunnel (completed) with 5.5 km) with two lanes per tube that is also excavated by drill & blast. The inner tunnel diameter of the Ryfast tunnels are approximately 8.5 m and 9.5 m respectively. These road tunnels will reach a maximum depth of up to 290 m (Ryfylke) and 95 m (Hundvåg) below sea level. Financing of the Ryfast tunnels will be mostly by toll income.

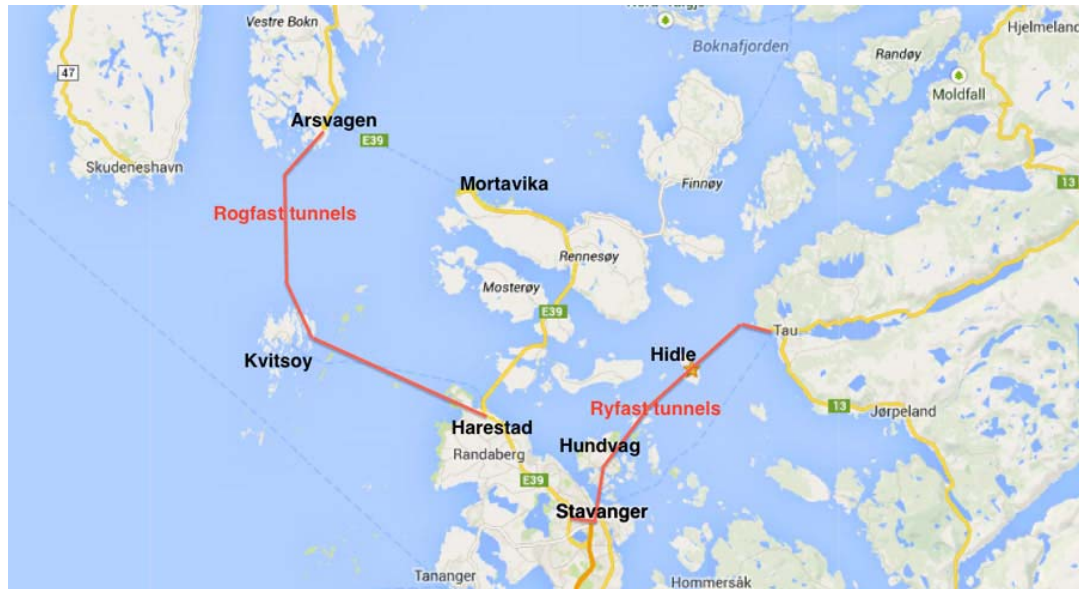


Figure 4-17: Rogfast Tunnels and Ryfast Tunnels in Norway
 (<https://www.tunneltalk.com/Norway-16Oct2013-World-record-subsea-Rogfast-highway-tunnel-moves-towards-final-design.php>)

4.10 Experience from Recent Relevant Fixed Link Projects and Implications for Strait of Belle Isle Fixed Link

In the following **Table 4-1** the relevant recent fixed link projects are summarized:

Table 4-1: Summary of Recent Fixed Links

| Fixed Link | Eurasia Tunnel | Follo Tunnel | Gotthard Base Tunnel | Eiksund Tunnel | Ryfast Tunnels | Ceneri Base Tunnel | Busan-Geoje Link | Marmaray Tunnel | Fehmarnbelt Link |
|---------------------------------------|---|---|---|-------------------|---|---|--------------------------------------|---------------------------------------|--|
| Type | Rock Tunnel | Rock Tunnel | Rock Tunnel | Rock Tunnel | Rock Tunnels | Rock Tunnel | ITT, Cable-Stayed Bridges | ITT | ITT |
| Excavation Method | TBM, partly Drill & Blast and cut and cover | TBM; partly Drill & Blast | TBM; partly Drill & Blast | Drill & Blast | Drill & Blast | Drill & Blast | n/a | n/a | n/a |
| Crossing Length | 5.4 km (3.4 km TBM; 1 km Drill & Blast) | Two tubes with 20 km each (18.5 km TBM; 1.5 km Drill & Blast/Split) | Two tubes with 57 km each | 7.8 km | Two tubes with 14.3 km each (Ryfylke) and 5.5 km each (Hundvåg) | Two tubes with 15.4 km each | 3.2 km ITT (total fixed link 8.2 km) | 1.4 km ITT (total fixed link 13.6 km) | 18 km |
| Excavation Cross Section | 13.7 m Ø | 10 m Ø | 8.8 / 9.6 m Ø; up to 13.1 m for Drill & Blast | 10 m width | Approx. 8.5 m and 9.5 m width | Approx. 8.7 m width (typ) | n/a | n/a | n/a |
| TBM Type | Mixed Shield | Double Shield | Open Face Gripper | n/a | n/a | n/a | n/a | n/a | n/a |
| Max Depth Below Sea Level | 106 m | n/a | n/a | 287 m | 290 m and 95 m | n/a | 48 m | 58 m | 40 m |
| Ground Cover | Variable | Variable | Up to 2450 m | 50 m minimum | 50 m minimum | Few m to 900m maximum | n/a | n/a | n/a |
| Road | ✓ | -- | -- | ✓ | ✓ | -- | ✓ | -- | ✓ |
| Rail | -- | ✓ | ✓ | -- | -- | ✓ | -- | ✓ | ✓ |
| Status | In use since 2016 | Under construction | In use since 2016 | In use since 2008 | Under construction | Excavation completed in 2016; opening planned in 2019 | In use since 2010 | In use since 2013 | Pre-construction (opening planned in 2026) |
| Vehicles per day (vpd) | 100,000 ²⁾ | n/a | n/a | 2,880 (in 2014) | 8,000 and 25,000 by 2035 ²⁾ | n/a | ¹⁾ | n/a | 9,500 ²⁾ |
| Cost CAN\$ (2017)³⁾ | 1.5 Billion | ¹⁾ | 16.5 Billion | 105 Million | 780 Million ²⁾ | 3.25 Billion ²⁾ | 1.1 Billion | 5 Billion | 13 Billion ²⁾ |

¹⁾ Information not available
²⁾ Forecast/Estimate
³⁾ Approximate cost estimates based on cost information available online

4.10.1 TBM Tunnels

There have been significant technological advances in mechanized tunnelling using TBMs over the recent years to successfully extend the use of TBMs to a wider range of ground conditions, improve safety and achieve higher advance rates. The challenge of mixed ground conditions has been addressed with technological developments such as the dual mode machines that can be adjusted to varying ground conditions.

The construction of a 4.8 km long intake tunnel at Lake Mead in the United States is an example of the successful use of a dual mode tunnelling machine. The rock in the project area contained alternating layers of hard rock and conglomerates as well as fault zones and required a hard rock TBM that could be converted rapidly from an open-face machine to a slurry machine with a closed pressurised face that could withstand hydrostatic pressures of up to 17 bar. During tunnelling, actual pressures of 15 bar were encountered. Successful sub-sea tunnel excavation under high hydrostatic pressures of 12 bar using a Mixed Shield TBM was also achieved at the Eurasia tunnel. A further significant advancement for safe TBM tunnelling is the remote cutterhead monitoring and replacement system that was implemented at the Eurasia Tunnel. This system monitored the cutterhead and tool conditions using sensors and cameras installed at the cutterhead. Replacement of cutter tools could be carried out from inside the cutterhead without the need of human exposure to pressurized conditions in front of the cutterhead.

To date, the longest and deepest rail tunnel is the Gotthard Base Tunnel with a total length of 57 km per tunnel tube, of which individual sections of 7 to 14 km length were excavated by TBM. The ground cover above the tunnel varies between approximately 500 m and 2,450 m. The excavation with an open face TBM and subsequently installed rock support (mostly rock bolts and shotcrete; steel arches and pre-grouting in difficult rock conditions) managed various challenges including difficult ground conditions, large fault zones, high pressure from the overlying rock cover and rock deformations of up to 0.7 m. Average advance rates (excluding downtime) between 12 and 18 m per work day were achieved in the Gotthard Base Tunnel, with maximum performances of up to 56 m within 24 hrs. The advance rates of the TBM currently excavating the Ryfast Tunnel are on average approximately 12 m to 15 m per day.

4.10.2 Drill & Blast Tunnels

Drill & blast tunnels (such as the Eiksund Tunnel) have continued to be implemented successfully in deep sub-sea conditions at depth of currently up to 287 m below sea level under high hydrostatic pressures. Many of these tunnels have great lengths such as the 20 km long Ryfast tunnels that are currently under construction. Measures such as probe drilling and pre-excavation grouting to control groundwater inflows and improve the stability of the surrounding rock have been in use successfully for many years and the rock support and reinforcement technology has developed to better address the issue of corrosion caused by aggressive environments such as exposure to sea water.

The proposed Rogfast Tunnel as part of the Rogaland Fixed Link project in Norway is planned to be excavated by traditional drill & blast technique. The tunnel will set a new record

for the longest and deepest drill & blast sub-sea tunnel with a length of 27 km and a maximum depth of 390 m below sea level.

Drill & blast methods have also been used successfully for long tunnels with great rock cover, such as the Ceneri Base Tunnel, as well as in challenging rock conditions, as encountered on the up to approximately 8 km long sections of the Gotthard Base Tunnel. Average advance rates in favourable rock conditions between 3 m and 4.5 m per work day were achieved in the Gotthard Base Tunnel, with maximum performances of up to 11.5 m within 24 hrs. Average advance rates in unfavourable conditions were approximately 1 m per work day.

4.10.3 Immersed Tube Tunnel (ITT)

The technology for ITT installations at greater depths and in challenging environments has developed over the past years. However, to date, no ITT has been constructed in water depths greater than approximately 60 m which is still significantly shallower compared with the over 100 m water depth at the Strait of Belle Isle.

The longest ITT to date is the approximately 5.6 km long immersed tunnel that is installed approximately 40 m below the water level as part of the Hong Kong-Zhuhai-Macau Fixed Link project. This length is still significantly shorter than the Strait of Belle Isle crossing with its minimum length of approximately 20 km. The Fehmarnbelt ITT with a comparable length of 19 km is yet to be built; however, it has to be noted that experience that will arise from the construction of the Fehmarnbelt ITT will be related to a significantly shallower water depth and a less hostile environment at the project location.

No ITT project to date has been executed in an area known for iceberg movements and had to be designed to withstand iceberg pitting and scour. Hence, no relevant experience from previous projects can be related to the Strait of Belle Isle Fixed Link. The risk associated with the construction of an ITT Fixed Link across the Strait of Belle Isle is still considered significant.

A detailed assessment and cost estimate for the ITT option was provided in the Pre-Feasibility Study in 2004 [1] and [2]. Overall, the scheme for the construction of an ITT fixed link remains the same as described in the 2004 study. However, due to the recent installation of the HVDC cable crossing on the sea floor between Forteau Point (Labrador) and Shoal Cove (Newfoundland), the dredging of a trench for the ITT installation on the bottom of the Strait between Point Amour and Yankee Point would interfere with the cable crossing and, hence, the alignment of the ITT would have to be changed. Since the Strait has its narrowest width between Point Amour and Yankee Point, a relocated alignment would inevitably result in a longer tunnel crossing, causing significantly higher costs. In addition, an alignment further east would place the ITT into shallower waters and would increase the risk for iceberg impacts.

Difficulties during trenching of the sea floor for the installation of the tunnel tubes will arise from the lack of overburden and the various steep bedrock scarps and offsets of up to 30 m height that are located across the Strait and trend perpendicular to the proposed alignment.

Since an ITT fixed link is not considered the preferred option for the Strait of Belle Isle, no further update of the previous assessment has been carried out for this study.

4.11 Outlook

A new technology currently under investigation for passenger and freight transportation is the Hyperloop, a mode of transportation that involves a sealed tube system in which travelling would be possible without friction or air resistance. Several potential above-ground routes as well as tunneled routes, for example a 500 km long connection between Stockholm and Helsinki passing under the Baltic Sea, have been proposed or are presently in an early stage of pre-feasibility studying, mostly for mid-range distances of some hundreds of kilometers. However, so far, no Hyperloop route has been implemented. Currently, the technology focusses on fast and energy efficient mass transportation, which is not the primary focus for the Strait of Belle Isle Fixed Link. However, going forward, the ongoing development of the Hyperloop concept could in the future provide a feasible transportation mode also for shorter, less frequented routes.

The developing technology of Automated Vehicles (AV) and Connected Vehicles (CV) will lead to new requirements for road and tunnel infrastructures. CVs are equipped with devices that allow the connection with other devices inside the vehicle or with devices, networks or services outside the vehicle, including, for example, other vehicles or infrastructures. Connected car features can have different functions such as safety, navigation or infotainment. Most newer generation vehicles possess some form of connection which is typically used to assist the driver to enhance safety and navigation. AVs, however, are defined as self-driving or driverless vehicles that detect their surrounding environment using artificial intelligence, sensors and global positioning systems and that make their own decisions about how to act rather than being controlled by a driver. In Canada as well as in many other countries, the technology of self-driving vehicles is currently in a stage of testing and does not yet serve the mass market. However, as the technology advances, self-driving vehicles are likely to become more and more frequent on the roads, resulting in the requirements for road and tunnel infrastructures to be equipped with the necessary technology to serve these vehicles.

It should be noted that large scale adoption of electrically powered vehicles would significantly reduce the ventilation requirements for road tunnels during normal operations.

5. Summary of Relevant Investigations and Studies since 2004

Several investigations, including offshore and onshore field investigations as well as desktop studies, have been performed in the vicinity of the project area since 2004. Most of these were related to the HVDC cable crossing of the Strait of Belle Isle as part of Nalcor Energy’s Lower Churchill Project. Available information from these investigations were used to update the assumptions and background information used in 2004. **Figure 5-1** and Figure A1 in Appendix A provide an overview of the borehole locations drilled during previous geotechnical investigations.

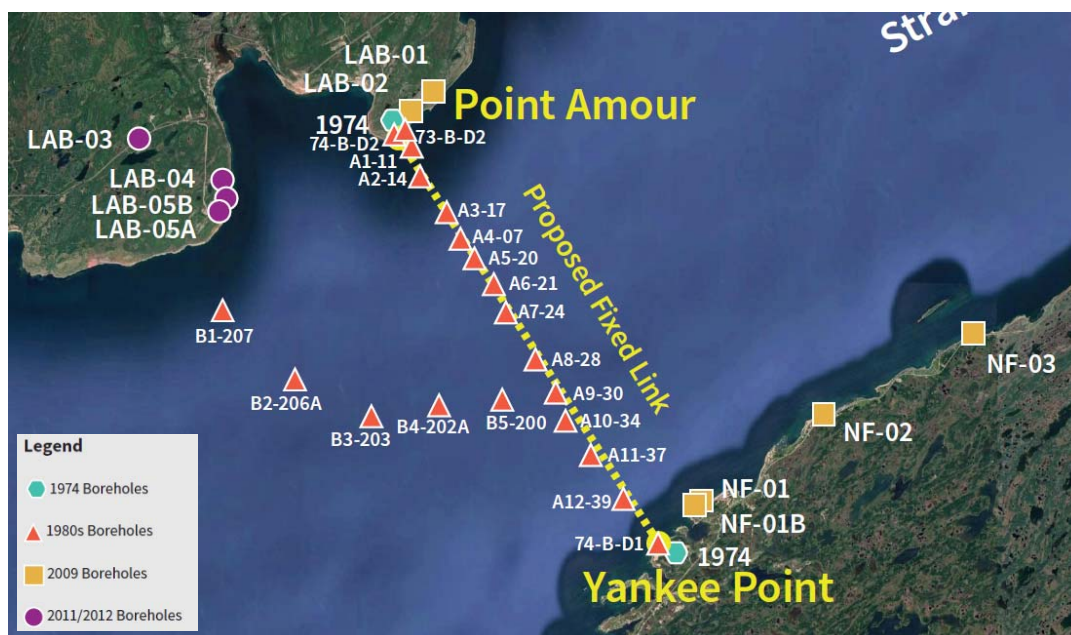


Figure 5-1: Borehole Locations near the Fixed Link Project Location

5.1 Onshore Field Investigations

2009 Geotechnical Investigation: The geotechnical investigation carried out by SNC-Lavalin (SNC) in 2009 comprised six inclined boreholes, drilled from shore into the basement rocks underneath the Strait of Belle Isle, to investigate the subsurface conditions for a potential HVDC cable tunnel across the Strait [10] to [13]. Two holes were drilled on the Labrador side of the Strait (borehole lengths of 540 m and 170 m); four holes were advanced on the Newfoundland side (borehole lengths of 71 m, 890 m, 955 m and 146 m). Packer tests were carried out in all holes to investigate the permeability of the rock. In addition to the drilling, seven test pits were excavated at Point Amour to determine the overburden thickness and bedrock surface elevation. The SNC-Lavalin borehole data was supplemented by an evaluation of the rock conditions by Landsvirkjun Power/Mannvit Engineering (Landsvirkjun) [9].

2011/2012 Geotechnical Investigation: The geotechnical investigation carried out by AMEC in 2011 and 2012 included drilling of four boreholes at Forteau Point on the Labrador side of the Strait of Belle Isle [19]. In addition to the drilling, 42 test pits were excavated at Forteau Point to determine the composition of the overburden material for future construction work.

2011/2012 HDD Pilot Bore Investigation: A horizontal directional drilling (HDD) pilot bore investigation was carried out by Hatch in 2011 and 2012 near Shoal Cove, Newfoundland, comprising a 251 mm diameter inclined pilot hole drilled approximately 1,558 m from the shore into the bedrock below the Strait of Belle Isle and subsequent reaming of the upper 326 m of the pilot bore to a diameter of 610 mm (refer to [21]).

5.2 Offshore Field Investigations

2007 Geophysical Survey: A high resolution acoustic survey was carried out by Fugro Jacques Geosurveys Inc. (Fugro) to characterize bathymetric and seafloor conditions along potential cable routes across the Strait of Belle Isle [4].

2009 Geophysical Survey: A 2D seismic survey was carried out by Fugro to define sub-surface bedrock structures along a possible subsea cable tunnel route [15].

2011 Geophysical Investigation: A geophysical survey was carried out by Fugro including nearshore and offshore geophysical surveys and general visual inspections of the proposed HDD exit location at Forteau Point, Labrador [20].

5.3 Desktop Studies

2008 Data Compilation Study: Fugro prepared a compilation of existing geophysical and environmental data relevant to a proposed seabed HVDC power transmission corridor across the Strait of Belle Isle [3]. The purpose of the study was to characterize physical and environmental conditions in the Strait that could affect the proposed subsea cable installations, and to identify potential human and environmental constraints to cable route design and planning.

2009 Desktop Study of Bedrock Conditions: Fugro prepared a study describing the current geological knowledge in support of a continuous bedrock micro-tunnel crossing of the Strait of Belle Isle, using the information from drilling campaigns at Point Amour and Yankee Point in 1973 and 1974, and from the 1981 drilling campaign executed by Beaver Dredging Company Ltd. [8].

2010 Conceptual Design for Cable Crossing: A conceptual design report was prepared by Hatch Mott MacDonald for the Strait of Belle Isle cable conduit option [17].

2010 Feasibility Study for HDD Cable Crossing: A feasibility study was carried out by Hatch for the installation of onshore cable conduits using horizontal directional drilling (HDD) [18].

6. Physical Environment at the Strait of Belle Isle

Since the preparation of the previous Pre-Feasibility Study in 2004, new information regarding the project site environment has been collected. Updates on the data included in the 2004 study are provided in the following sections. Information provided in the 2004 study is repeated only where it was deemed required for the understanding in context with the new information.

6.1 Climate Data

Long-term data for Climate Normals and Averages was available from the Government of Canada, Environment and Natural Resources [30] for the time period between 1981 and 2010, collected at the weather stations at Flower’s Cove (Newfoundland) and Lourdes-de-Blanc-Sablon (Québec) as the nearest stations to the project site. The relevant data for these stations is listed in **Table 6-1**. The long-term data from the Blanc-Sablon station including the most recent data up to 2014 shows that most of the monthly extreme high temperatures occurred in recent years, whereas the occurrence of monthly extreme low temperatures has become less likely, indicating that overall the temperatures show a rising trend. Only few occasions with temperatures below -30°C occurred. This is consistent with the observed tendency of reduced sea ice cover in the Strait of Belle Isle in the recent years (refer to Section 6.3).

Overall, the changes noted in the climate data in comparison to the data available in 2004 are not expected to have a significant impact on the proposed fixed link options.

Table 6-1: Summary of Long-Term Climate Data (1981 to 2010)

| | | Flower’s Cove (Newfoundland) | Lourdes-de-Blanc-Sablon (Québec) |
|---|---------------------------|------------------------------------|--------------------------------------|
| Air Temperature | | | |
| Daily Maximum Temperature in Aug [°C] | | 17.3 ²⁾ | 16.6 ³⁾ |
| Daily Minimum Temperature in Feb [°C] | | -15.8 ²⁾ | -16.5 ³⁾ |
| Daily Average Temperature in Feb [°C] | | -11.4 ²⁾ | -12 ³⁾ |
| Extreme Minimum Temperature [°C] | In February ²⁾ | -34 ²⁾ (Feb 04, 1995) | -34.1 ³⁾ (Feb 08, 1994) |
| | In March ²⁾ | -31 ²⁾ (March 08, 1990) | -32.5 ³⁾ (March 10, 1986) |
| Precipitation | | | |
| Average Total Precipitation per Year [mm] | | 1039.0 ²⁾ | 1021.7 ²⁾ |
| Extreme Daily Rainfall [mm] | | 74 ²⁾ (Jun 08, 1995) | 88 ²⁾ (Oct 02, 2010) |
| Extreme Daily Snowfall [cm] | | 36 ²⁾ (Jan 25, 1982) | 41.8 ²⁾ (March 15, 2001) |
| Wind | | | |
| Average Wind Speed per Year [km/h] | | Not available | 19.6 ⁴⁾ |
| Predominant Wind Direction | | Not available | W in winter, SW in summer |
| Maximum Hourly Wind Speed [km/h] | | Not available | 111 (March 17, 1987, NE) |
| Maximum Wind Gust Speed [km/h] | | Not available | 141 (Jan 12, 1987) |
| Freezing Degree Days (FDD) | | | |
| FDD Below 0°C | | 1148.3 ¹⁾ | 1424.7 ¹⁾ |

¹⁾ Cumulative Freezing Degree Days (FDD) for at least 15 years of record.

²⁾ For at least 15 years of record.

³⁾ For at least 20 years of record.

⁴⁾ World Meteorological Organization (WMO) "3 and 5 rule" (i.e. no more than 3 consecutive years and no more than 5 total missing years for either temperature or precipitation).

An updated version of the National Building Code of Canada (NBC) [22] became effective in 2015. The Code provides climate design data for selected locations. On the Newfoundland side of the Strait, the location closest to the project area is St. Anthony; on the Labrador side, the nearby location listed in the Code is Harrington Harbour in Québec. The most relevant climate design data for these locations is summarized in **Table 6-2**. The recent version of the NBC used an annual probability of exceedance of 1 in 50 years instead of the previously used 1 in 100 years in the version in effect in 2004.

Table 6-2: Climate Design Data, National Building Code of Canada (2015)

| | | St. Anthony (Newfoundland) | Harrington Harbour (Québec) |
|---|--------------------|-------------------------------|--------------------------------|
| Minimum Design Air Temperature (January) [°C] | 1% ¹⁾ | -27 | -29 |
| | 2.5% ¹⁾ | -25 | -27 |
| One Day Rain [mm] ²⁾ | | 86 | 96 |
| Annual Total Precipitation [mm] ³⁾ | | 1280 | 1150 |
| Snow Load [kPa] ²⁾ | | 6.1 | 4.9 |
| Hourly Wind Pressures [kPa] ²⁾ | | 0.87 | 0.72 |

¹⁾ The 1% and 2.5% values represent percentiles of the cumulative frequency distribution of hourly temperatures and correspond to January temperatures that are colder for 8 and 19 hours, respectively, on average over the long term.

²⁾ Probability of exceedance of 1 in 50 in any one year.

³⁾ Precipitation observations for the 30-year period from 1961 to 1990.

No additional studies regarding icing that were carried out since the 2004 Pre-Feasibility Study were found by Hatch.

6.2 Oceanography

Recent bathymetry data was collected by Fugro in 2007 as part of the subsea cable route survey for the HVDC cable crossing (refer to [4]). The focus of the survey was on the proposed cable route; however, the survey also covered the area of the proposed Strait of Belle Isle fixed link crossing. The bathymetric information for the project area is shown in **Figure 6-1**. The data indicates a wide area of deep water in the mid-portion of the Strait (Central Trough) as well as deep water close to the Labrador shore, with water depths up to 115 m. These deep-water areas are divided by an area of shallower waters (Bank) with water depths less than 75 m. Shallower waters occur also along the shore of Newfoundland. **Figure 6-1** provides the surveyed water depths in the various zones of the Strait of Belle Isle. (Note that the cable route shown in **Figure 6-1** does not present the final location of the cable crossing.)

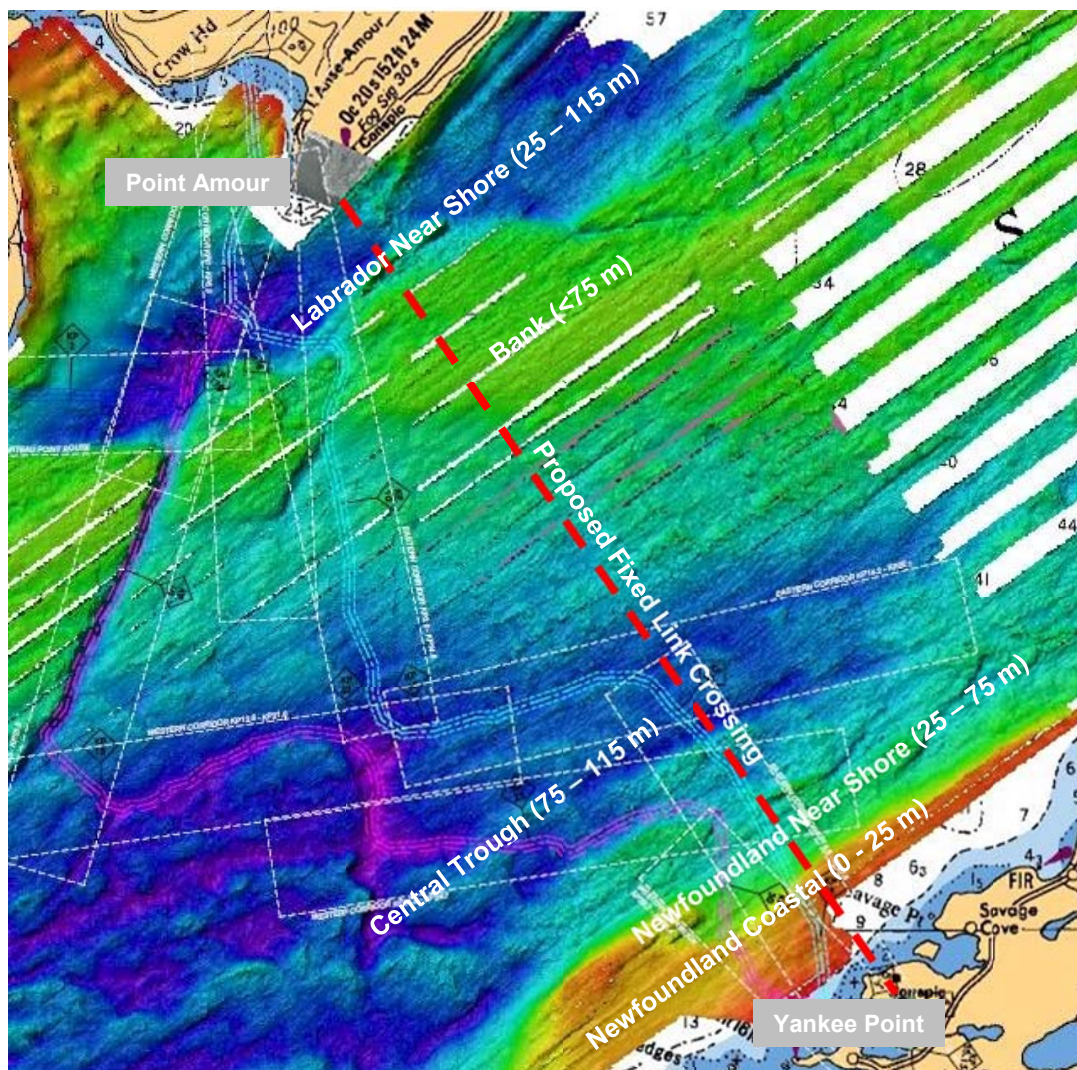


Figure 6-1: Bathymetry Data at Strait of Belle Isle (modified from [4] and [7])

No relevant new information regarding waves and currents in the Strait of Belle Isle became available since the Pre-Feasibility Study in 2004.

6.3 Sea Ice

Statistical data regarding sea ice in the Strait of Belle Isle (Government of Canada, Environment and Climate Change, <https://www.ec.gc.ca/glaces-ice> [29]) provides information about sea ice conditions over the past 30 years. The data for the years 1981 to 2010 indicates that the sea ice cover in the Strait of Belle Isle can last until the end of July and can form as early as mid December. The ice coverage can vary significantly from year to year; however, in general, from 1980/81 to 1994/95, the condition of the sea ice cover were above normal conditions while the condition between 1995/96 and 2009/10 were below normal. The least amount of ice for the period 1981 to 2010 occurred in the winter of 2009/10. However, sea ice conditions vary and extreme conditions with thick sea ice cover have occurred in recent years, such as in 2017.

Figure 6-2 illustrates the minimum (2010) and maximum (1993) extent of ice cover in the Newfoundland and Labrador area encountered on March 1st during the time period from 1981 to 2010.

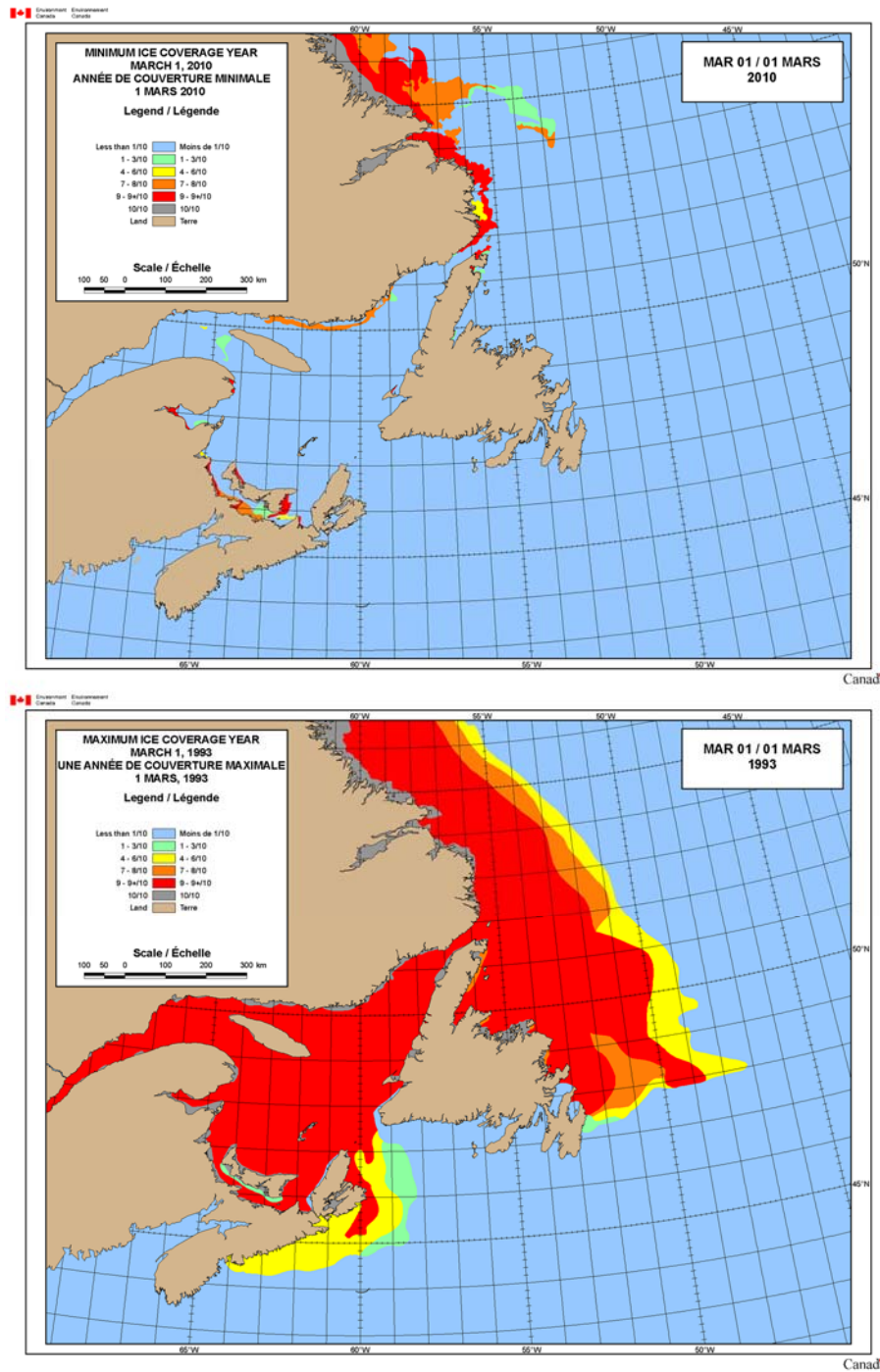


Figure 6-2: Minimum (top) and Maximum (bottom) Ice Cover on March 01 (for the time period between 1981 and 2010) [29]

6.4 Icebergs and Iceberg Scour

Recent information regarding iceberg scour is presented in the investigation report by Fugro [4] pertaining to the subsea cable route survey carried out in 2007. The investigation was commissioned by Nalcor Energy to investigate two proposed subsea HVDC corridors across the Strait of Belle Isle. During this investigation, numerous intersecting iceberg scours were noted on the seafloor in larger water depths (below about 100 m with a transition zone starting at about 80 m depth). These scours were interpreted as having occurred in the past, possibly thousands of years ago. Using the 2007 data, Fugro also interpreted numerous northeast-southwest striking boulder- and cobble-rich berms in water depth greater than 80 m, which were previously understood as glacial Ribbed Moraines, as relict iceberg scours. The absence of these old scour marks in shallower water led to the interpretation that in the past, iceberg scours were confined to greater depths due to different environmental conditions at that time.

In their survey report from 2010 [14], Fugro noted that the seabed in the deep Central Trough has an overall rough texture that was shaped by iceberg grounding and scouring. Iceberg scours were interpreted to align with the dominant currents in the Strait and with the bedrock structure; it was assumed that the bedrock topography could have steered scouring ice keels.

Recent iceberg scour marks were observed by Fugro in water depths up to 77 m on the Labrador side and up to 69 m on the Newfoundland side [4]. **Figure 6-3** shows examples of apparently fresh iceberg scour marks offshore from Forteau Point, Labrador.

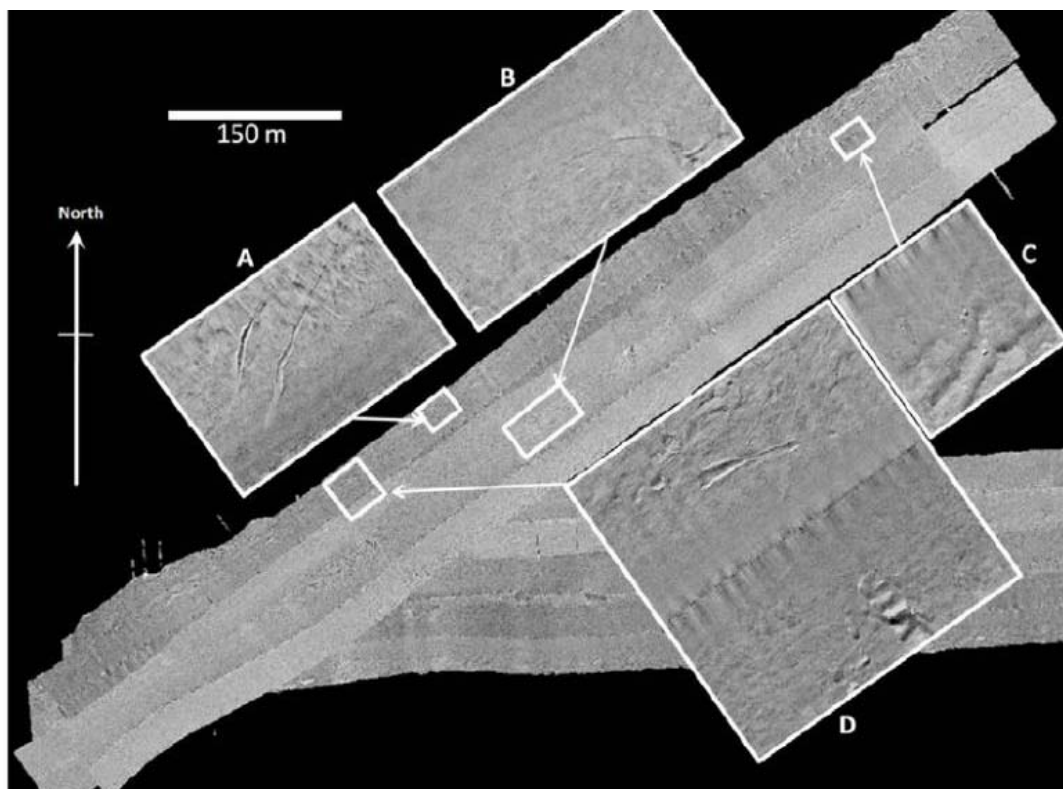


Figure 6-3: Examples of Recent Iceberg Scours near Forteau Point [4]

The depth of these features was typically less than 0.75 m. The longest recent scour mark was noted to be 350 m. Recent scour marks are typically oriented approximately parallel to the Strait. No fresh scour marks were observed in deeper water during the 2007 investigation, which led to the conclusion that icebergs with larger drafts are prevented from entering the Strait of Belle Isle, presumably due to the shoal located east of the project area with water depth up to approximately 75 m.

In the past years, HVDC cable crossing was installed across the Strait between Forteau Point in Labrador and Shoal Cove in Newfoundland as part of the Lower Churchill Project. The cable crossing project included a cable installation in underground conduits onshore and an offshore placement of the cable on the seafloor where the cable was covered with a rock berm for protection against impacts from icebergs and fishery. **Figure 6-4** shows the general cable crossing layout.



Figure 6-4: Location of HVDC Cable Crossing across the Strait of Belle Isle
 (www.muskatfalls.nalcorenergy.com [27])

For the exit elevations of the cable conduits and for the required cover of the subsea cables, shielding of the cable crossing route against iceberg scour due to the shallower waters upstream of the project area was considered. Exit elevations for the cable conduits of 70 m below water level on the Newfoundland side and 80 m below water level on the Labrador side were chosen, considering a shielding water depth of 60 m (**Figure 6-5**).

It is assumed that the exit elevations and the height of the rock coverage of the subsea cable were chosen based on the risk assessment carried out by C-Core, as summarized in C-Core's 2007 report regarding ice scour risk (included in the Data Compilation Study by Fugro [3]). In their 2007 risk assessment for various cable crossings, C-Core evaluated shielding effects for the proposed cable routes and considered areas that are shielded from iceberg impact. **Figure 6-6** illustrates the sheltered areas along the proposed routes.

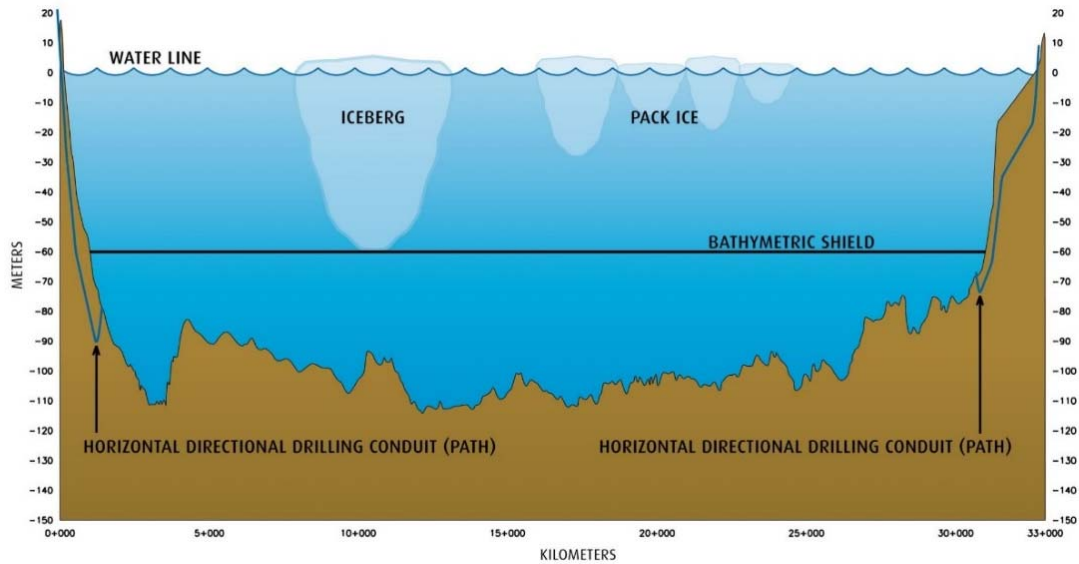


Figure 6-5: HVDC Cable Conduit Exit Depths
 (www.muskatfalls.nalcorenergy.com [27])

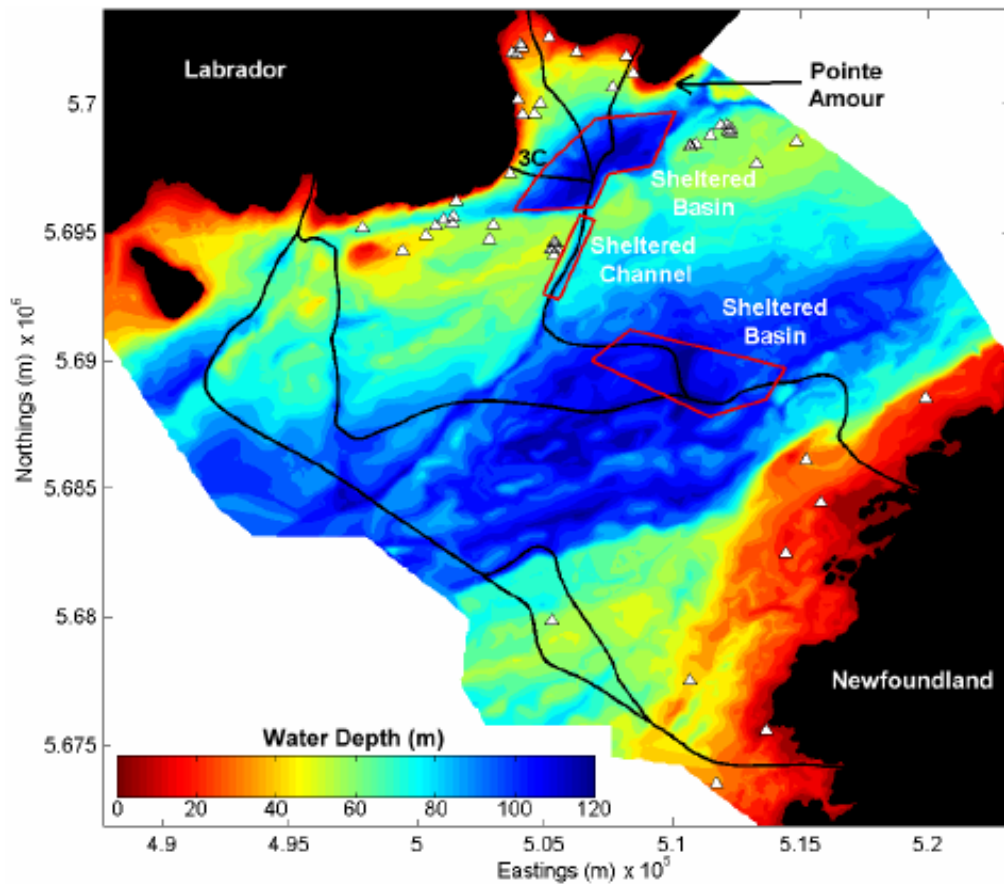


Figure 6-6: Potential Cable Routes and Sheltered Areas (C-Core 2007, included in [3])

No additional information regarding numbers, travel routes or grounding locations of icebergs drifting into the Strait of Belle Isle was available for this study update. However, some news articles from past years indicate that in some years a higher than normal number of icebergs drift into the Strait, such as in 2013 with reportedly ‘three to four times more [icebergs] than in the previous five years’ (<http://globalnews.ca/news/701657/watch-up-close-look-at-icebergs-off-newfoundland/>) or in 2017 when by April the amount of icebergs reached a number usually reported over the entire ice season (<http://www.citynews.ca/2017/04/28/iceberg-dead-ahead-record-673-icebergs-counted-off-newfoundland/>).

6.5 Geology

The bedrock and surficial geology was described in the 2004 Pre-Feasibility Study [1]. Based on the 2004 study by HMM and additional recent studies, such as the study by Fugro [8], a brief overview of the geological setting in the general project area is presented below; the discussion follows largely the geological overview presented in [17].

6.5.1 *General Geological Setting at the Strait of Belle Isle*

The general geology at the Strait of Belle Isle has been described in the 2004 Pre-Feasibility Study [1] and since 2004 in numerous reports related to the investigations and studies for the Strait of Belle Isle HVDC cable crossing as part of the Lower Churchill Project. A brief general overview is provided below; for a detailed description, the reader is referred to the previous study as well as the report ‘Descriptive Overview of Regional Bedrock Geology’ [8], prepared by Fugro Jacques Geosurveys Inc. for the HVDC cable crossing project.

The bedrock at the Strait of Belle Isle between Point Amour (Labrador) and Yankee Point (Newfoundland) comprises a vertical succession of sedimentary rock of Cambrian age overlying much older gneisses of Precambrian age (basement rock). The general project area and a geological cross section presented in **Figure 6-7** and **Figure 6-8** (modified from [22]) provide a general overview of the interpreted bedrock stratigraphy underneath the Strait of Belle Isle, showing the Precambrian basement rock (AHn) underlying the gently east to southeast dipping sedimentary rocks. (Note that the Eddies Cove Formation shown in **Figure 6-8** was renamed to the Port-au-Port Group which includes the Formations Petit Jardin, March Point and Berry Head [21]. These formation names were used in the recent geotechnical investigations; however, the Berry Head Formation does not exist in the project area).

The Precambrian basement rocks in the area of the Strait of Belle Isle belong to the Grenville Province and consist of a complex of metamorphic and granitic rocks. On the Newfoundland side, the complex consists of schists and gneisses, cut by several granitic and gabbroic intrusives and by numerous steeply dipping diabase dykes striking north-easterly. On the Labrador side, the complex consists of granite and granodiorite intrusives. Near the project site, the upper basement rocks are deeply eroded and weathered gneisses.

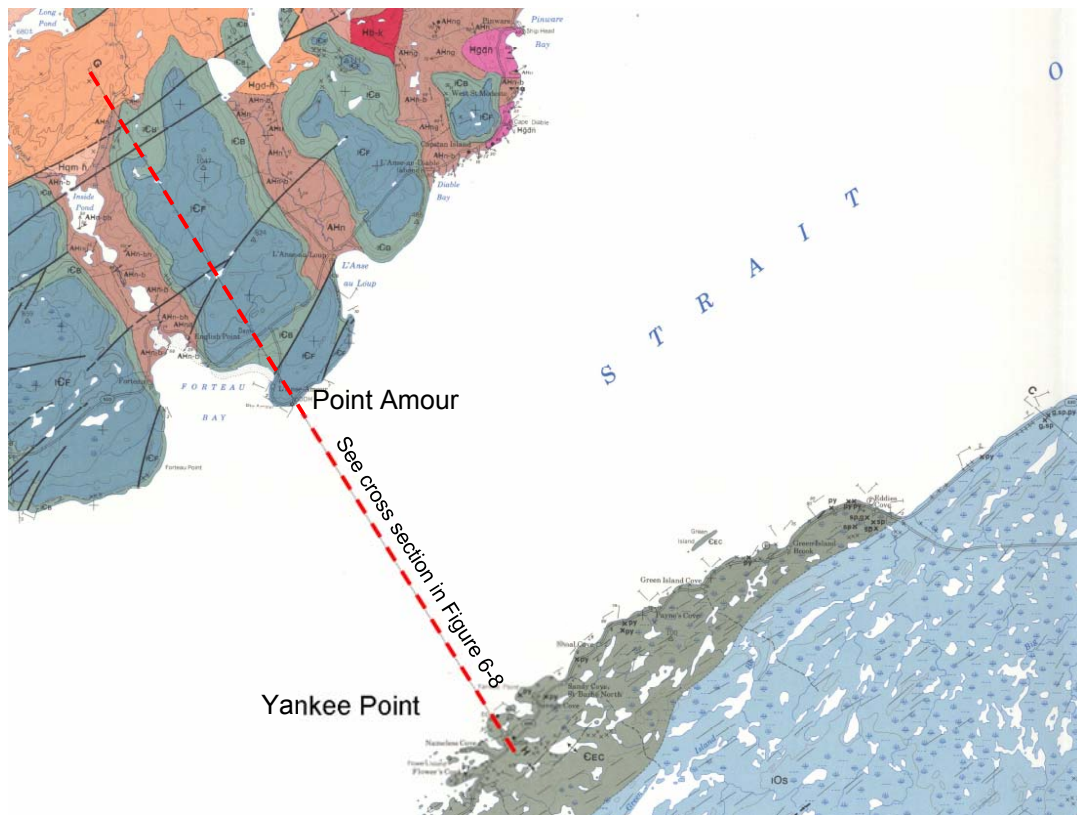


Figure 6-7: Geological Plan View of the Project Area (modified after [22])

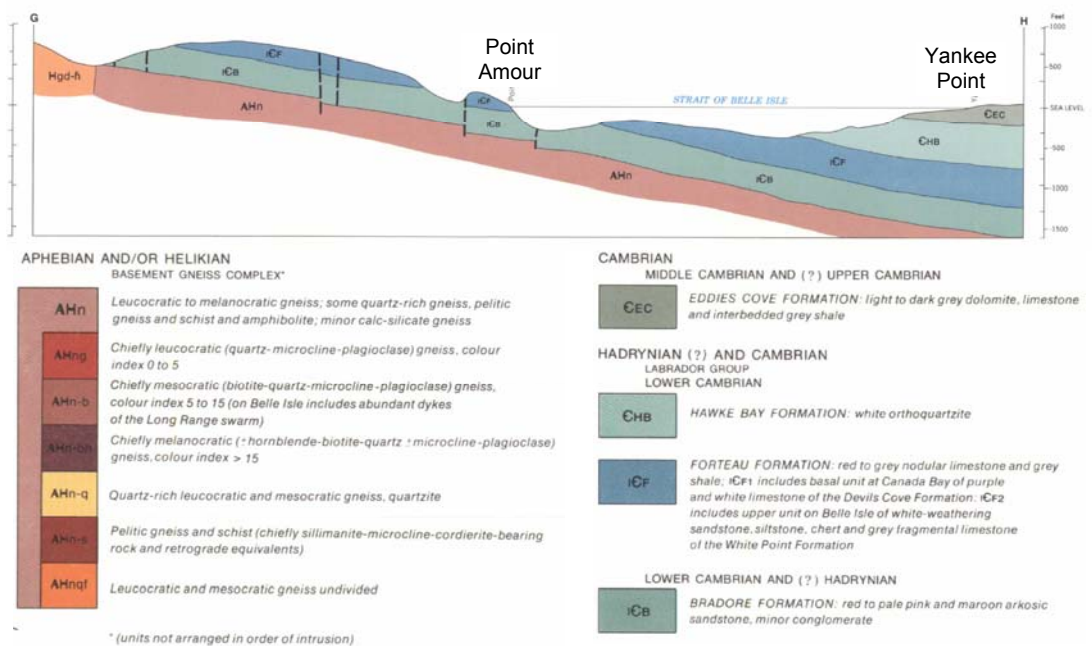


Figure 6-8: Geological Cross Section between Point Amour (A) and Yankee Point (B) (modified after [22])

The sedimentary rock layers overlying the Precambrian basement rock are nearly flat-lying or gently dipping east to southeast and are mainly composed of sandstones, dolomite, limestone and shale. The contact between the sedimentary rock and the Precambrian rock strata is an angular unconformity that occurs approximately 95 m below the Labrador shoreline and approximately 460 m below the Newfoundland shoreline. The contact dips gently southerly but is frequently offset vertically by extensive faulting occurring in the Strait.

Geophysical offshore investigations and visual observations on shore indicate extensive faulting in the sedimentary rock strata and in the Precambrian basement rock. The predominant fault set strikes approximately parallel with the direction of the Strait from north-east to south-west and has vertical offsets in the order of 10 to 40 m which are typical for normal (downwards slip) or reverse (thrust or upwards slip) faults. Less prominent faulting occurs at right angles to the predominant faults. Joint sets generally follow the fault pattern.

A seismic profile across the Strait of Belle Isle between Point Amour and Yankee Point showing the approximate formation boundaries as well as mapped faults is presented in **Figure 6-9**.

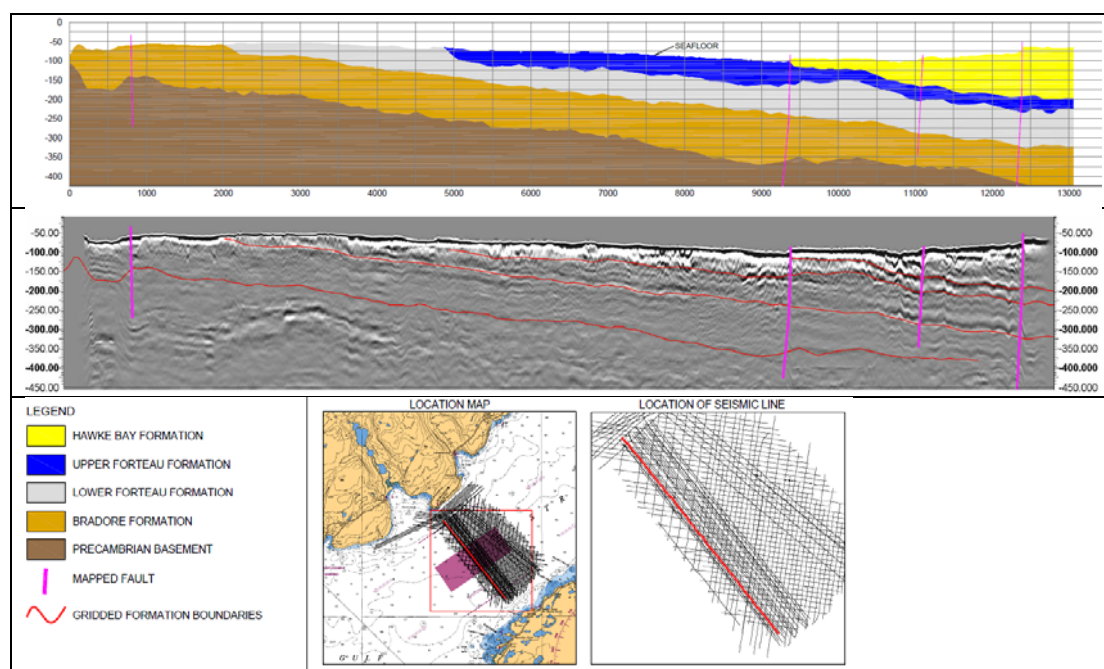


Figure 6-9: Seismic Profile across the Strait of Belle Isle (modified after [15])

The seafloor topography is dominated by the underlying bedrock structure and is gently dipping towards east and southeast. Features such as bedrock outcrops, ridges and channels are frequent and glacial till, erratics and striations are common. The overburden on the seabed is typically shallow and composed mostly of uniformly distributed sand, gravel, cobbles and boulders. In many places, a more recent deposition of a thin layer of shells and shell fragments overlies the soil and bedrock.

6.5.2 Seabed Surface Stratigraphy

An understanding of the surficial geology is needed for an assessment of the immersed tube tunnel concept as well as for the nearshore and offshore excavations for the tunnel approaches. Based on the information presented by Fugro in 2008 [4], prominent seafloor structures identified during recent geophysical surveys included bedrock channels and scarps, moraine ridges and iceberg scour. **Figure 6-10** provides an overview of these features along the route between Point Amour and Yankee Point.

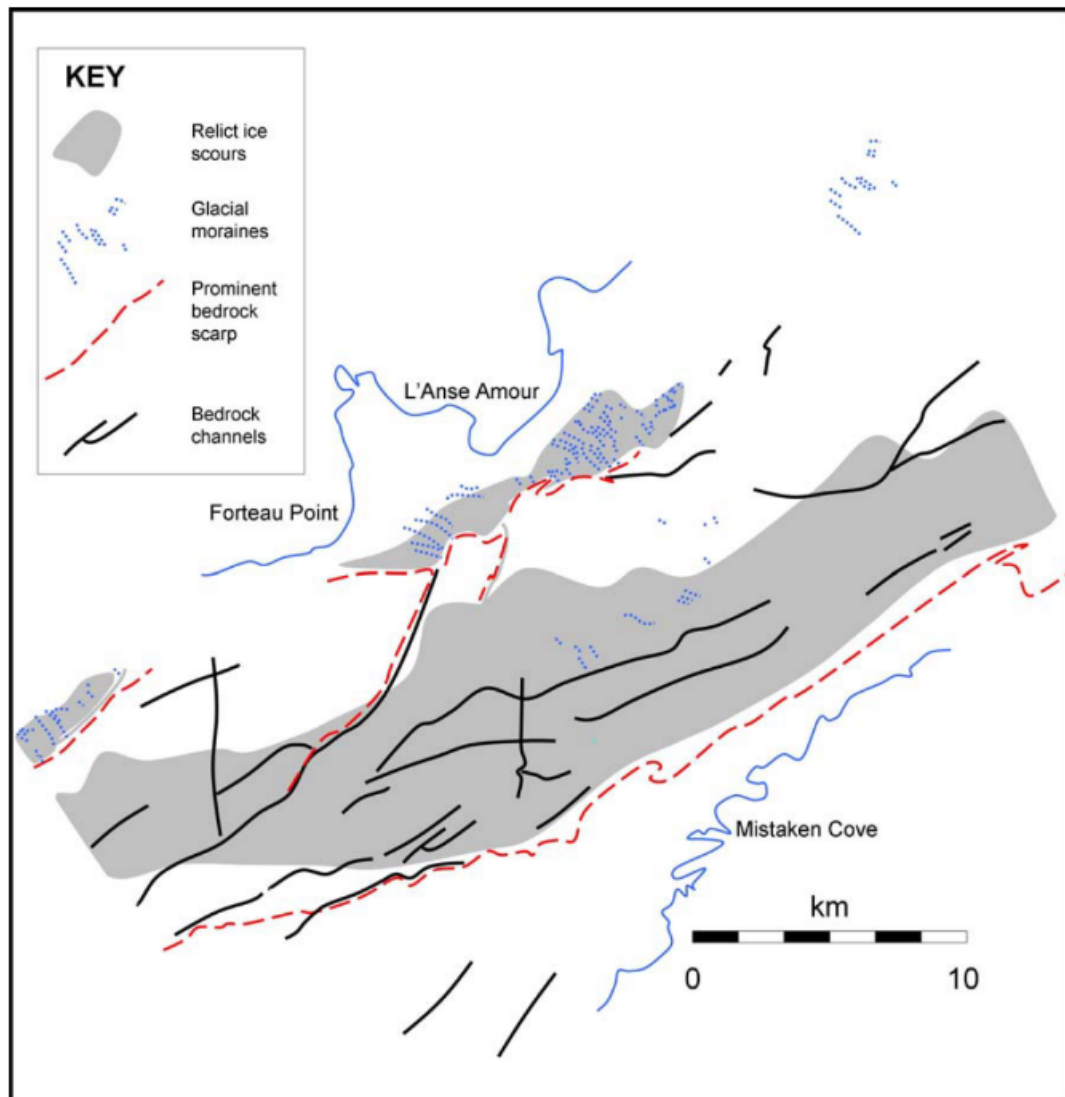


Figure 6-10: Prominent Sea Floor Features in the Strait of Belle Isle [4]

Bedrock Channels and Scarps: A network of bedrock channels was observed on the sea floor which trend approximately northeast-southwest parallel to the Strait [4]. The vertical relief of the channels varied up to 30 m; the channel width ranged from 200 to 500 m. Prominent bedrock scarps were noted off the coastlines on both sides of the Strait, marking

the transitions on the Labrador side from the deep near shore water to the offshore bank and on the Newfoundland side from the deep water of the Central Trough to the near shore area.

Iceberg Scour: Iceberg scour and pitting marks were observed across the Strait; however, scours in greater water depths (below approximately 100 m) were all interpreted as relict. Modern iceberg scour marks were identified only in water depths above 77 m. Iceberg scour occurrence is discussed in greater detail in Section 6.4 above.

Glacial Moraines: Various groups of ridge-like features were identified on the bedrock surface [4] that were interpreted as glacial moraines (accumulations of dirt and rocks that have fallen onto the glacier surface or have been pushed along by the glacier as it moves). The spacing between the ridges is typically approximately 250 m; their length was measured up to 750 m with a trend approximately northwest-southeast, almost perpendicular to the relict iceberg scour marks. The ridge crests were noted to be cut by relict iceberg scours, indicating that the ridges are of older age than the scours. Based on the acoustic signals during the geophysical investigation, the ridges appear to comprise massive, compacted sediments. Based on the data from 2007, some features in greater water depth previously interpreted as ribbed moraines were identified to be iceberg scour berms [4].

Seafloor Mounds: Irregular mounds with relief less than 1 m and ranging in width between 10 and 50 m and in length from 50 to 175 m were identified on the sea floor; these mounds were interpreted as concentrations of cobbles and boulders [20].

Sea Floor Sediment: Based on the survey of various cable crossing routes carried out in 2007, Fugro described the seafloor in the survey region in general as a gravelly to bouldery lag deposit with overlying sand ribbons that trend northeast to southwest [4]. The thickness of the overburden on the tunnel route between Point Amour and Yankee Point was investigated in previous investigations, as reported in [8], and ranged from 2 to 4 m, with some areas of exposed bedrock. A maximum thickness of 12 m was noted in isolated areas. **Figure 6-11** shows the overburden thickness and composition identified from drillholes advanced across the Strait in the 1980s. The locations of these drillholes are shown in **Figure 5-1**.

The 2007 survey by Fugro confirmed that most of the sea floor is covered by a thin sediment layer of 1 to 3 m thickness. Areas directly along the proposed Point Amour – Yankee Point fixed link route revealed a typical overburden thickness of less than 1 m, with some areas of 2 to 2.5 m thickness. An isolated area of 4 to 4.5 m thick sediment was noted.

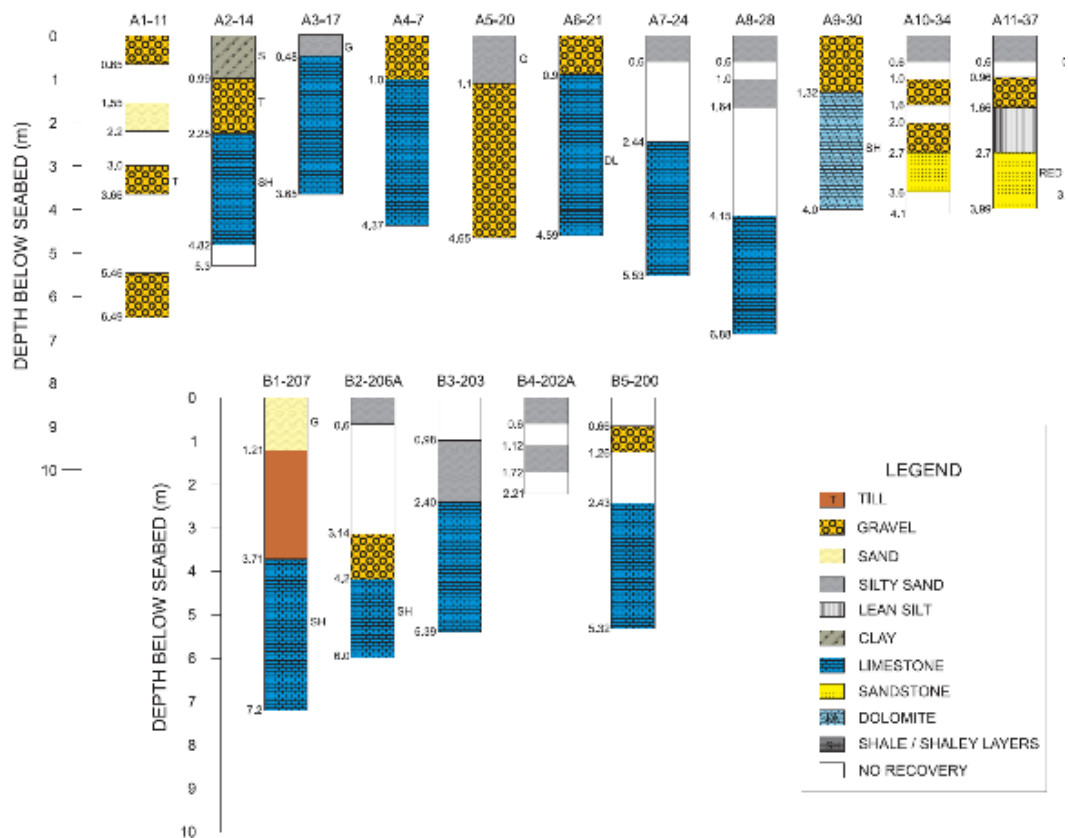


Figure 6-11: Overburden Thickness on Sea Floor identified during 1980s Investigation (included in [4])

6.5.3 Bedrock Stratigraphy

The rock units that are relevant for the project location are the gneisses that form the upper part of the Precambrian basement rock and the overlying formations of Cambrian age. These are described from oldest to youngest in the following.

The Precambrian basement complex comprises mafic and felsic intrusive rocks and gneisses that outcrop along the coast, with granitoid intrusions occurring further inland [8]. These rocks are described as granite or granitic gneiss [9], which typically consist of quartz, oligoclase and potassium feldspar, along with biotite, hornblende and orthopyroxene [17]. Based on the available borehole information, the depth to the top of the Precambrian rock is in the range of about 95 m to 110 m on the Labrador side and at about 460 m near the Newfoundland coast.

An unconformity (defined as a gap in the geologic record, representing a time during where no sediments were deposited or as an erosional surface between rocks of significantly different age) with minor relief of 1-2 m separates the Precambrian basement rocks from the cover rocks of the Bradore Formation [8]. The contact was found to be well defined in the 2009 boreholes. The surface of the unconformity is expected to be irregular but overall gently dipping to the south or southeast.

The Precambrian rocks are overlain by a succession of sedimentary rocks of Cambrian age. These rocks are comprised of sandstone, shale, siltstone and carbonates (limestone and dolomite) that belong to two stratigraphic groups: the Labrador Group and the Port au Port Group [17].

The Labrador Group includes the following formations (oldest to youngest):

- **Bradore Formation:** Dominantly sandstone, but includes minor layers of conglomerate, shale and siltstone. The Bradore Formation was deposited unconformably on the Precambrian surface. The lower section above the unconformity tends to be conglomeratic and includes arkosic sandstones, which yield to cleaner quartz sandstones higher in the formation [17]. Where observed in the available boreholes, the Bradore Formation exhibits a relatively uniform thickness of approximately 100 m along the Labrador coast and slightly thicker at 107 to 112 m along the Newfoundland coast [9]. At the Labrador shoreline and under most of the area of the Strait, the Bradore is overlain by younger sedimentary rocks; however, at the deepest seafloor location in the Strait near the Labrador shoreline, the exposed upper part of the Bradore is eroded and only a partial thickness remains (refer to Drawing H354440-SK-001 in Appendix B).
- **Forteau Formation:** Dominated by limestone, but includes interbedded shale and minor dolomite and sandstone in the basal part of the formation. Based on the boreholes advanced from the Newfoundland coast, the Forteau Formation is approximately 111 m to 117 m thick. At the Labrador coast, the Forteau Formation occurs at the ground surface, and only a partial thickness ranging between 7 m and 28 m was observed [17]. Formations stratigraphically higher than the Forteau Formation are not observed along the Labrador coast.
- **Hawke Bay Formation:** Comprised of an interlayered and interbedded mix of quartzitic sandstone, orthoquartzite, shale and dolomite. The lower part of the formation is dominantly quartzite and sandstone, but it becomes more interbedded with shale and dolomite toward the top of the formation. Where the Hawke Bay is observed in boreholes drilled along the Newfoundland Coast, it ranges in thickness from approximately 160 m to 170 m.

The Port au Port Group includes only two formations in the immediate project vicinity which are observed only along the Newfoundland coast. Both formations are dominated by carbonate rocks (limestone and dolomite). These formations are (oldest to youngest):

- **March Point Formation:** Described by Landsvirkjun [9] as dolomite with thin shaly partings; however, Fugro [8] describes the formation as limestone, dolomitic limestone and dolomite. In the two boreholes where the apparent full thickness of the March Point Formation was observed, the thickness ranges from approximately 31 m to 35 m.
- **Petit Jardin Formation:** Dominantly dolomite but includes intervals of interbedded shales. Since the Petit Jardin Formation occurs at the ground surface at the borehole locations, only a partial thickness of at maximum 28 m was observed [17].

A brief description of each formation is summarized in the following **Table 6-3**, based on the information provided by SNC in 2009 [10] and summarized by Hatch in 2013 [21]. Considering the proposed tunnel alignment, all the formations identified below will be encountered during the tunnel excavation.

Table 6-3: Bedrock Formations at the Strait of Belle Isle

| Age | Group | Formation | Description |
|-------------|----------------|--------------|--|
| Cambrian | Port au Port | Petit Jardin | Light to dark grey dolomite, shale interbeds, shaley dolomite, minor shale, shale partings and laminations |
| | | Marche | Medium to dark grey dolomite, shaley dolomite, shale partings |
| | Upper Labrador | Hawke Bay | Sandstone with shale interbeds, shale with sandstone interbeds, quartzite, orthoquartzite |
| | | Forteau | Grey limestone, shale interbeds, limestone reef, Devils Cove Member comprising limestone and shale at base |
| | | Bradore | Reddish brown sandstone, grey to pink arkose; unconformity at base |
| Precambrian | N/A | N/A | Pink to grey crystalline granitic gneiss |

The bedrock stratigraphy encountered in the deep boreholes advanced in 2009 from both shores of the Strait is shown in the geological cross sections in **Figure 6-12** and **Figure 6-13** that were developed by Landsvirkjun [9] based on the results from the 2009 investigation. No borehole was drilled at Yankee Point during this investigation; however, the boreholes advanced on the opposite side of Savage Cove and at Shoal Cove indicate a reasonable correlation between the 2009 holes and the borehole carried out at Yankee Point in 1974.

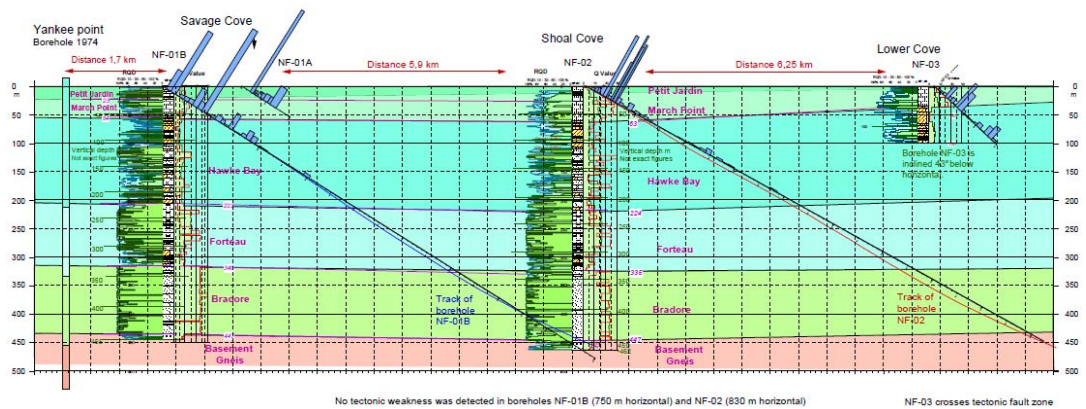


Figure 6-12: Geological Cross-Section between Yankee Point and Lower Cove (Newfoundland), modified from [9]

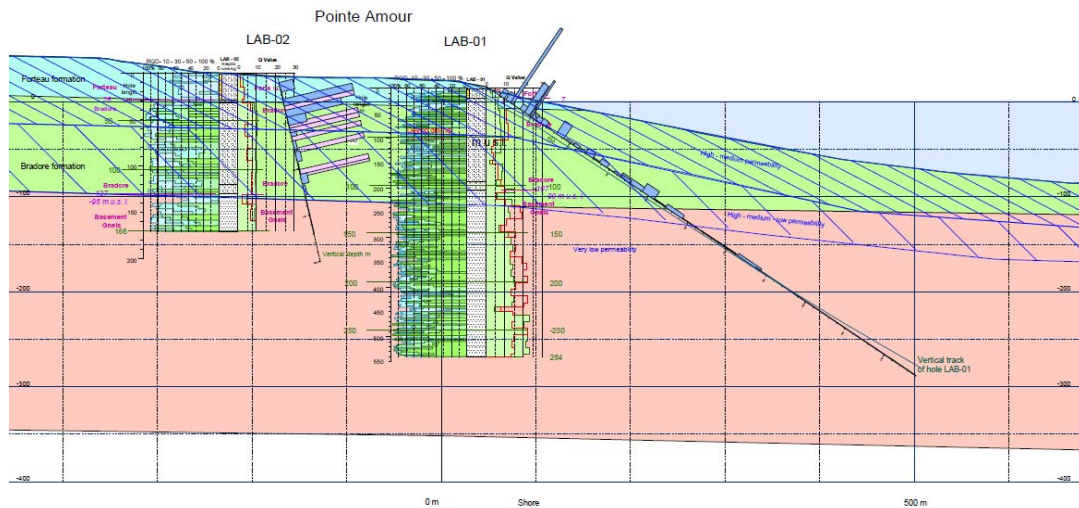


Figure 6-13: Geological Cross-Section at Point Amour (Labrador), modified from [9]

The approximate vertical thickness of each formation in the immediate project area near Yankee Point and Point Amour is summarized in **Table 6-4**, based on the borehole information from 2009. The information for Yankee Point is taken from borehole NF01B at Savage Cove; however, the formation depths identified in borehole NF02 at Shoal Cove are in general accordance with the depth found in borehole NF01B.

Table 6-4: Thickness of Bedrock Formations at Proposed Fixed Link Location

| Formation | Point Amour, Labrador ¹⁾ | | Yankee Point, Newfoundland ²⁾ | |
|--------------------|--|------------------------------------|--|------------------------------------|
| | Depth from Surface (m) | Approximate Vertical Thickness (m) | Depth from Surface (m) | Approximate Vertical Thickness (m) |
| Petit Jardin | Not existent on shore | N/A | 0 to 23 | 23 (partial formation thickness) |
| Marche | Not existent on shore; outcrops offshore | N/A | 23 to 54 | 31 |
| Hawke Bay | Not existent on shore; outcrops offshore | N/A | 54 to 223 | 169 |
| Forteau | 0 to 28 Exists only locally on shore | 28 (partial formation thickness) | 223 to 340 | 117 |
| Bradore | 28 to 96-127 | 68 to 99 | 340 to 447 | 107 |
| Precambrian Gneiss | 96-127 (surface of basement rock) | N/A | 447 (surface of basement rock) | N/A |

1) Borehole LAB 02 (Point Amour) [9]
 2) Borehole NF 01B (Savage Cove) [9]

Updated surface contacts between the geologic formations were provided by Fugro [14] based on the 2009 survey data; AMEC [19] summarized the surface contacts at Point Amour during the investigation in 2011/2012. **Figure 6-14** shows the interpreted offshore boundaries between the different bedrock formations near surface across the Strait of Belle Isle as well as on the Labrador shore.

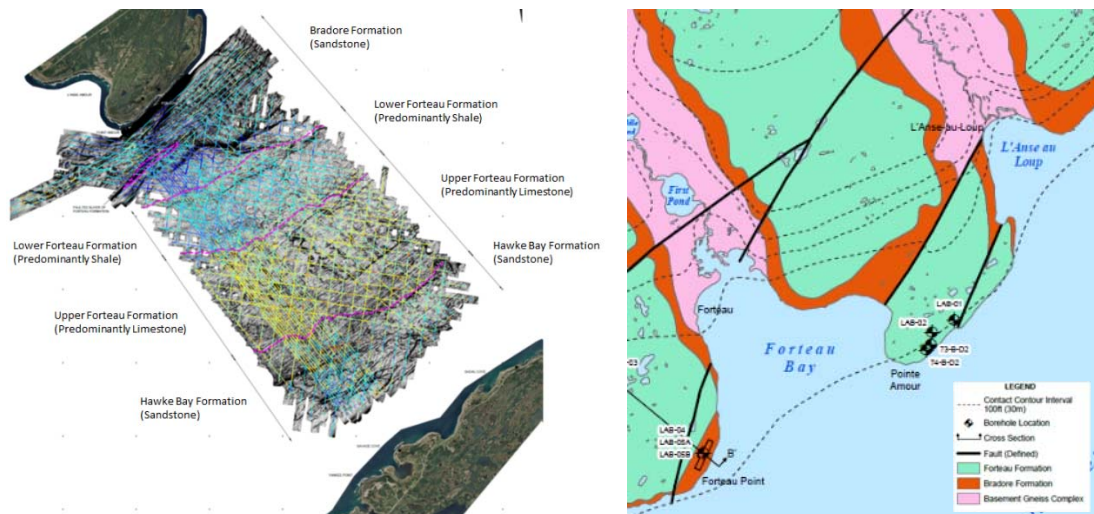


Figure 6-14: Formations Boundaries on Surface; offshore within the Strait [15] (left) and onshore at Point Amour (Labrador) [19] (right)

6.5.4 Geological Faulting

Recent information about geological faulting underneath the Strait of Belle Isle was provided by Fugro in the ‘Descriptive Overview of Regional Bedrock Geology’ based on onshore mapping and a 2007 geophysical survey [8]. **Figure 6-15** shows the inferred locations of faults and formation boundaries in the Strait of Belle Isle that were developed based on mapped onshore faults and stratigraphic boundaries and on offshore structures identified in the 2007 geophysical survey.

Further information was gained during the 2009 investigations that included a drilling investigation on both shores, as described by Landsvirkjun [9], and a geophysical offshore survey, as discussed by Fugro in [14].

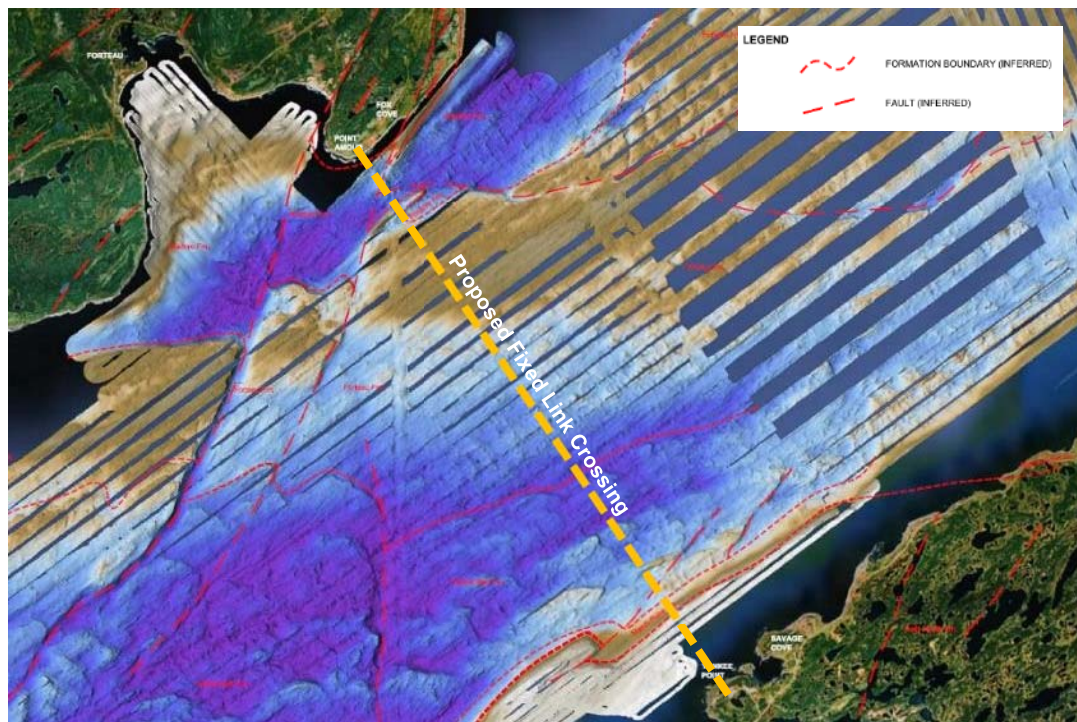


Figure 6-15: Inferred Faults, modified from [8]

Although various faults and tectonic lineaments are visible onshore (examples of faults visible on surface are shown in **Figure 6-16**), no major faults or tectonized zones were identified in the boreholes drilled during the 2009 geotechnical investigation [9]. The potential fault zones that were identified in the boreholes included relatively small areas of fractured rock or lost core.

On the Newfoundland side, some sections of lost core occurred in a borehole near Lower Cove which were identified as a potential fault zone. On the Labrador shore, a zone of jointed rock and lost core at approximately 60 m vertical depth in a borehole near Fox Cove located north-east of Point Amour (see **Figure 6-17**) was interpreted as having experienced tectonic action and associated degrading due to shearing and weathering [18].

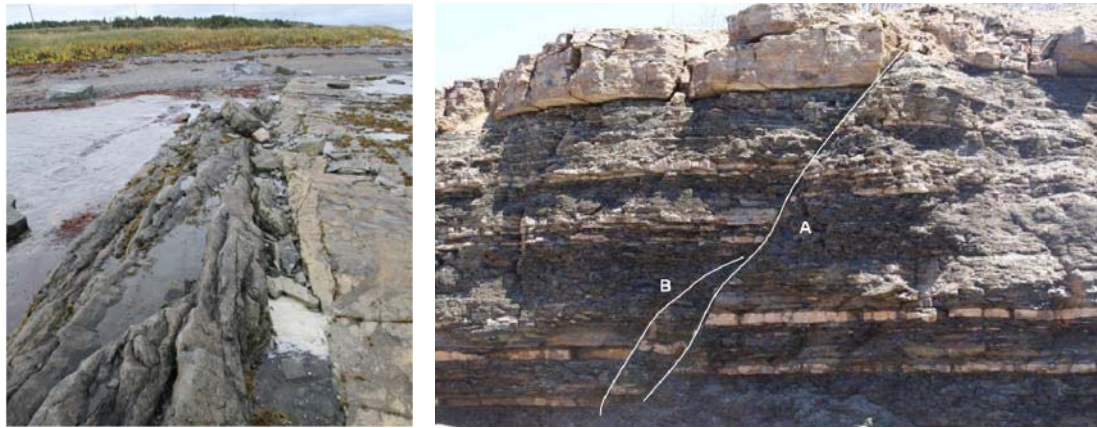


Figure 6-16: Faults visible on Surface: Tectonic Lineament near Lower Cove, NL (left) [9]; Fault Zone in Forteau Formation near L'Anse au Loup, LAB (right) [14]

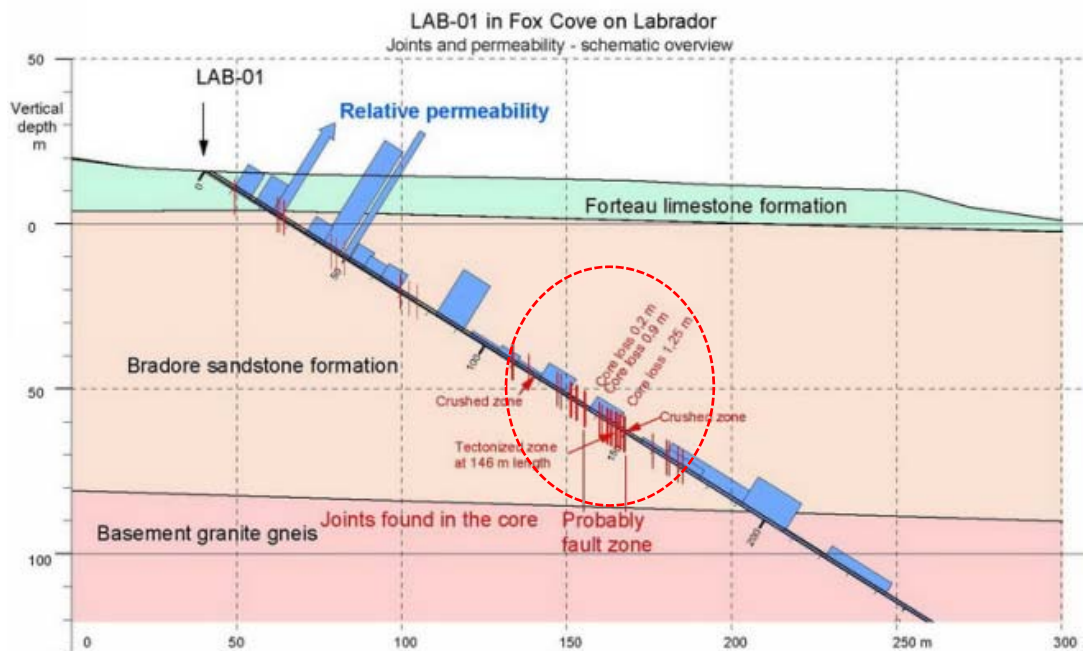


Figure 6-17: Potential Fault Zone identified in Borehole LAB-01 near Fox Cove on Labrador Shore [9]

Based on the 2009 geophysical survey results, Fugro provided a map showing interpreted bedrock outcrops and fault locations across the Strait of Belle Isle, as shown in **Figure 6-18**. The majority of the potential fault zones were identified towards both shores, mainly in the Bradore and Hawke Bay Formations. While the data provides an overview of the fault locations, it does not provide information about the width or characteristics of the material or fault gouge within the faults.

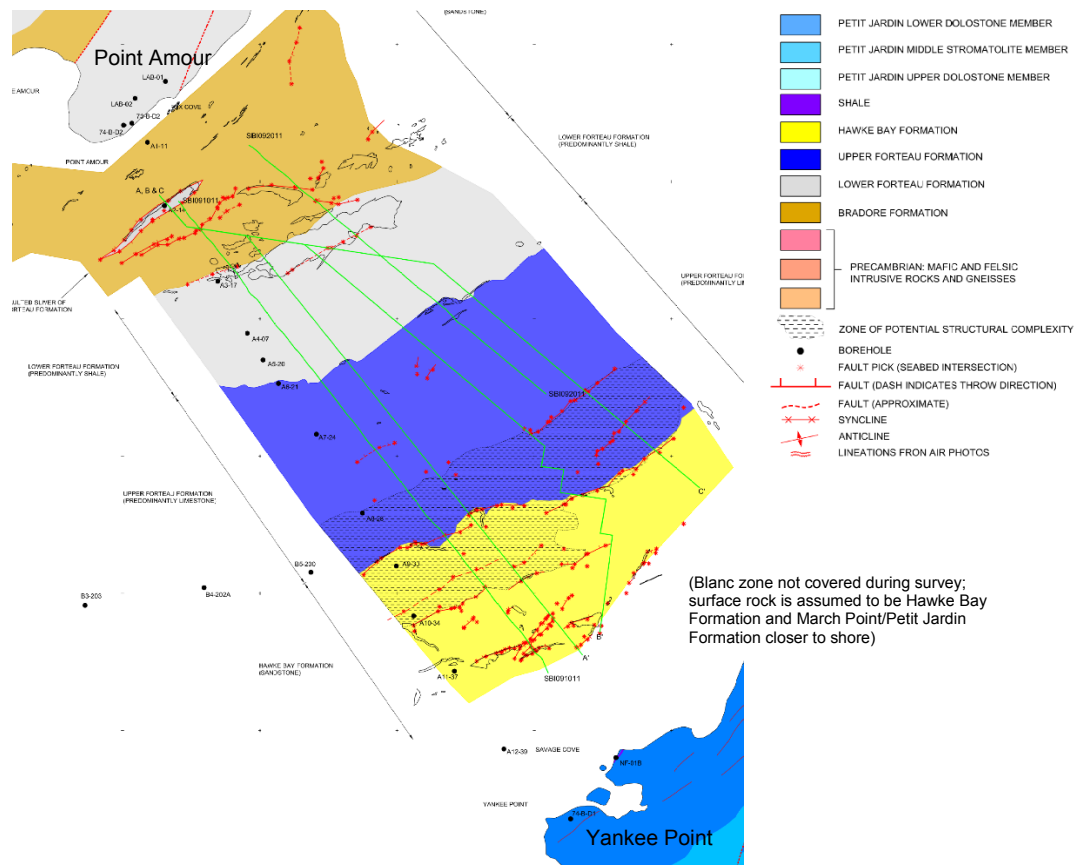


Figure 6-18: Interpreted Fault Zones across the Strait of Belle Isle, modified from [15]

6.6 Seismicity

According to recent information from Natural Resources Canada, the Strait of Belle Isle lies in a region of low risk regarding seismic hazards [31]; hence, seismic events are not considered a significant risk for the Strait of Belle Isle fixed link options. Figure 6-19 shows the hazard rating for the project area.

Mean seismic hazard values (Table 6-5) were determined for the project site using the seismic hazard calculator from Natural Resources Canada. The values were provided for a 2% in 50 years probability of exceedance. Spectral ($S_a(T)$, where T is in seconds) and peak ground acceleration (PGA) values are given in units of g (9.81 m/s^2). Peak ground velocity is given in m/s.

Table 6-5: Mean Seismic Hazard Values at Proposed Fixed Link Location

| $S_a(0.05)$ | $S_a(0.1)$ | $S_a(0.2)$ | $S_a(0.3)$ | $S_a(0.5)$ | $S_a(1.0)$ | $S_a(2.0)$ | $S_a(5.0)$ | $S_a(10.0)$ | PGA | PGV |
|-------------|------------|------------|------------|------------|------------|------------|------------|-------------|-------|-------|
| 0.046 | 0.066 | 0.068 | 0.061 | 0.054 | 0.036 | 0.020 | 0.0049 | 0.0022 | 0.038 | 0.044 |

Note: Values are for 'firm ground' (NBC 2015 Soil Class C, average V_{s30} shear wave velocity 450 m/s)

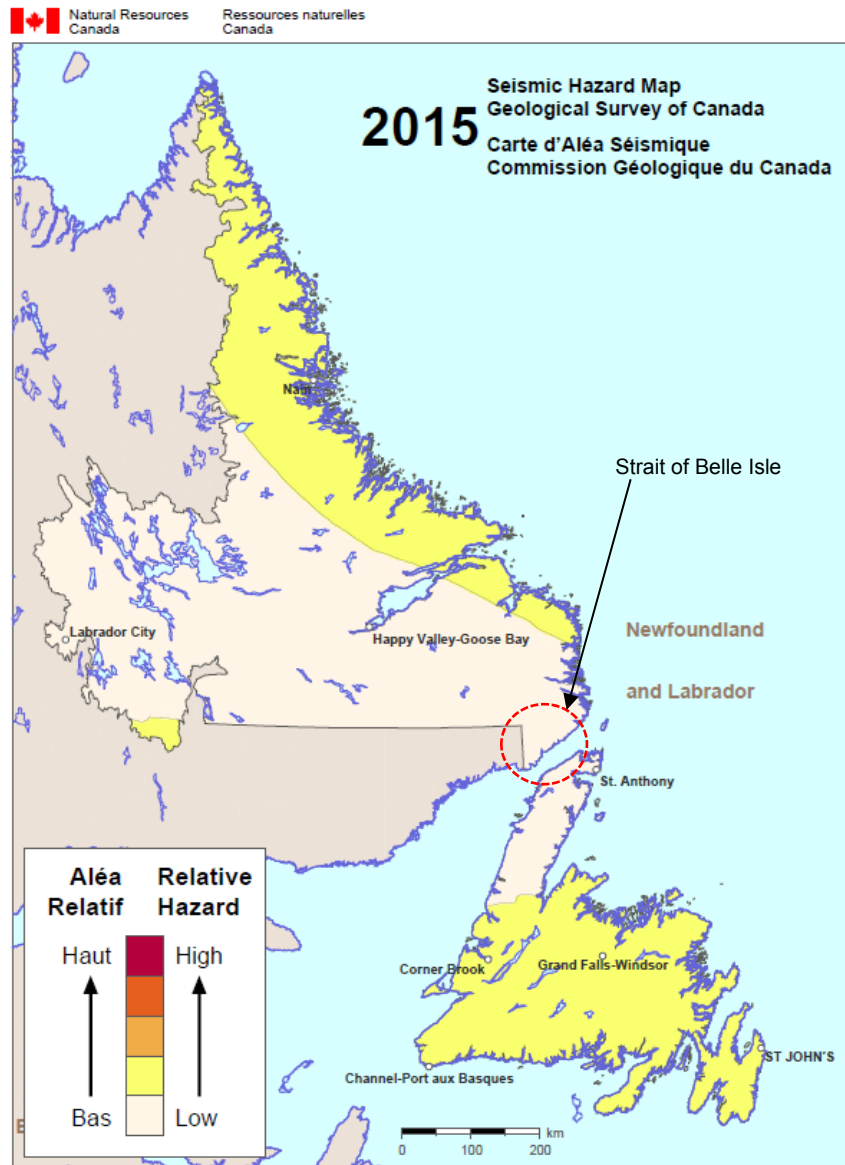


Figure 6-19: Seismic Hazard Map 2015 [31]

6.7 Geotechnical Properties

Field and laboratory testing were carried out as part of the investigations in 2009 to determine geotechnical characteristics of the rock. The results are provided in [19], [12] and [13] and are summarized in [21]. The tested rock samples are taken in the general project area and are not specific to the immediate project area; however, the results provide a general understanding on the rock properties in the project area.

6.7.1 Rock Quality

The rock quality derived from the Rock Quality Designation (RQD) index, an index used to describe the quality of rock by evaluating the length of the intact rock core retrieved from boreholes. The rock quality on both shores at the proposed project location, based on RQD values provided by Landsvirkjun [9] and SNC [10], is summarized by formation in **Table 6-6**.

The rock quality in potential fault zones that were encountered in the boreholes during the 2009 investigation was described typically as very poor to poor quality.

Table 6-6: Quality of the Rock Mass by Formation

| Formation | Point Amour, Labrador ¹⁾ | | Yankee Point, Newfoundland ²⁾ | |
|---------------|-------------------------------------|---|--|--|
| | Vertical Depth (m) | Rock Quality (RQD) | Vertical Depth (m) | Rock Quality (RQD) |
| Petit Jardin | N/A | Not existent on Labrador shore | 0 to 23 | RQD 20 to 100 (very poor to excellent) Average indicates fair quality |
| March Point | N/A | Not existent on Labrador shore | 23 to 50 | RQD 50 to 98 (fair to excellent) Average indicates fair to good quality |
| Hawke Bay | N/A | Not existent on Labrador shore | 50 to 223 | RQD 0 to 100 (very poor to excellent) Average indicates fair to good quality RQD 0 to 50 (very poor or poor) mostly in upper 50 m of formation and in shale throughout the borehole. |
| Forteau | 0 to 33 | RQD 15 to 65 (very poor to fair) Average indicates poor quality | 223 to 340 | RQD 0 to 100 (very poor to excellent) Average indicates good quality RQD 0 to 50 (poor to very poor) mainly in lower part of formation; typically related to shale |
| Bradore | 33 to 123 | RQD 60 to 100 (fair to excellent) Average indicates good quality | 340 to 447 | RQD 80 to 100 (good to excellent) Average indicates excellent quality |
| Basement Rock | 123 to 163 | RQD 43 to 95 (poor to excellent) Average indicates fair to good quality Poor quality related to top portion of basement rock (contact with Bradore) (borehole advanced approx. 40 m into basement rock) | 447 to approx. 477 | RQD 60 to 90 (fair to excellent) Average indicates fair to good quality (borehole advanced approx. 30 m into basement rock) |

¹⁾ Borehole NF 01B [9]

²⁾ Borehole LAB 02 [9]

6.7.2 Rock Mass Classification

An assessment and classification of the rock mass has to be carried out to define the requirements for initial rock support during tunnel excavation, which applies in particular to Drill & Blast or mechanically excavated tunnels. Landsvirkjun [9] used the Q-System, a rating system based on various rock parameters. The chart used for the evaluation of the rock on both shores is shown in **Figure 6-20**. An overall Q range between 4 and 25 was determined for the core rock, indicating poor to good rock quality, with an average between 8 and 12, corresponding to fair quality. To account for adverse conditions such as effects of blasting and water inflow, Landsvirkjun reduced the rock quality by 50%, resulting in Q values between 2 and 12, corresponding to poor to fair quality. Based on these Q-values and assuming a tunnel diameter of 10 m (with Excavation Support Ratio (ESR) of 1.0 for major tunnels), initial rock support with shotcrete and rock bolts would be generally sufficient; however, additional support will most likely be required in areas of poor rock conditions, such as weak shale, fractured rock, fault zones, etc. Pre-excavation grouting ahead of the face will most likely be required to reduce the permeability of the rock and control water inflow into the tunnel.

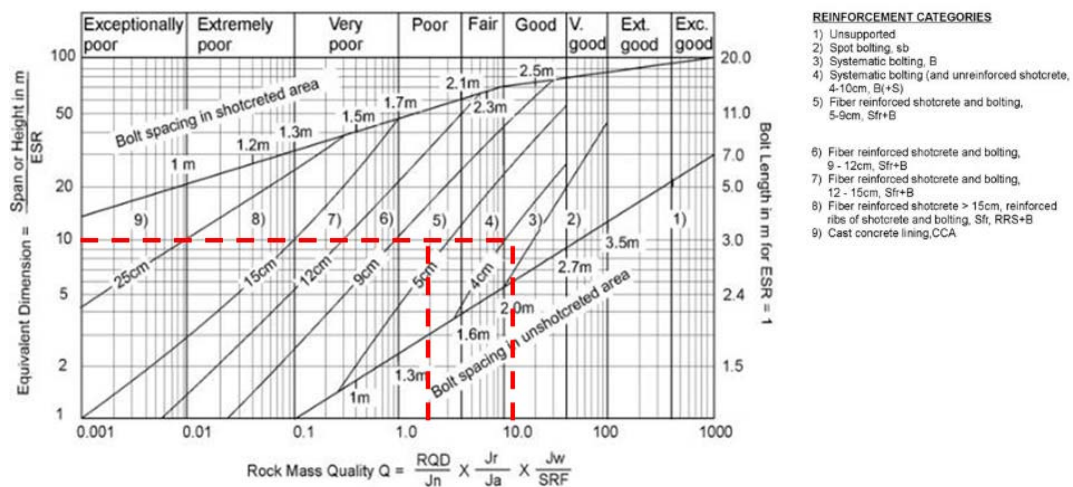


Figure 6-20: Q-System: Rock Mass Quality and Rock Support Chart

6.7.3 Unconfined Compressive Strength (UCS)

UCS tests were carried out on 12 sedimentary rock core samples of the Hawke, Forteau and Bradore Formations, taken on both the Labrador and Newfoundland side (refer to [12]). The results are summarized in **Table 6-7**. The intact rock strength was determined to be typically between 76 MPa and 178 MPa, indicating strong to very strong rock. Three results showed low values between 1.9 MPa (Hawke Bay Formation) and 16.3 MPa (Forteau Formation), indicating very weak to weak rock. The rock type of the sample was not provided; however, low strength results are assumed to be related to rock samples containing a plane of weakness or larger amounts of shale. This assumption is supported by the results of further

strength testing (Point Load Tests) that typically indicate reduced strengths for shale or shale containing samples.

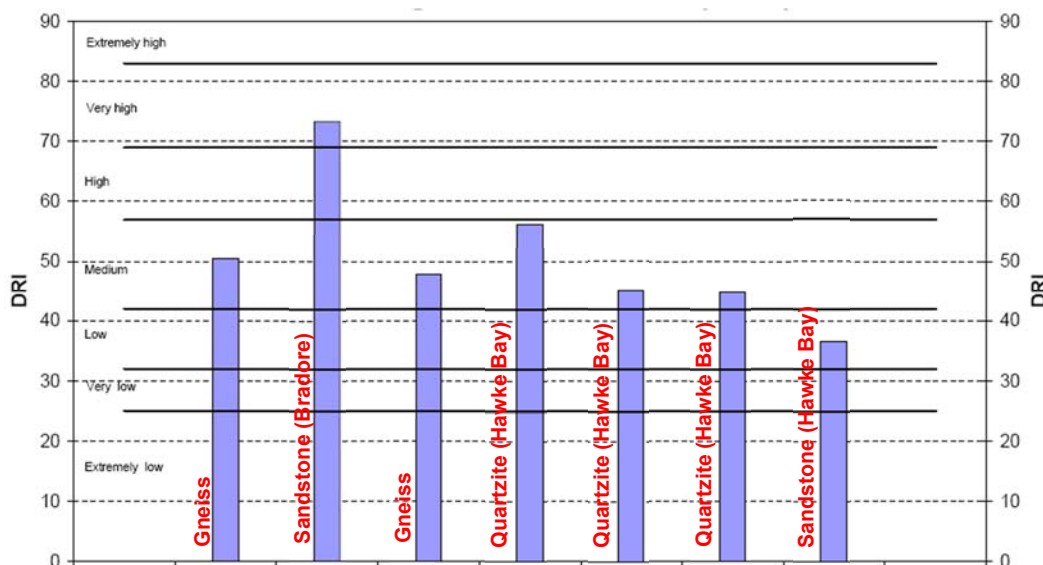
Table 6-7: Unconfined Compressive Strength of the Intact Rock by Formation

| Formation | Newfoundland | | Labrador | |
|-----------|--------------|---------------|-----------|--------------|
| | UCS (MPa) | Depth (m) | UCS (MPa) | Depth (m) |
| Hawke Bay | 1.9 | 281.6 – 281.8 | -- | -- |
| | 171 | 155.1 – 156.0 | | |
| | 178 | 403.4 – 403.8 | | |
| Forteau | 5.5 | 458.4 – 459.1 | 148 | 17.5 – 17.95 |
| | 142 | 461.4 – 462.1 | | |
| | 16.3 | 514.7 – 515.6 | | |
| Bradore | 108 | 785.7 – 786.5 | 138 | 36.8 – 37.4 |
| | | | 125 | 50.3 – 51.05 |
| | | | 76 | 46.1 – 46.5 |
| | | | 103 | 60.4 – 61.0 |

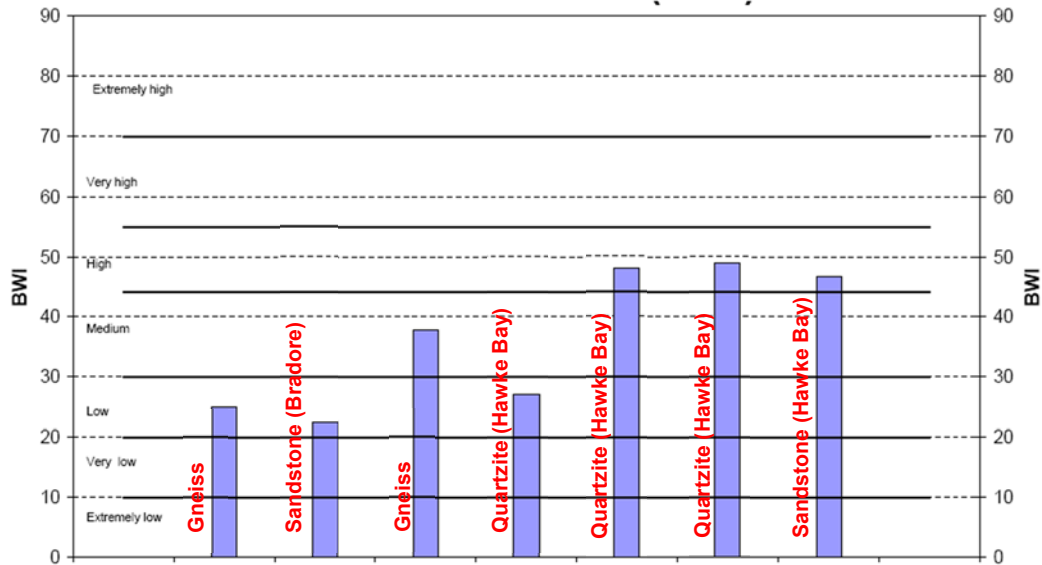
6.7.4 Drillability Properties

Laboratory tests to determine drillability properties of the rock (including Drilling Rate Index DRI, Bit Wear Index BWI and Cutter Life Index CLI) were carried out on seven rock core samples, including samples of gneiss, sandstone and quartzite (refer to [12]). The test results indicate the following:

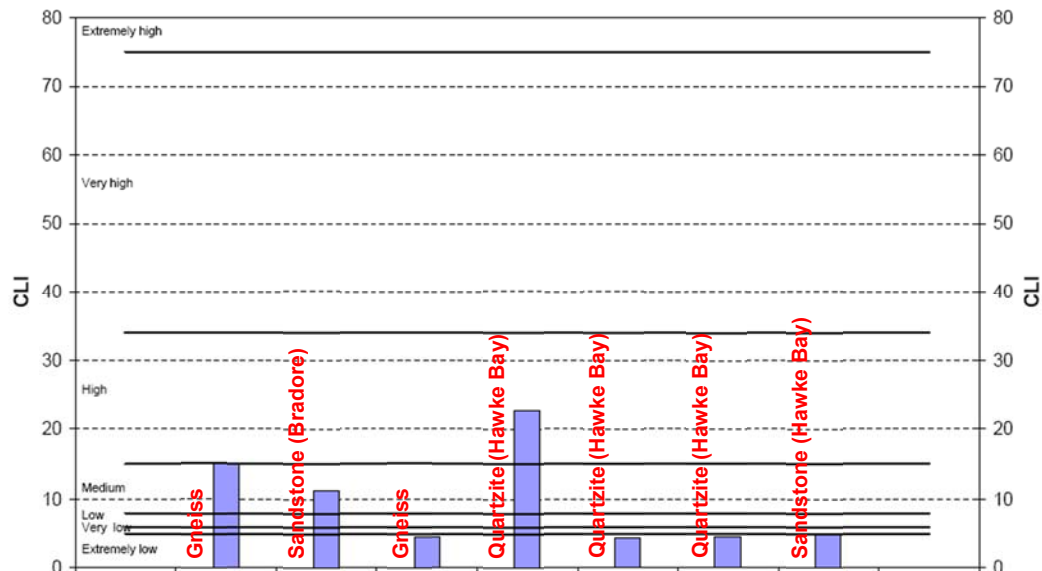
- The DRI values for the rock samples were low (sandstone, Hawke Bay Formation) to very high (sandstone, Bradore Formation) with most samples classified as medium.



- The BWI is used to estimate the lifetime of drill bits. The values for the rock samples varied uniformly; three were classified as low and three as high. The value for one sample was medium (basement gneiss).



- CLI expresses the lifetime of TBM disc cutter steel rings. Four samples showed extremely low or very low values. Three sample were classified as medium to high (quartzite, Hawke Bay Formation).



6.7.5 Abrasiveness

The abrasiveness of the rock is an important factor to evaluate the expected TBM disc cutter wear and influences the choice of the appropriate cutters. Laboratory tests to determine the abrasiveness (Cerchar Abrasivity Index) of the rock were carried out on eight sedimentary

rock core samples, including dolomite, sandstone and quartzite from the Petit Jardin, March Point and Hawke Bay Formations. The test results indicate abrasivity indices ranging between abrasive (index 1.0 - 2.0) to extremely abrasive (index 4.0 - 6.0), with the lowest abrasiveness at 1.5 (sandstone, Hawke Bay Formation) and the highest at 5.7 (quartzite, Hawke Bay Formation). The dolomite samples from the Petit Jardin and March Point Formations were abrasive to highly abrasive, corresponding to indices of 1.9 and 2.2.

6.7.6 Time-Dependent Deformations (Swelling)

The time-dependent behavior of the rock will influence the choice of TBM and the choice of tunnel lining. Free swell tests were carried out on six shale samples taken from the cored rock (refer to [13]). The samples were taken on the Labrador side from rock core of the Hawke Bay and Forteau Formations. The results of the preliminary laboratory investigation indicate that a strong potential for swelling in the shale under investigation exists. In summary, the results showed the following:

- The rock samples show swelling behaviour (i.e. time-dependent deformation) in all directions.
- There is no evidence of significant amounts of swelling clay minerals in the samples.
- The existence of a significant salt concentration in the rock and relatively low calcite content provide a mechanism for swelling. Two samples with high calcite content showed very little swelling.

6.7.7 Permeability

As part of the geotechnical investigations in 2009 and 2011/2012, hydraulic conductivity testing was carried out in boreholes on both sides of the Strait to investigate the permeability of the various rock formations. The test results from the 2009 investigation as provided in [9] are illustrated in the geological cross sections in **Figure 6-21** and **Figure 6-22**.

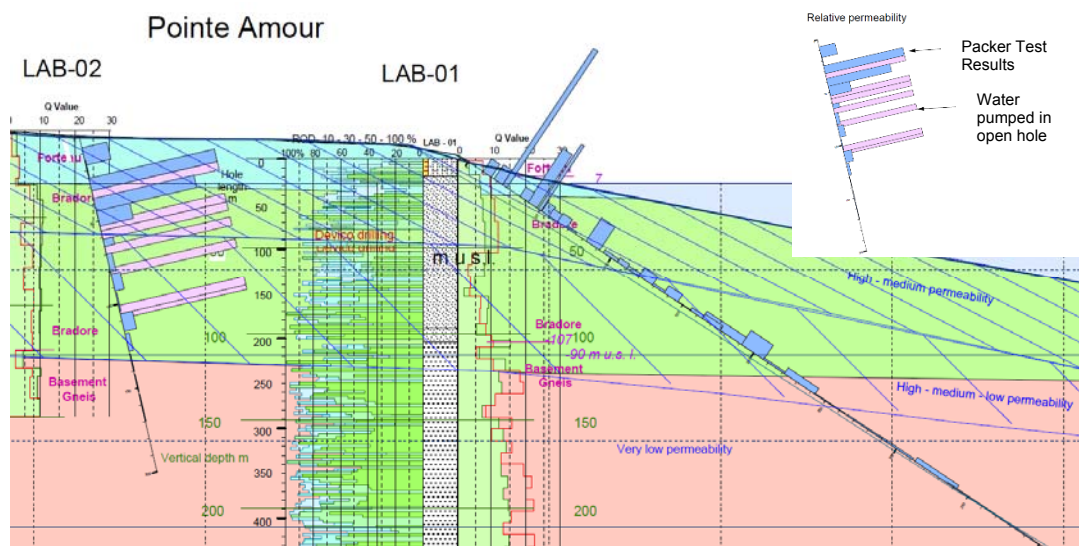


Figure 6-21: Permeability Test Results – Point Amour [9]

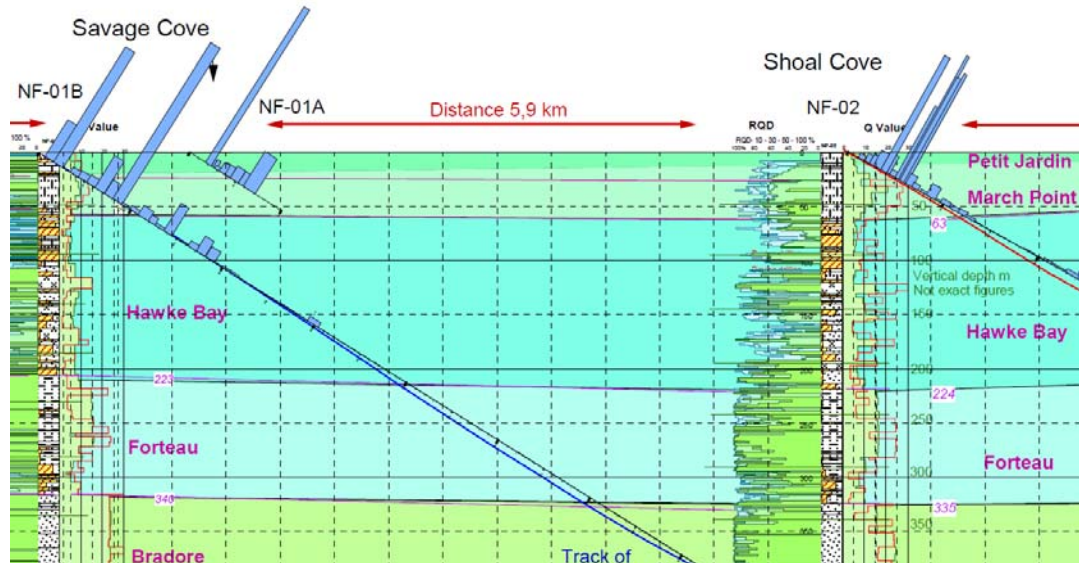


Figure 6-22: Permeability Test Results – near Yankee Point [9]

The test results were analyzed by Landsvirkjun [9] and reviewed and summarized by HMM [17]. Based on the analysis, three zones of varying rock mass permeability were identified, related to ranges of depth below the ground surface. The zone from ground surface to approximately 50 m depth was identified as the zone with high permeability. The zone ranging in depth approximately from 50 m to 100 m was found to have typically a medium permeability. The permeability of the rock in depths greater than approximately 100 m was identified as medium to low. It was found that the permeability decreases rapidly with depth and was identified generally as very low or impermeable below depths of 150 m [9]. The ranges of permeability associated with the three zones was summarized by HMM [17], as shown in **Table 6-8**.

Table 6-8: Summary of Rock Mass Permeability by Range of Depth (after [17])

| Depth below Ground Surface (m) | K (m/s)* | | | |
|--------------------------------|------------------|------------------|------------------|------------------|
| | 10 ⁻⁸ | 10 ⁻⁷ | 10 ⁻⁶ | 10 ⁻⁵ |
| 0 to 50 | 1% | 22% | 57% | 2% |
| 50 to 100 | 18% | 48% | 34% | 0% |
| > 100 | 41% | 48% | 11% | 0% |
| Fault | 0% | 0% | 50% | 50% |

* K values are presented as order of magnitude ranges based on the packer test results provided in [9] as Lugeon values

Although the permeability ranges associated with the three zones overlap, it was found that the permeability of the rock mass decreases with increasing depth. Considering that the rock formations dip towards the Newfoundland shore and that some formations, such as the Forteau and Bradore Formation, occur near surface on the Labrador shore as well as in greater depth on the Newfoundland shore, it can be concluded that the zones of varying permeability are related to depth below surface rather than to the rock formations.

Fault zones that occur throughout all rock formations underneath the Strait of Belle Isle typically comprise zones of more intensely fractured rock and as a result tend to have a higher permeability than the surrounding rock mass.

Permeability tests were also carried out in 2011/2012 in boreholes on the Labrador side of the Strait [19]. The tests were carried out in the Bradore Formation in depths of approximately 20 to 80 m below the ground surface. The test results indicated hydraulic conductivity values between 5×10^{-8} and 2×10^{-6} m/sec, corresponding to a very low to moderate permeability. The results of tests at the contact between Bradore Formation and basement rock suggest a mean hydraulic conductivity of 2×10^{-7} m/s, corresponding to low permeability of the rock mass.

7. Design Criteria and Construction Considerations for a Tunnel Fixed Link

7.1 Environmental Design Considerations

Based on the data provided above, the updated design criteria associated with the environmental parameters of the study area are summarized in the following **Table 7-1**.

Table 7-1: Environmental Design Considerations

| Climate | | |
|--|---------------|---------------------------------|
| Minimum design air temperature | | -30°C |
| Maximum hourly wind speed | | 100 km/h |
| Maximum gust wind speed | | 148 km/h |
| Design hourly wind pressure | | 0.87 kPa |
| Atmospheric Icing | | 11.5 cm with 115 km/h wind gust |
| Ground snow loading | | 6.1 kPa |
| Freezing Degree Days Below 0°C | | 1323 |
| Oceanography | | |
| Currents | at 15 m depth | 3.6 m/sec |
| | at 50 m depth | 2.5 m/sec |
| Maximum Wave Height (100-year return period) | | 10 m |
| Sea Ice | | |
| Uniform ice floe thickness | | 0.6 m |
| Ice compressive strength (uniform ice) | | 2 MPa |
| Icebergs | | |
| Mass | | 1 million tonnes |
| Speed | | 1 m/sec |
| Ice strength | | 5 MPa |
| Scour depth (1000-year return period) | | 5.5 m |

7.2 Alignment Location and Depth

The investigated fixed link alignment between Yankee Point and Point Amour would cross the HVDC cable crossings that were installed in HDD tunnels on shore and covered with a rock berm on the sea floor between Forteau Point, Labrador (approximately 7 km west of Point Amour) and Shoal Cove, Labrador (approximately 5.5 km east of Yankee Point). The cable location and the proposed fixed link alignment are shown in **Figure 7-1**. Considering a subsurface fixed link (TBM or drill & blast tunnel), the proposed tunnel alignment would not affect the existing cable crossing. The tunnel portal locations as well as the onshore portion of the tunnel would be in a distance larger than 5 km away from the HDD cable conduits on shore and the HDD exits under water and no interference between the new tunnel and the existing structures are expected.

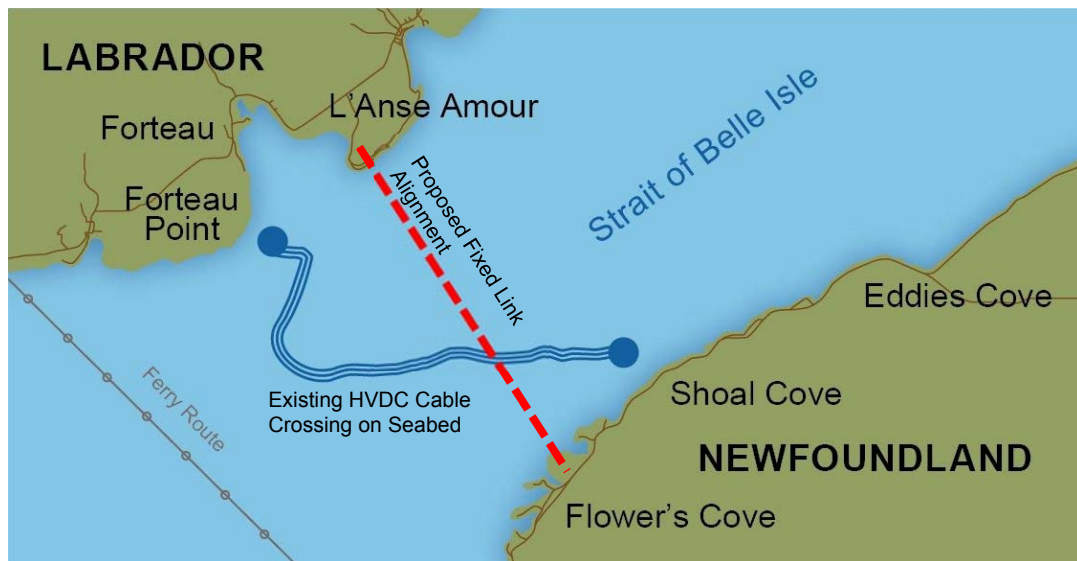


Figure 7-1: Subsea Cable crossing Fixed Link Alignment

Considering surface fixed link options, including bridge, causeway with bridges and ITT, these options would be impacted by the existing HVDC cable crossing. The installation of bridge foundations would have to consider the location of the cable conduits on the sea floor; however, considering the wide span of 2 km between the individual foundations, the cable crossing is not expected to prevent the installation of bridge foundations. The construction of a causeway would have to either consider the load impact from the rock berm on the existing cable conduits or strategically place the causeway openings spanned with bridges above the cable conduits. The installation of an ITT in a trench on the sea floor would be impacted by the existing cable crossing since dredging of a trench would not be feasible at the intersection with the surface cable conduit. ITT construction would require an alignment change to prevent an alignment crossing. This would involve either moving the entire ITT alignment or replacing the entry locations on either the Labrador side or the Newfoundland side. All alignment changes would result in an increased length of the ITT fixed link.

The required depth of the tunnel alignment below ground surface depends on the geotechnical conditions underneath the Strait as well as the excavation method. The poor ground conditions that are expected locally due to fault zones and the likelihood of highly permeable rock in the upper 50 m of rock must be considered when defining the required depth.

Information available from sub-sea tunnels excavated in Norway between 1980 and 2009 using drill & blast methods indicate that typical rock covers ranged between 20 and 50 m. Experience from the Oslofjord sub-sea tunnel (completed in 2000) with low rock covers and poor ground conditions, which required extensive rock reinforcement including ground freezing operations, led to the requirement of greater rock covers. Based on the sub-sea tunneling practices in Norway as described in [25] and assuming a maximum water depth of 110 m in the Strait of Belle Isle, a rock cover of at least 50 m should be considered for a tunnel excavation using drilling and blasting. This cover thickness would avoid the area of

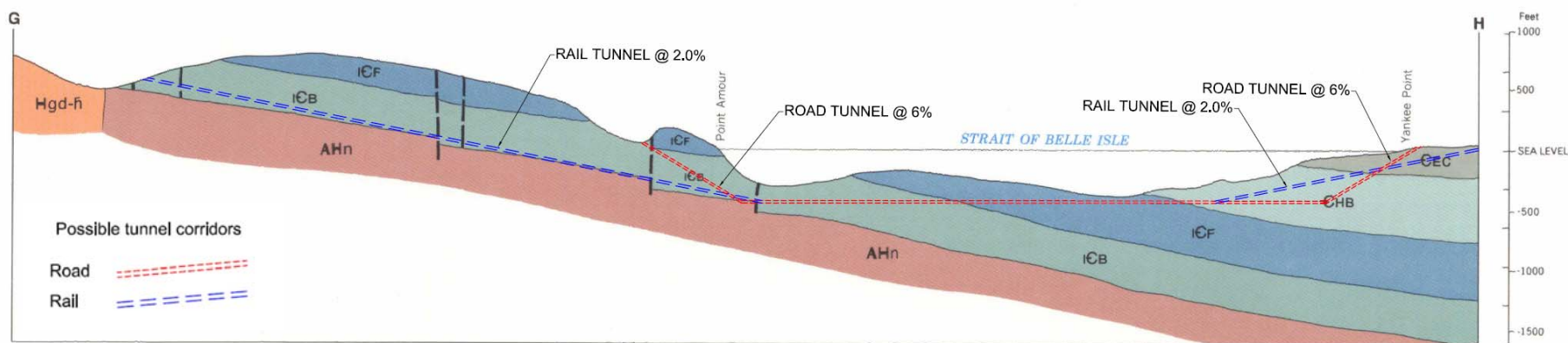
highest permeability on the upper 50 m of rock. A bored tunnel typically requires less coverage than a drill & blast tunnel. Considering the current knowledge of the ground conditions and to prevent critical situations in the tunnel due to rockfalls and water inflow, a rock cover of approximately two tunnel diameters is recommended for the tunnel excavation using a TBM. However, alignment depths can be adjusted when more information about the subsurface conditions become available from recommended future investigations in the Strait.

An alignment depth of approximately 130 m below sea level would have to be considered, based on the assumed deepest water in the Central Trough of approximately 110 m. However, the inflow rates at this elevation should still be expected to be significant and measures must be in place during excavation to address high water inflows. Hydraulic pressures of up to at least 13 bar should be taken into account for the selection of the tunnelling machine at this depth, assuming that the tunnel will be connected to the water body in the Strait due to fault zones and/or open joints.

Assuming that the up to 100 m of high to medium permeable ground encountered in the boreholes on shore also exist below the Strait, the avoidance of the entire high to medium permeability zone would result in a tunnel depth of approximately 200 m below sea level.

The depth of the alignment influences the length of the tunnel. A deeper alignment would be accompanied by a significant increase in tunnel length due to the extended access ramps and, consequently, by increased construction costs. Considering drill & blast excavation, however, parts of these costs will likely be offset by the reduced risk of encountering poor ground and high water inflow and hence reduced mitigation efforts.

A geological profile across the Strait of Belle Isle is provided in **Figure 7-2** and on Drawing 1 in Appendix B that shows the suggested TBM tunnel alignment at an approximate depth of 22 m (two tunnel diameters, based on the TBM road tunnel diameter) below the seabed. It has to be noted that the currently available information about the permeability of the rock mass is restricted to the immediate shore area. Additional investigations would be required to verify the rock mass permeability below the Strait prior to defining the most suitable tunnel alignment depth of the Strait of Belle Isle Fixed Link. However, the assumption that full hydrostatic pressure occurs at the TBM excavation face reduces the sensitivity to the permeability of the rock mass.



APHEBIAN AND/OR HELIKIAN
 BASEMENT GNEISS COMPLEX*

| | |
|--------|---|
| AHn | Leucocratic to melanocratic gneiss; some quartz-rich gneiss, pelitic gneiss and schist and amphibolite; minor calc-silicate gneiss |
| AHng | Chiefly leucocratic (quartz-microcline-plagioclase) gneiss, colour index 0 to 5 |
| AHn-b | Chiefly mesocratic (biotite-quartz-microcline-plagioclase) gneiss, colour index 5 to 15 (on Belle Isle includes abundant dykes of the Long Range swarm) |
| AHn-bh | Chiefly melanocratic (+ hornblende-biotite-quartz + microcline-plagioclase) gneiss, colour index > 15 |
| AHn-q | Quartz-rich leucocratic and mesocratic gneiss, quartzite |
| AHn-s | Pelitic gneiss and schist (chiefly sillimanite-microcline-cordierite-bearing rock and retrograde equivalents) |
| AHnql | Leucocratic and mesocratic gneiss undivided |

* (units not arranged in order of intrusion)

CAMBRIAN
 MIDDLE CAMBRIAN AND (?) UPPER CAMBRIAN

| | |
|-----|--|
| CEC | EDDIES COVE FORMATION: light to dark grey dolomite, limestone and interbedded grey shale |
|-----|--|

HADRYNIAN (?) AND CAMBRIAN
 LABRADOR GROUP
 LOWER CAMBRIAN

| | |
|-----|---|
| CHB | HAWKE BAY FORMATION: white orthoquartzite |
| iCF | FORTEAU FORMATION: red to grey nodular limestone and grey shale; iCF1 includes basal unit at Canada Bay of purple and white limestone of the Devils Cove Formation; iCF2 includes upper unit on Belle Isle of white-weathering sandstone, siltstone, chert and grey fragmental limestone of the White Point Formation |

LOWER CAMBRIAN AND (?) HADRYNIAN

| | |
|-----|--|
| iCB | BRADORE FORMATION: red to pale pink and maroon arkosic sandstone, minor conglomerate |
|-----|--|

Figure 7-2: Profile across the Strait of Belle Isle with proposed Tunnel Alignment

Due to the increased depth of the tunnel alignment, the exit ramps for both the rail and the road tunnel will increase in length. The exit grades of the proposed rail tunnel were slightly modified to 2% (slightly steeper than the 1.66% suggested in the 2004 study) to reduce the length of the exit ramps but still be feasible for modern electric shuttle trains. The exit grades for the road tunnel were set to 6% (in agreement with the 2004 study).

On the Labrador side, the length of the rail tunnel exit ramp extends significantly depending on the surface elevations, as can be seen in **Figure 7-2**. However, the ramp, as shown, extends along a tunnel with a straight horizontal alignment. The design of a curved ramp that follows a more favorable surface profile will most likely allow a significant reduction in length. Steeper exit grades for the rail tunnel to further shorten the ramps could be achieved by custom made rail shuttle locomotives.

7.3 Tunnel Usage

In the 2004 Pre-Feasibility Study, the tunnel cross-sections included space for the integration of three HVDC cables. The cables were assumed to run through the tunnel for electricity transfer from Labrador to Newfoundland as part of the Nalcor Energy Lower Churchill project and would provide a means of additional revenue. In the past years, these cables have been installed in cable conduits across the Strait; however, the tunnel cross sections still provide space that could be utilized for utilities should the need arise during the design stage.

7.4 Capacity Requirements

The principal factors in determining the spatial requirements of a tunnel including the required number of traffic lanes or tracks are the projected traffic volumes as well as the requirements for emergency egress and tunnel ventilation.

7.4.1 Projected Traffic Volumes

Traffic projections for the Strait of Belle Isle fixed link are based on the recent traffic volume data for the existing Strait of Belle Isle Ferry between Blanc-Sablon and St. Barbe and the traffic volume data for the existing Gulf Ferries providing services between North Sydney and Port aux Basques as well as between North Sydney and Argentia.

Based on discussions with the Harris Centre and the Government, similar traffic growth assumptions as in 2004 were made to estimate the projected traffic volumes for the Strait of Belle Isle Fixed Link; it was assumed that all traffic currently using the Strait of Belle Isle Ferry would be transferred to the new fixed link and that up to 60% of the current Gulf Ferry traffic can be diverted to the new Strait of Belle Isle Fixed Link.

7.4.1.1 Current Strait of Belle Isle Ferry Traffic Volume

Traffic volume data was provided to Hatch by the Government of Newfoundland and Labrador and was available for the years from 2000 to 2016, each annual data set for the months from May to October. The data is divided into the categories passenger vehicles and commercial vehicles. It is assumed that the number of tour busses using the Strait of Belle Isle Ferry is included in the commercial vehicles numbers; separate tour bus counts were carried out only until 2007. Traffic numbers are provided as Twenty-foot Equivalent Units (TEU). Based on information provided by the Government of Newfoundland and Labrador, the vehicle types associated with the TEUs are as shown in **Table 7-2**.

Table 7-2: Vehicle Types and associated Twenty-foot Equivalent Units (TEU)

| Vehicle | TEUs |
|--------------------------------------|------|
| Automobile | 1 |
| Motorcycle | 0.5 |
| Commercial Vehicles up to 6 m (20ft) | 1 |
| Commercial Vehicles over 6 m (20ft) | 2 |
| RVs up to 6m (20 ft) | 1 |
| RVs over 6m (20 ft) | 1.5 |
| Bus | 2 |
| Tractor Trailer | 4 |

Figure 7-3 illustrates the ferry usage development over the past 17 years. Except from some years of declining ferry usage, the traffic numbers have increased continuously over the past years.

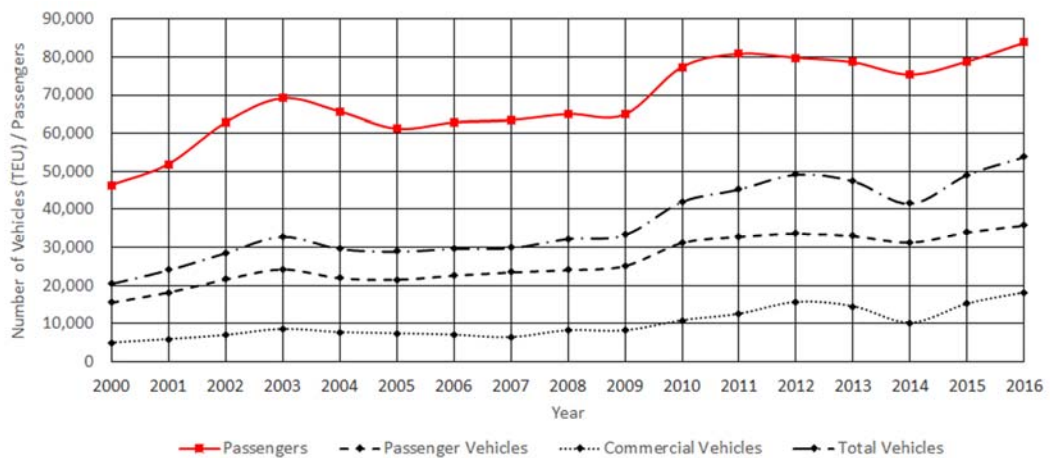


Figure 7-3: Strait of Belle Isle Ferry Usage - Private and Commercial Traffic

The data was further analyzed regarding total numbers of vehicles and percentage of traffic growth since 2004 (**Table 7-3**). Overall, there has been an increase in vehicle crossings of approximately 81% since 2004; the average annual growth was 5.5%. The largest annual growth occurred in 2010 with 26%; the most considerable negative growth of -12.5% was noted in 2014. Commercial vehicles account for 33% of the total traffic and represent the area of most significant growth.

Table 7-3: Strait of Belle Isle Ferry Usage - Blanc-Sablon/St. Barbe

| | 2004 (May to October) | 2016 (May to October) | Growth since 2004 ¹⁾ |
|-----------------------------|--------------------------|--------------------------|---------------------------------|
| Passengers | 65,623 | 83,815 | 27.7% |
| Passenger Vehicles (TEU) | 21,986 | 35,798 | 62.8% |
| Commercial Vehicles (TEU) | 7,715.5 | 18,079 | 134.3% |
| TOTAL Vehicles (TEU) | 29,701.5 | 53,877 | 81.4% |

1) Green indicating traffic increase; red indicating traffic decrease.

Most vehicles used the ferry during the tourist season in the months of July and August. Considering the maximum amount of 8,173 passenger vehicles (TEUs) and 3,615 commercial vehicles (TEUs) that used the ferry in August 2016 and assuming an even distribution over the entire month, the daily use of the ferry included 380 vehicles (TEUs).

7.4.1.2 Current Gulf Ferry Traffic Volume

Traffic volume data for the Gulf ferries was provided to Hatch by the Government of Newfoundland and Labrador.

7.4.1.2.1 Private Traffic Data

Traffic volume data for private vehicles was available for the ferry route North Sydney - Port aux Basques as well as the route North Sydney - Argentia for the years from 1999 to 2016. The data is divided into different ridership categories, including automobiles, trailers, campers, busses and motorcycles/bicycles. Total traffic numbers were provided as Passenger Related Vehicles (PRV) units. To allow a comparison with the Strait of Belle Isle Ferry traffic data, the PRV units were converted into TEUs using the following conversions which were chosen in accordance with the conversions provided by the Government of Newfoundland and Labrador for the Strait of Belle Isle Ferry.

- Automobile: 1 PRV = 1 TEU
- Trailer (assumed to be travel trailer): 1 PRV = 2 TEU
- Camper: 1 PRV = 1.5 TEU
- Busses: 1 PRV = 2 TEU
- Motorcycles/Bicycles: 1 PRV = 0.5 TEU

Figure 7-4 illustrates the development of the ferry usage by private traffic over the past 18 years. Except for some years of increasing ferry usage, the total traffic numbers have decreased continuously over the past years.

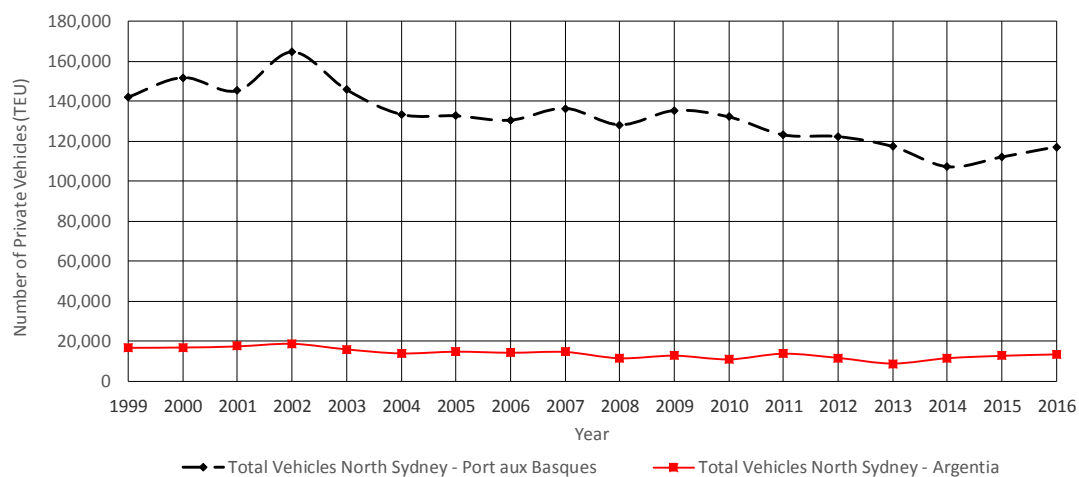


Figure 7-4: Gulf Ferry Usage - Private Traffic

The data was further analyzed for both ferry routes regarding total numbers of vehicles and percentage of traffic growth since 2004 (**Table 7-4** and **Table 7-5**).

North Sydney - Port aux Basques: Although the private traffic numbers for trailers, campers and motorcycles/bicycles have increased since 2004, an overall reduction of the ferry usage of 12.2% (including both directions) was noted due to the reduced demand for automobile and bus transfers; the average annual decline was approximately 1%. However, it should be noted that in more recent years the traffic numbers have slightly increased for all traveler categories. Overall, slightly more traffic is entering Newfoundland on this route than exiting.

Table 7-4: Gulf Ferry Usage - North Sydney/Port aux Basques - Private Traffic

| | 2004 | 2016 | Growth since 2004 ¹⁾ |
|------------------------------------|----------------|----------------|---------------------------------|
| Passengers | 386,078 | 299,121 | -22.5% |
| Automobiles ²⁾ | 109,818 | 91,293 | -16.9% |
| Trailers ²⁾ | 16,134 | 17,588 | 9.0% |
| Campers ²⁾ | 4,965 | 5,733 | 15.5% |
| Busses ²⁾ | 1,082 | 670 | -38.1% |
| Motorcycles/Bicycles ²⁾ | 1,377 | 1,852 | 34.5% |
| TOTAL Vehicles²⁾ | 133,376 | 117,136 | -12.2% |

1) Green indicating traffic increase; red indicating traffic decrease.

2) Values have been converted from PRV to TEU considering the assumptions stated above.

North Sydney - Argentia: On this ferry route, private traffic numbers for automobiles, trailers and campers have decreased since 2004; only the demand for bus and motorcycle/bicycle transfers have increased over the past years. The overall ferry usage by private traffic has decreased by 3.7% with an average annual decline of 1.1% (including both directions). In more recent years, the private traffic volume has increased for all traveler categories. Overall, slightly more traffic is exiting Newfoundland on this route than entering. Compared to the ferry route North Sydney - Port aux Basques, the route North Sydney – Argentia is significantly less travelled with only approximately 10% of all vehicles.

Table 7-5: Gulf Ferry Usage - North Sydney/Argentia - Private Traffic

| | 2004 | 2016 | Growth since 2004 ¹⁾ |
|------------------------------------|---------------|---------------|---------------------------------|
| Passengers | 33,393 | 29,407 | -11.9% |
| Automobiles ²⁾ | 9,620 | 9,299 | -3.3% |
| Trailers ²⁾ | 2,046 | 1,804 | -11.8% |
| Campers ²⁾ | 1,596 | 1,325 | -17.0% |
| Busses ²⁾ | 144 | 154 | 6.9% |
| Motorcycles/Bicycles ²⁾ | 506 | 812 | 60.6% |
| TOTAL Vehicles²⁾ | 13,912 | 13,394 | -3.7% |

1) Green indicating traffic increase; red indicating traffic decrease.

2) Values have been converted from PRV to TEU considering the assumptions stated above.

Most vehicles used the Gulf Ferries during the months of July and August. Considering the maximum amount of 24,798 private vehicles (TEUs) on the North Sydney – Port aux Basques route and 4,772 private vehicles (TEUs) on the North Sydney – Argentia route that used the ferry in August 2016 and assuming an even distribution over the entire month, the daily use of the ferries included 954 (29,570/31) private vehicles (TEUs). Assuming a 60% diversion towards the Strait of Belle Isle, 572 private vehicles should be considered using the new fixed link per day in addition to the traffic currently using the Strait of Belle Isle Ferry.

7.4.1.2.2 Commercial Traffic Data

Commercial traffic data was available as traffic entering and exiting the province for the years 1989 to 2016. The data set was cumulative for both Gulf Ferries and no differentiation between different types of commercial vehicles was made. No indication of the traffic unit used was given; it was assumed that the data is in CRV (Commercial Related Vehicles) units which were converted into TEUs considering the following assumptions:

- 40% of all traffic are tractor trailers: 1 CRV = 4 TEU
- 60% of all traffic are commercial vehicles over 6 m length: 1 CRV = 2 TEU

Figure 7-5 illustrates the development of the ferry usage by commercial traffic over the past 27 years. Although the overall total traffic numbers have increased over the past years, some years of decreasing ferry usage were observed, in particular in more recent years (2012 to 2015).

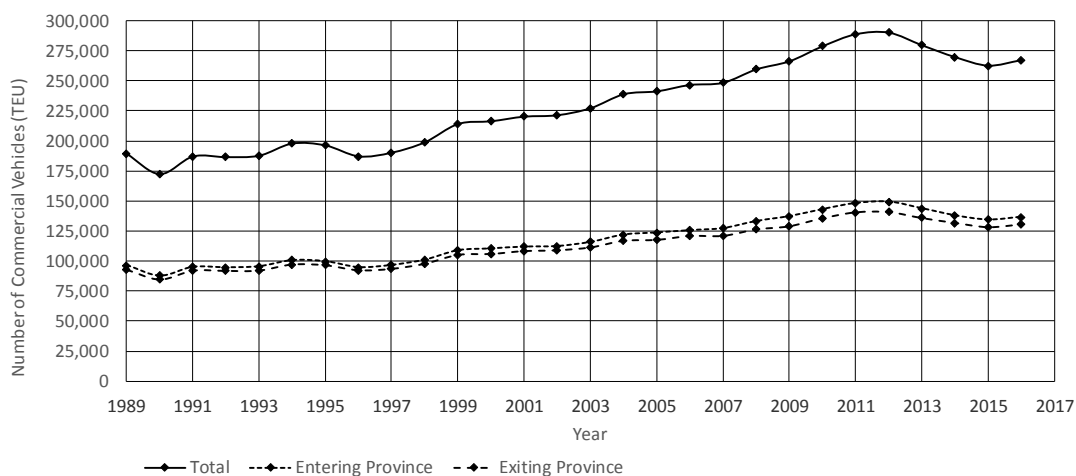


Figure 7-5: Gulf Ferry Usage - Commercial Traffic

The data was further analyzed regarding total numbers of vehicles and percentage of traffic growth since 2004 (**Table 7-6**). Overall the commercial traffic increased by 11.9 % since 2004, with a slightly larger number of vehicles leaving the province compared to the vehicles entering the province.

Table 7-6: Gulf Ferry Usage - Commercial Traffic

| | 2004 | 2016 | Growth since 2004 ¹⁾ |
|------------------------------------|----------------|----------------|---------------------------------|
| Entering Province ²⁾ | 116,992 | 130,609 | 11.6% |
| Leaving Province ²⁾ | 121,887 | 136,665 | 12.1% |
| TOTAL Vehicles²⁾ | 238,879 | 267,274 | 11.9% |

- 1) Green indicating traffic increase; red indicating traffic decrease.
- 2) Values have been converted from CRV to TEU considering the assumptions stated above.

Considering the maximum amount of 24,604 commercial vehicles (TEUs) that used the Gulf Ferries in August 2016 and assuming an even distribution over the entire month, the daily use of the ferries included 794 (24,604/31) commercial vehicles (TEUs). Assuming a 60% diversion towards the Strait of Belle Isle, 476 commercial vehicles (TEUs) should be considered using the new fixed link per day in addition to the private vehicles and the traffic currently using the Strait of Belle Isle Ferry.

7.4.1.3 Projected Fixed Link Traffic Volume

The traffic forecast was undertaken for the 30 years following the construction of the fixed link. Assuming a project duration of 12 years, the projection includes 42 years from today. An annual traffic growth of 2.5% was assumed. The resulting traffic volumes are shown in **Table 7-7**.

Table 7-7: Traffic Projection for Strait of Belle Isle Fixed Link

| | Volume Year 1 | Volume Year 42 ³⁾ |
|--|---------------|------------------------------|
| Strait of Belle Isle Ferry per Day (TEU) | 380 | 1046 |
| 60% Diversion from Gulf Ferries per Day – private traffic (TEU) ¹⁾ | 572 | 1574 |
| 60% Diversion from Gulf Ferries per Day – commercial traffic (TEU) ²⁾ | 476 | 1310 |
| TOTAL per Day (including both directions)¹⁾ | 1428 | 3930 |

- 1) Traffic volume for the Gulf Ferries were converted from PRV units to TEU (see 7.4.1.2.1).
- 2) Traffic volume for the Gulf Ferries were converted from CRV (assumed) units to TEU (see 7.4.1.2.2).
- 3) Assumed increase of 2.5% per year

7.4.2 Fixed Link Required Capacity

As in 2004, the fixed link capacity requirements were assessed in terms of peak hour volume that allows safe traffic conditions and the following assumptions were applied:

- Peak periods occur over a 5-hour window per day.
- 50% of the average daily traffic volume occurs during the peak period.

With a total daily volume of 3930 vehicles per day, approximately 393 vehicles should be accommodated during a peak design hour (50% of 3930, divided by 5 hrs), including both directions, or 197 vehicles per hour per direction, which is below the assumed theoretical capacity of 1,200 PCU (Passenger Car Units) per hour per direction for two-lane undivided Canadian highways.

However, for the fixed link road tunnel, the capacity will be determined partly by the traffic composition as well as the speed and geometric conditions of the tunnel. Since the tunnel is

to be designed for both passenger and commercial vehicles, the critical sections will be exit sections that include long uphill gradients for exiting vehicles. The profile for the tunnel includes exit sections that have a vertical gradient of 6% over lengths of more than 2 km. With heavy trucks using the fixed link, these vehicles will slow down considerably climbing out of the tunnel. At a travel speed of 50 km/h as assumed in the 2004 study, trucks will slow to approximately 27 km/h by the time they exit the tunnel. To reduce the delay to other vehicles and raise the level of service, an overall operating speed of up to 80 km/h should be permitted. Consideration could be given to adding a truck climbing lane in each of the exit sections; however, the implementation of a climbing lane would require a larger TBM diameter or a tunnel excavation using drill & blast.

Based on the updated peak traffic, a one-lane tunnel with alternating single direction operation is still appropriate to safely manage the projected traffic subject to modifying the permitted travel speed.

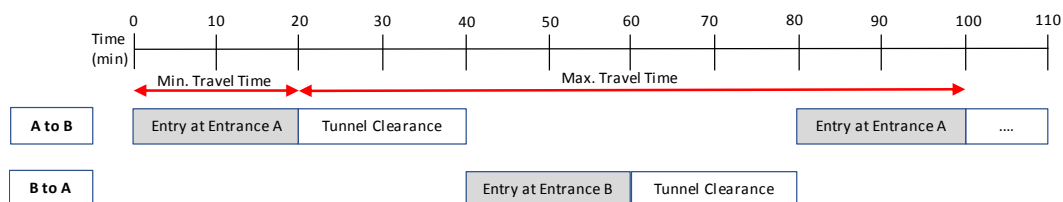
7.4.3 Tunnel Operation and Crossing Times

Two tunnel options, a single lane tunnel for individual vehicle traffic as well as a single-track tunnel for an electric shuttle train, were considered for the assessment of the tunnel operation and the potential crossing times.

7.4.3.1 Single-Lane Vehicle Tunnel

A single-lane tunnel with an emergency lane would be operated alternating in both directions with an assumed design operating speed of 80km/h. The advantage of using a higher speed in the tunnel (as opposed to the operating speed of 50 km/h as assumed in the 2004 study) is that it permits a shorter crossing time, shorter opening frequencies and higher level of service as the speed on the exit sections behind trucks would be approximately 42 km/h in lieu of approximately 27 km/h in the 50 km/h case.

The average travel time through the tunnel at a maximum operating speed of 80 km/h would be approximately 17 minutes (at an average speed of 72 km/h after adjusting for the effects of trucks slowing down at the exit sections). The operation scenario illustrated in **Figure 7-6** with a 20-minute operation phase could be considered. Operating hours for the road tunnel were assumed to be 7 days per week, 24 hours per day.



Note: A and B indicating the two tunnel entrances; 'A to B' indicating the travel direction.

Figure 7-6: Single-Lane Vehicle Operation

Entry into the tunnel would be alternating from both sides, allowing for a window of 20 minutes to enter the tunnel at one entrance and another 20 minutes of clearance time before

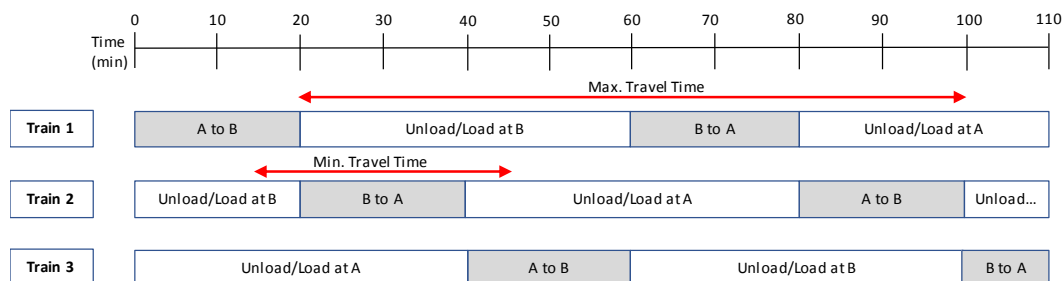
entry from the opposite entrance will commence. This would result in an 80 minutes schedule of entering the tunnel on each side.

The minimum crossing time for a vehicle arriving at a tunnel entrance during entry time would include 20 minutes of travel time through the tunnel. The maximum crossing time (waiting time plus travel time) of 80 minutes would occur for a vehicle arriving at a tunnel entrance just after the entry is closed. In this event, the crossing time would include 60 minutes of waiting time and 20 minutes of travel time.

During each 20-minute period on average approximately 66 vehicles will arrive at each portal (based on the peak amount of 197 vehicles per hour per direction arriving at the portal on each end of the tunnel). Considering that during a maximum waiting time of 60 minutes approximately 197 vehicles arrive at one tunnel entrance, the maximum number of vehicles that travel through the tunnel within the entry window of 20 minutes is 263 vehicles (197 vehicles accumulated during the waiting time plus 66 vehicles that arrive during the 20-minute entry time). This volume is equivalent to an hourly directional volume of 789 vehicles, which is below the theoretical capacity of 1,200 vehicles per hour as noted above.

7.4.3.2 *Single-Track Rail Shuttle Tunnel*

A single-track rail tunnel would also be operated alternating in both directions with an assumed design operating speed of 100 km/h. The operation scenario illustrated in **Figure 7-7** could be considered.



Note: A and B indicating the two tunnel entrances; 'A to B' indicating the travel direction.

Figure 7-7: Single-Track Shuttle Train Operation

Entry into the tunnel would be alternating from both sides, allowing for a window of approximately 20 minutes to travel through the tunnel, after which time a train will enter the tunnel from the opposite entrance. This would result in a 40-minute schedule of entering the tunnel on each side. A minimum of three shuttle trains would be required to accomplish this schedule, considering that at all times one train is travelling through the tunnel (as shown in **Figure 7-7**). While the first train (Train 1) enters the tunnel from one side (Entrance A), a second train (Train 2) loads vehicles on the opposite side (Entrance B). Train 2 starts travelling through the tunnel from Entrance B towards Entrance A as soon as Train 1 arrives at Entrance B. At the same time, a third train (Train 3) loads vehicles at Entrance A and gets ready to pass through the tunnel as soon as Train 2 arrives at Entrance A.

The minimum crossing time for a vehicle arriving at a tunnel entrance shortly before departure of the shuttle train would include assumed 5 minutes of loading, 20 minutes of travel time through the tunnel and further assumed 5 minutes for unloading. The maximum crossing time of 80 minutes would occur for a vehicle arriving at a tunnel entrance just after the shuttle train has left. In this event, the crossing time would include a maximum of 40 minutes of waiting and loading time, 20 minutes of travel time and further 20 minutes for unloading.

The rail tunnel option assumed operating hours of 7 days per week, 12 hours per day. The shuttle train configuration would have to allow for the maximum number of vehicles that are to be transported through the tunnel within the window of 20 minutes. Based on the peak amount of 197 vehicles per hour and direction, approximately 130 vehicles will arrive at one tunnel entrance during a maximum waiting time of 40 minutes, for which a 22-car train with six vehicles per car and sufficient seating capacity for passengers would be sufficient.

7.5 Clearance Requirements

7.5.1 Road Tunnel

The traffic analysis discussed above indicates that a single-lane tunnel operated in a cycle with flow in one direction followed by flow in the opposite direction will satisfy the projected traffic demands. There are no specific vertical and lateral clearance standards for single-lane uni-directional tunnels, and in North America, general standards for clearances in tunnels are not well established. The following documents were, therefore, referenced to obtain guidance on appropriate clearance standards for the road tunnel option:

- PIARC: World Road Association “Cross Section Geometry in Unidirectional Road Tunnels”
- AASHTO Standard Specification for Bridges, Section 2.5 Highway Clearance for Tunnels
- Geometric Design Guide for Canadian Roads, Part 1, Transportation Association of Canada

Using the guidelines within these documents and Hatch’s experience on similar highway tunnel projects, the parameters shown in **Table 7-8** were adopted for the development of highway tunnel cross-sections.

Table 7-8: Design Parameters – Road Tunnel Option

| Design Element | Desirable Criteria |
|---|-------------------------|
| Tunnel Design Speed | 80 km/h |
| Maximum Truck Width | 2.6 m |
| Maximum Truck Height | 4.2 m |
| Lane Width | 3.75 m |
| Vertical Clearance from Vehicle Running Surface | 4.65 m |
| Shoulder Width | 1.5 m |
| Off-Roadway Distance (without Walkway) | 2.25 m (0.75 m + 1.5 m) |
| Off-Roadway Distance (with Walkway) | 2.50 m (1.0 m + 1.5 m) |

This configuration permits vehicles to pass a truck or other vehicle that has broken down within the tunnel as shown in **Figure 7-8**.

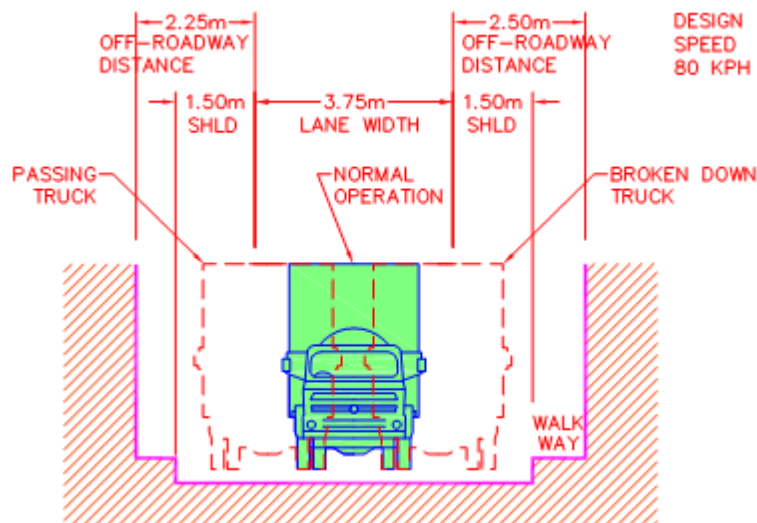


Figure 7-8: Road Tunnel Clearance Details

For higher speed applications, it is important to provide a barrier system that will redirect an errant vehicle back onto the roadway. Typically, this is done with the use of concrete barriers (Jersey barriers) delineating the edges of the shoulders. With the dimensions shown above, there is sufficient room to incorporate two 0.4 m wide barriers adjacent to the tunnel walls and a 1.0 m wide walkway on one side of the roadway.

7.5.2 Rail Tunnel

Roll-on/roll-off (RORO) tunnels are very rare globally; there are currently two operational tunnels that resemble the Strait of Belle Isle Fixed Link Project: the Channel Tunnel (completed 1994) connecting England to France and the Vereina Tunnel in Switzerland (completed 1999). The tunnel size requirements that were established in the 2004 study assumed that a shuttle railcar similar to that used for the Channel Tunnel and an associated tunnel clearance envelope are appropriate for the fixed link. These dimensions are still reasonable assumptions. **Figure 7-9** shows the required dynamic clearance envelope for such a vehicle.

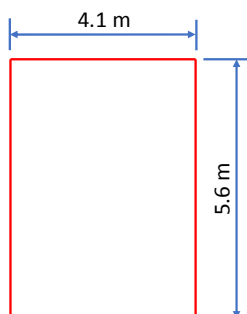


Figure 7-9: Dynamic Clearance Envelope for Shuttle Railcar

Locomotives, cars, and equipment would need to be custom designed for a project of this size and type which would require design effort and consultation with train manufacturers prior to determining the final tunnel dimensions.

7.6 Ventilation

7.6.1 Code and Standards

When developing a tunnel ventilation system, applicable codes and standards provide guidelines and recommendations to govern the design. However, it should be noted that most international standards and guidance documents are typically generic and not tailored to specific tunnel scenarios and being generic, they must cater for a wide range of alternative designs. Therefore, it is often appropriate to introduce some level of performance based design to ensure that a suitable system is provided. This approach then considers emergency ventilation in context and within a fully integrated safety strategy for the complete tunnel. For the tunnels subject of this study, National Fire Protection Agency codes NFPA502 and NFPA130 are applicable standards.

7.6.2 Ventilation Design Consideration

Standards and guidance would recommend that a mechanical ventilation system (ranging from a non-ducted longitudinal ventilation system, to a fully ducted transverse ventilation system) should be incorporated for a road or rail tunnel of an equivalent length to the Strait of Belle Isle Fixed Link. The type of system would mainly depend on the type of tunnel (uni-directional or bi-directional) and the expected traffic capacity (i.e. non-congested or congested traffic in the case of road). In the case of a road tunnel, compared with similar fixed link projects, the traffic data for the Strait of Belle Isle Fixed Link indicates a relatively low number of vehicles. It is, therefore, expected that under normal operation, congestion inside the tunnel can be prevented, even during peak hours. On this basis, the use of a longitudinal ventilation system for smoke control is proposed in the conceptual design.

A longitudinal ventilation approach would also eliminate the need for a ventilation island at mid-tunnel, an undesirable structure for shipping through the Strait. A longitudinal ventilation design supported by ventilation plants at the portals only could thus reduce both capital and running costs and environmental impacts. **Figure 7-10** shows a typical example of a longitudinal ventilation system in a road tunnel using jet fans.

For road tunnels, the most probable fire hazards will arise from the vehicles using the tunnel, the materials from which they are manufactured and the materials they are transporting. Another potential source of fire hazard comes from failures of equipment in the road tunnels or any plant and equipment room which, although fire separated from the tunnel, may still impact the operation.



Figure 7-10: Example of Jet Fans installed in a Road Tunnel

The conceptual design of the ventilation system has incorporated fan redundancy requirements to account for the potential of a fan failure during an emergency.

7.6.2.1 Road Tunnels

Road tunnels pose specific challenges in terms of ventilation and fire safety design. While diluting the traffic exhaust pollutants to acceptable international limits remains the purpose of any ventilation design, the issue of light extinction and visibility through diesel smoke can be more significant in tunnels with a high proportion of diesel engines or commercial trucks. In normal operation, the average speed of traffic usually provides adequate piston effect to ventilate a tunnel longitudinally through the portals, without operating any fans. In peak hours, when low-speed congested traffic is expected, the aerodynamic drag of vehicles drops while the journey time and hence the pollution rate per vehicle increases. The tunnel ventilation system shall be able to cater for this critical scenario.

Finally, the case of fire emergency has to be considered, with the ventilation system capable of either removing smoke through duct-work (transverse ventilation) or providing critical velocity to prevent smoke back-layering (longitudinal ventilation). In many tunnels, it is this last design criterion of tenability for fire emergencies that drives the fan capacity and overall system cost. Although longitudinal design is the most cost-effective method (typically a fraction of the cost of fully transverse), it has obvious limitations with tunnel length and in a fire emergency, where one side of the fire has to be sacrificed as smoke downstream of the fire makes that portion of tunnel untenable. Because of their smaller space demand and lower cost, the use of jet fans has become considered the choice among many recent tunnel ventilation designers for longer tunnels, other variants combining jet fans with intermittent

vent shafts have been used. In the case of the Strait of Belle Isle Fixed Link, having an intermittent vent shaft is not considered a feasible solution.

In the event of a fire in the road tunnel, it is assumed that the traffic in front of the fire will continue to drive through the tunnel and will be travelling faster than the flowing smoke layer. The traffic behind the fire will stop and the occupants of these vehicles will commence the evacuation upstream of the fire (as illustrated in **Figure 7-11**). The occupants will be notified via the signage and the alarm system to evacuate into the designated evacuation path. Cross passages into the evacuation path could be spaced at intervals of 200 m, less than recommendations in international guidance, providing an increased level of life safety.

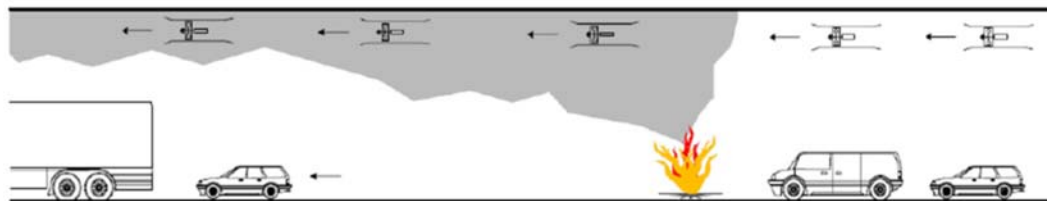


Figure 7-11: Ventilation of Road Tunnel in the Event of Fire using Jet Fans

For the proposed Strait of Belle Isle Fixed Link tunnel, the use of a ducted ventilation system significantly increases the cross-sectional area and perhaps necessitates the need for a ventilation island in the middle of the tunnel. Preliminary concept calculation has indicated that the required ventilation duct would be in the order of up to 45 m² to overcome the pressure differences associated with ducting hot gases. This duct area could be reduced by the introduction of a ventilation island in the middle of the tunnel, thereby only requiring hot gases to be ducted a maximum of 10 km. This would have the benefit of providing additional space for ventilation fan stations. However, introducing an island in the middle would have the recognisable drawback of increasing the risk for ship collisions and environmental consequences. A ducted system however has a number of other implications for the tunnel design over and above the longitudinal system. These include a reduced reliability and an increased risk to life safety due to more single points of failure, e.g. extract fan failure or fire damper failure, both of which have the potential to impact on the effectiveness system wide. Furthermore, there are increased maintenance requirements, an increased construction time and increased costs.

7.6.2.2 Rail Tunnels

In accordance with current practice for most railway tunnels, a train on fire should make all attempts to leave the tunnel and this practice will be initiated across the Strait of Belle Isle Fixed Link. Considering the block sections and timetabling discussed above, the low rail traffic volumes in the operating period indicate that it is unlikely that more than one train will be present in the tunnel at any one time. For both road and rail tunnels based on past experiences of tunnel fires and the predictive traffic volumes and composition, it is possible to identify relevant fire hazards.

For the rail tunnel, the potential source of fire hazard comes from failures of equipment in the tunnel or any plant and equipment room which, although fire separated from the tunnel, may still impact the operation.

7.6.3 Emergency Egress

Typically, in tunnel fires the critical areas for life safety are within the vicinity of the fire and in the order of a few hundred metres away, particularly downstream of the fire. At that distance, away from the fire, visibility is likely to be lost; however, smoke temperatures will be reduced (especially with the inclusion of a suppression system) and toxicity often reduced to tenable conditions because of dilution. Within the Strait of Belle Isle Fixed Link road tunnel, exits are proposed every 200 m for self rescue. With a single-lane tunnel, emergency exits can only be provided by a passageway that is fire-separated from the vehicular space. For the rail tunnel, emergency egress will be via the walkway to a rescue vehicle that will be dispatched from the upstream (ventilation-wise) end of the tunnel.

7.6.4 Fire Suppression System

Fire suppression systems have been installed for many years in many long road tunnels in Japan and, during the last 20 years, in all road tunnels in Australia. Recently, following the major fires in the Mont Blanc, Tauern, Gotthard and Frejus tunnels in Europe, the World Road Association (PIARC) and the National Fire Protection Association (NFPA) have both revised their recommendations concerning suppression systems to much more positive consideration, but clearly state that such systems should only be installed as a part of an overall safety approach.

The trend for railway tunnels is less defined as there are limited known rail tunnels internationally incorporating a suppression system. This may be for a number of reasons including that rail fires are less frequent than road tunnel fires and, that in the event of a fire, trains are designed to drive out of the tunnel and thereby further reduce the incidents of rolling stock fires in tunnels. On this basis, the cost/benefit of the suppression system may not be perceived to be significant enough to warrant the initial investment. However, the shuttle vehicles will be equipped with self contained fire suppression systems.

In both road and rail tunnels with fire suppression installed, the immediate effect will be to limit the fire size and control the fire growth. Fires occurring in vehicles may sometimes not be directly affected by a suppression system if the fire is shielded by the vehicle. In these instances, the suppression system is controlling the fire rather than suppressing or extinguishing the fire. By controlling the fire, the development of a catastrophic fire can be avoided and the tunnel structure protected, minimizing damage and repair time. If the fire is controlled, the chances of a successful evacuation will increase while aiding the emergency services' ability to control the situation. Another aspect is that by minimizing the frequency of development of a catastrophic fire, the risk to life safety can be drastically reduced and, furthermore, the downtime as a result of a fire can be minimized. In the context of the overall risk assessment for the Strait of Belle Isle Fixed Link tunnel, these two aspects assist in ensuring that the risk level to tunnel users, operational revenue and reputation is as low as reasonably practicable. For this reason, a deluge suppression system is recommended to be

considered for installation in both the road and rail tubes of the Strait of Belle Isle Fixed Link provided issues related to low ambient temperatures can be resolved.

7.6.5 Ventilation Strategy

The use of a longitudinal ventilation system for smoke control is the basis for the conceptual design of the Strait of Belle Isle Fixed Link. This longitudinal ventilation approach is cost effective and eliminates the need for a ventilation island at mid-tunnel. For this design, jet fans will be placed along the tunnel as shown in **Figure 7-12**. A typical jet fan is shown in **Figure 7-13** below. During normal operation, both road and rail tunnels are self-ventilated as a result of generated piston effects with moving cars/ trucks or trains. At later stages of design for the option of road tunnel, in case generated piston effect was determined not be sufficient to bring the level of contaminants down to an acceptable level, some of the jet fans could be in operation to exhaust contaminants through portals.

In the event of emergency for road tunnel option, the traffic in front of the fire will continue to drive through the tunnel. The traffic behind the fire will stop and the occupants of these vehicles will commence the evacuation upstream of the fire. Jet fans will be in operation to direct the smoke opposite to the direction of evacuation. For rail tunnel in a similar pattern, smoke will be directed to the opposite direction of evacuation. In both tunnel options, a protected egress path within the tunnel has been allocated for safe evacuation of passengers.

With 21 km length of Strait of Belle Isle Fixed Link, four jet fans are proposed to be installed at every 500 m interval (**Figure 7-12**). This will translate to about 168 fans in the tunnel.

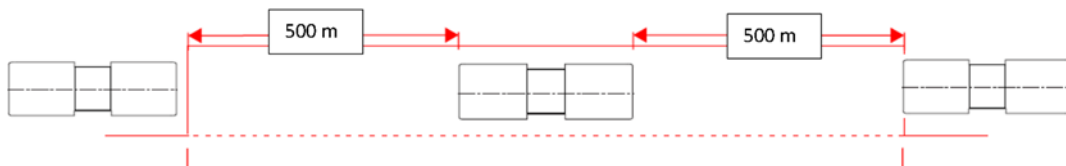


Figure 7-12: Longitudinal Cross Section Along Proposed Ventilation Arrangement

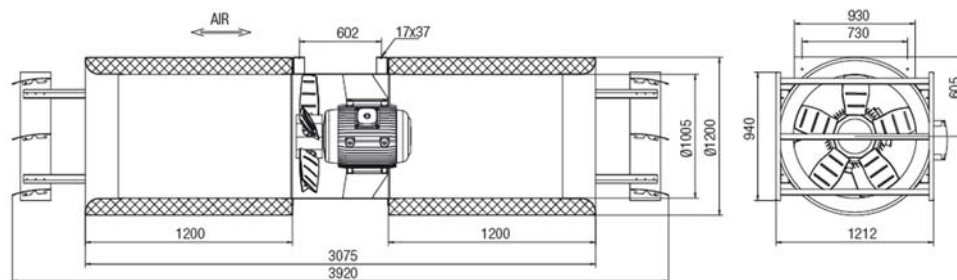


Figure 7-13: Typical Jet Fan

7.7 Tunnel Services

There are a number of tunnel services that will impact the tunnel spatial arrangements, schedule and costs. Dedicated services shall be serving both, the travelled portion of the tunnel and the emergency egress space.

For the rail tunnel, a separate consideration is required for traction power provisions within the tunnel, including catenary integrity, grounding conditions within the tunnel and traction power reliability.

7.7.1 Road Tunnel

7.7.1.1 Tunnel Communications Services

Fire detection is particularly important for road tunnels. Due to the tunnel length, video surveillance based fire detection is recommended as a part of the overall tunnel surveillance strategies. Thermal cameras located at strategic locations within the tunnel and connected to the main backbone should be installed for early warning of a fire within the tunnel so that traffic in and out of the tunnel could be properly managed. The entire tunnel should be under the surveillance so that all tunnel conditions for safety, traffic control and tunnel operations are fully observed.

A dedicated tunnel control centre should act as the main communications node. Responsibility for the centre includes safety, traffic control and tool collection. A backup control centre should be planned at the other tunnel end.

All tunnel services, such as ventilation, road conditions monitoring, power and lighting controls, drainage pumps, etc., should also be remotely controlled.

As per NFPA-502, emergency call stations must be located at strategic locations within the tunnel. Typically, such emergency nodes include an emergency telephone, blue light for identification and fire-extinguisher. Such emergency stations are placed within the tunnel at regular distances (typically 250 m apart).

Tunnel signage are placed at strategic locations within the tunnel for both emergencies as well as traffic announcements. Both static and dynamic signage are used along the tunnel clearly identifying tunnel stationing, access to closest escape route, status of traffic flow, etc. Tunnel portals require additional signage to ensure safe passage and constant supervision of the traffic flows, which is specifically important for a single-lane tunnel with alternating travel directions.

Significant communications requirements for tunnel operations will be served by a dedicated backbone, redundant optical cables that run along the entire tunnel length.

7.7.1.2 Tunnel Electrical Services

Utility power provisions for the tunnel are assumed to be available at both tunnel portals. Otherwise, additional large in-situ backup generation power needs to be provided. Due to the tunnel length, medium voltage power distribution within the tunnel is required. As the result, intermediate power stepdown transformers would be required within the tunnel, as illustrated

in **Figure 7-14**. Placements of those stepdown transformers within the tunnel would require accessible space, fire-protection, alarming and fire-fighting facilities.

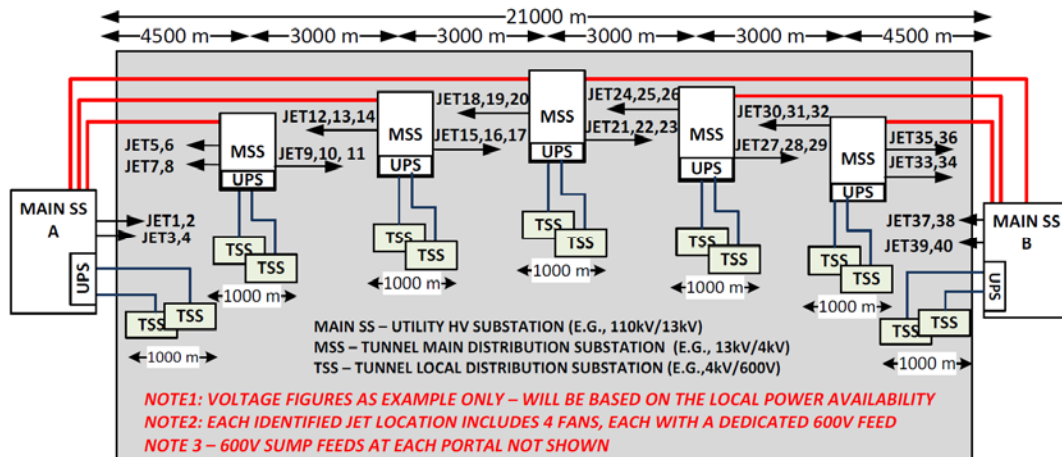


Figure 7-14: Power Distribution for Road Tunnel Services (illustration only)

Tunnel electrical services considered life safety for tunnel lighting, communications and tunnel smoke ventilation. Tunnel power distribution for emergency services is based on a 600V AC distribution infrastructure along the entire tunnel length, designed and placed to ensure continuous power services. Tunnel electrical infrastructure is designed to ensure a minimum 2-hr fire rating for all emergency services.

All tunnel lighting is designated as emergency, LED based lighting that ensures durability, low consumption and low maintainability requirements. Tunnel lighting is rated at 600V AC to optimize overall power requirements and coverage. Tunnel lighting is designed so that any single fault will not impact the minimal lighting conditions within both traffic and emergency escape tunnels. Additionally, emergency power at 120V AC, typically required for communications needs, is obtain by the local 600/120V AC mini-substations strategically located within the tunnel (**Figure 7-15**).

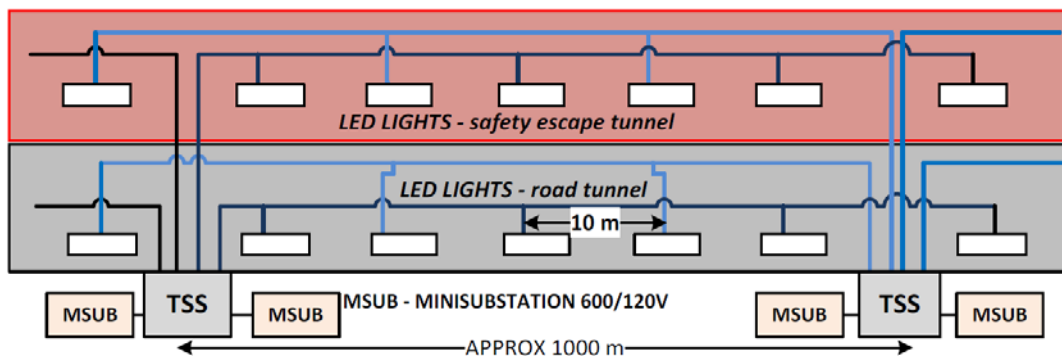


Figure 7-15: Road Tunnel Power Distribution - Typical Section

Additional (normal) power should be provided for other tunnel services like fans, receptacles, sumps, traffic lane lights and portal lights. Preliminary tunnel power load estimates for both emergency and normal power are provided in the following **Table 7-9** and **Table 7-10**.

Table 7-9: Road Tunnel Emergency Power Requirements

| Essential Services (emergency power) | Total Load kVA (estim.) | Quantity | Comment |
|--|-------------------------|----------|--|
| Emergency lights one every 10 m for the main tunnel; 64W per light fixture | 240 | 21 x 100 | Section length approximated. Total tunnel length 21 km. Each light 1.73*64=111 VA |
| Emergency lights one every 10 m per 1000 m emergency tunnel section; 32W per light fixture | 120 | 21 x 100 | Section length approximated. Total tunnel length 21 km. Each light 1.73*32=55 VA. |
| Emergency Blue Stations – four per 1000 m tunnel section (approx. 250 m apart) | 370 | 21 x 4 | Each Blue Station with telephone, repeater, camera and networking switches (2.5kW each = 4.3 kVA). |
| Portal Infrastructure | 40 | 2 | Portal light, info boards, security |

Total 770 kVA

NOTE: Electrical losses are considered in the above rounded total load numbers.

Table 7-10: Road Tunnel Normal Power Requirements

| Other Services (normal power) | Total Load (estim.) | Quantity | Comment |
|--------------------------------------|---------------------|------------|--|
| Tunnel ventilation (redundant feeds) | 3,600 kVA | 21 x 2 x 4 | 4 jet fans per 500 m. 90 kVA each |
| Tunnel sumps | 120 kVA | 2 | One at each portal end; 60 kVA each |
| Traffic lane tunnel lights | 250 kVA | 21 x 200 | 2 x 100 LED (32W) per 1000 m (one above each lane) |

Total 3,970 kVA

NOTE: Electrical losses are considered in the above rounded total load numbers.

7.7.2 Rail Tunnel

7.7.2.1 Tunnel Communications Services

Fire detection is less of an issue for rail tunnels compared to road tunnels. Fire detection is recommended for areas with potential fire hazards, e.g., electrical substations and the major electrical components. A standard in-situ fire/smoke detection is recommended. Places of interest within the tunnel, such as main communications and electrical nodes, should be under surveillance. Moreover, all rail cars should be under surveillance with on-line video streams available within the control centre.

A dedicated tunnel control centre should act as the main communications node. Responsibility for the centre includes safety and security for all rail and tunnel operations. A backup control centre should be planned at the other tunnel end.

All tunnel services such as ventilation, tunnel conditions monitoring, power and lighting controls, drainage pumps, etc. should be remotely controlled.

As per NFPA-130, emergency call stations must be located at strategic locations within the tunnel. Typically, such emergency nodes include an emergency telephone and blue light. The emergency stations are placed within the tunnel at maximum distances of 240 m apart.

Tunnel signage are placed at strategic locations within the tunnel for information during emergency situations but also for general information during normal maintenance activities.

Significant communications requirements for tunnel operations will be served by a dedicated backbone, with redundant optical cables, that runs along the entire tunnel length.

7.7.2.2 Tunnel Electrical Services

Utility power provisions for the tunnel are assumed to be available at both tunnel portals. Otherwise, additional large in-situ backup generation power needs to be provided. Due to the tunnel length, medium voltage power distribution within the tunnel is required. As the result, intermediate power stepdown transformers would be required within the tunnel, as illustrated in Figure 7-16. Placements of those stepdown transformers within the tunnel would require accessible space, fire-protection, alarming and fire-fighting facilities.

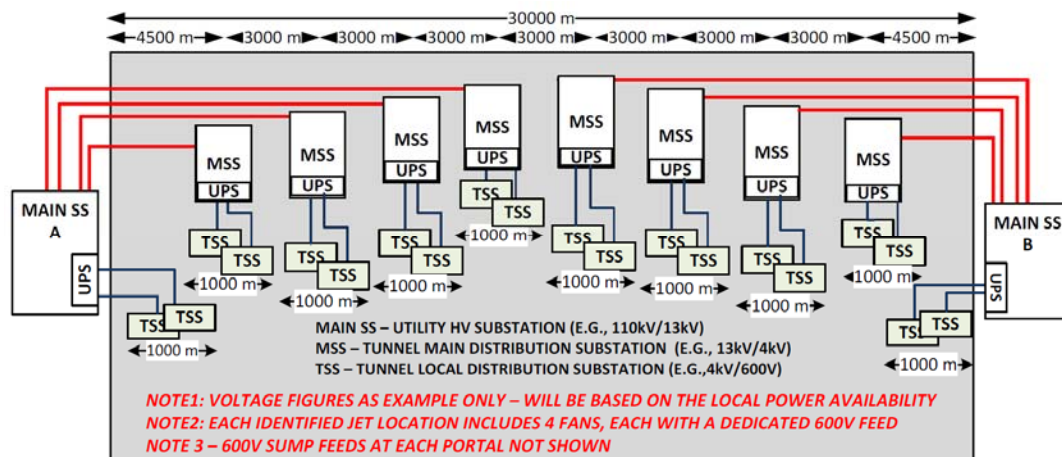


Figure 7-16: Power Distribution for Rail Tunnel Services (illustration only)

Tunnel electrical services considered life safety; for tunnel lighting, communications and tunnel smoke ventilations. Tunnel power distribution for emergency services is based on a 600V AC distribution infrastructure along the entire tunnel length, designed and placed to ensure continuous power services. Tunnel electrical infrastructure is designed to ensure a minimum 2-hr fire rating for all emergency services.

All tunnel lighting is designated as emergency. LED based lighting that ensures durability, low consumption and low maintainability requirements. Tunnel lighting is rated at 600V AC to optimize overall power requirements and coverage. Tunnel lighting is designed so that any single fault will not impact the minimal lighting conditions within both traffic and emergency escape tunnels. Additionally, emergency power at 120V AC, typically required for communications needs, is obtain by the local 600/120V AC mini-substations strategically located within the tunnel (Figure 7-17).

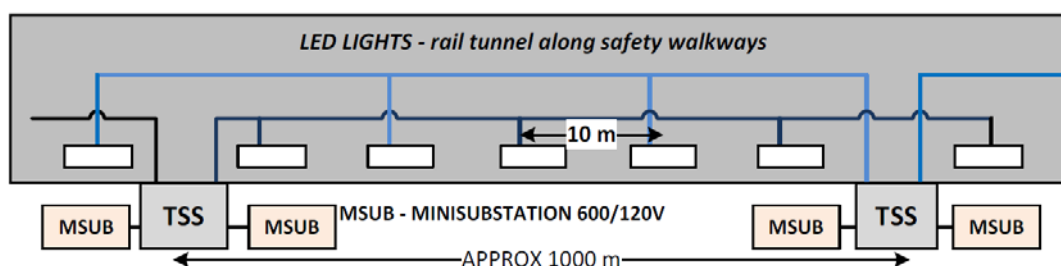


Figure 7-17: Rail Tunnel Lighting – Typical Section

All tunnel communications power shall be on emergency services. Additional (normal) power shall be provided for other tunnel services e.g., receptacles, sumps and portal lights. Preliminary tunnel power load estimates for both emergency and normal power are provided in the following **Table 7-11** and **Table 7-12**.

Table 7-11: Rail Tunnel Emergency Power Requirements

| Essential Services | Total Load (estm.) | Quantity | Comment |
|--|--------------------|----------|---|
| Emergency lights one every 10 m for the main tunnel; 64W per light fixture | 350 kVA | 30 x 100 | Section length approximated. Total tunnel length 30 km. Each light 1.73*64=111 VA. |
| Emergency Blue Stations – four per 1000 m tunnel section (approx. 250 m apart) | 550 kVA | 30 x 4 | Each Blue Station with telephone, repeater, camera and networking switches (2.5kW each = 4.3 kVA) |

Total 900 kVA

NOTE: Electrical losses are considered in the above rounded total load numbers.

Table 7-12: Rail Tunnel Normal Power Requirements

| Other Services | Total Load (estm.) | Quantity | Comment |
|--------------------------------------|--------------------|----------|---|
| Tunnel ventilation (redundant feeds) | 1400 kVA | 2 x 2 | Push-Pull fan operation, 1 backup fan at each portal. |
| Portal Infrastructure | 40 kVA | 2 | Portal light, info boards, security. |
| Tunnel sumps | 120 kVA | 2 | One at each portal end; 60 kVA each. |

Total 1,560 kVA

NOTE: Electrical losses are considered in the above rounded total load numbers.

7.7.2.3 Traction Power

Traction power is assumed to be supplied to the train cars at 25kV AC single phase. Sources for traction power would be from each end of the tunnel at the portal area and will be from dedicated traction power substations (TPSS). Power utilities in the Province of Newfoundland and Labrador would need to be consulted and accept potential de-balance that could be caused by the project as the train will run on a single phase 25kV AC.

Traction power substations will be sized to accommodate the proposed 3MW train propulsion system. This will consider train traction power and auxiliary loads as well as system losses for delivering the power to point of use as the train travels along the track alignment. Under normal operating conditions, each TPSS would feed half of the tunnel (AC systems are typically sectionalized) but the system will be designed to be able to feed the whole tunnel under a contingency condition from a single TPSS. This would allow getting the train out of the tunnel if one of the TPSS fails.

Overhead contact rail would provide a compact design from a space and ampacity prospective. The 25kV overhead insulators must abide by clearance limits from live to grounded components. All mounting hardware and other associated equipment will need to be grounded as per the Canadian Electrical Code requirements throughout the tunnel. Isolation switches for the overhead rail system to be placed at intervals within the tunnel to accommodate isolation and grounding of the overhead rail for maintenance purposes. Track and OCS maintenance is assumed to take place outside of the normally scheduled operational times of the trains.

At the midpoint of the tunnel (**Figure 7-18**), three high voltage (HV) cabinets (one for each HV connection to the OCS and one to house the remotely operable HV breaker or current interruptible switch) will be placed to allow for an isolation point between two different power sources, one from each tunnel end. Space within the tunnel for the HV equipment and related auxiliary cabinets would be required at this location (e.g., step down transformer, UPS system, controls, communication, etc.).

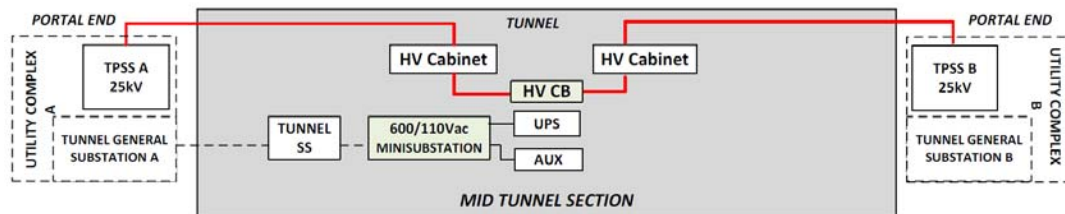


Figure 7-18: Rail Tunnel Traction Power – Typical Mid Tunnel Section

The midpoint isolation breaker and supply breakers from the TPSS provide an integrated protection scheme for transfer trip of the breakers under abnormal condition detection along a section of the line.

Return path for the power supplied to the trains will be by the rail system. Rails on the AC system will be grounded for safety reasons. All wayside equipment will also be grounded. DC stray current is minimal for an AC systems and cathodic corrosion will be minimal, but should be reviewed as part of the due diligence for the project.

7.7.3 TBM Construction Power Requirements

If the tunnel will be excavated by two TBMs, each working from the opposite tunnel sides, two construction camps with all facilities are required near each portal. An initial estimate is to have temporary (construction) power means of 4 MVA at each launch shaft for a TBM, plus 2 MVA for temporary tunnel ventilation and lighting and the construction site needs.

7.8 Rescue Strategy

Due to difference in tunnels construction and operations, separate rescue strategies apply to the road and rail tunnel options.

7.8.1 Road Tunnel

Considering that rescue efforts would apply to the entire tunnel, an emergency space separated from the travelled portion of the tunnel will be available along the entire tunnel length. Access into the emergency escape route will be provided every 250 m; fully identified, easy accessible and with public announcements and security cameras to support evacuations.

The emergency escape space will have its own lighting, communication and ventilation infrastructures to ensure passenger safety and access.

The key emergency features are traffic controls to minimise the number of vehicles remaining within the tunnel in case of an emergency. Traffic flow planning and supervision will require increased traffic controls, lane signalling and surveillance. Therefore, in addition to a single lane normal transit through the tunnel and strict speed controls, the tunnel layout will enable emergency vehicle passage even if traffics stops within the tunnel.

7.8.2 Rail Tunnel

The evacuation strategy for the rail tunnel option considers that only a single train will be present within the tunnel at any time and that the number of passengers per train will be strictly controlled. Protocols for shuttle train inspections before trains enter the tunnel have to be regular and completed for each tunnel run. Each train car must be equipped with its own fire detection and suppression systems.

An electrical train within the tunnel will follow a predetermined emergency strategy. Priority will remain to get the train out of the tunnel in case of emergencies. Any emergency train stop within the tunnel will be followed by a catenary system disconnection prior to evacuation efforts.

Communication infrastructure between the control centre and the train drivers is of utmost importance; redundancy and additional protective measures will be required to ensure such communications systems are available.

Rescue vehicles will be available at each tunnel portal and will be sized to evacuate all passengers in a single attempt. A separate emergency space along the tunnel will not be required considering this rescue approach.

7.9 Tunnel Dimensions

The dimensions of the tunnel cross-sections were defined based on the requirements for clearance, ventilation, emergency egress and other services as discussed above.

7.9.1 Road Tunnel

As in 2004, a tunnel cross section with an inner tunnel diameter of 11 m is suitable to accommodate the required widths of the traffic lane including passing lane and the emergency space. The proposed cross-section is shown in **Figure 7-19**. The area above the

travelled lane, that was previously designated as exhaust ventilation space, can accommodate the proposed jet fan ventilation system. Room for required utilities, cables etc. as well as for potential future utilities is available above the emergency space.

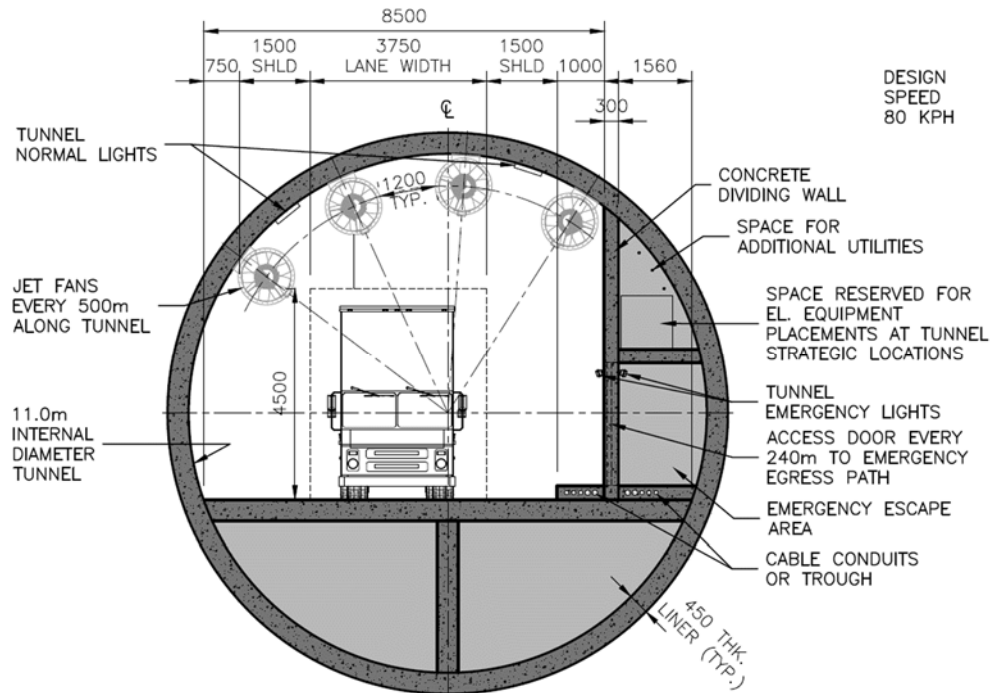
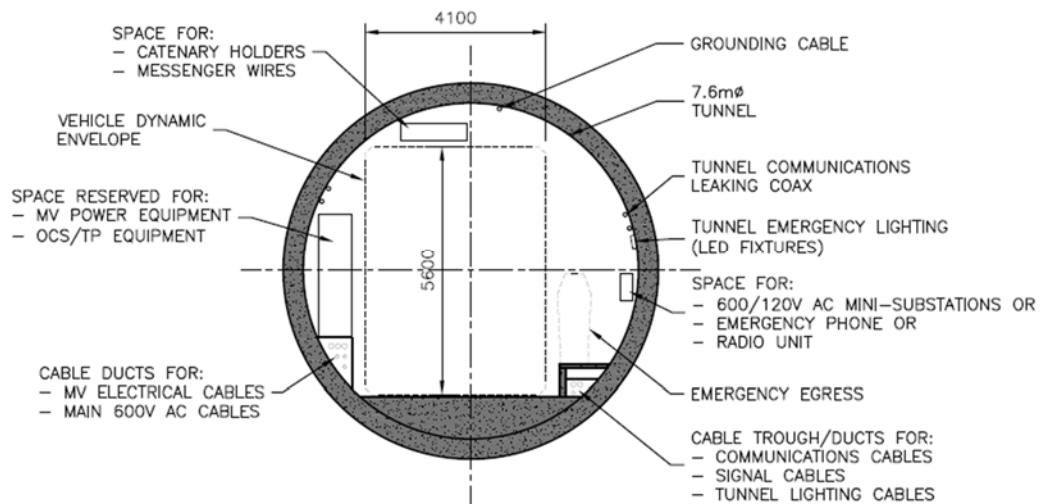


Figure 7-19: Proposed Cross-Section – Road Tunnel

7.9.2 Rail Tunnel

For the rail tunnel, as in 2004 a tunnel cross section with an inner tunnel diameter of 7.6 m is suitable to accommodate the required widths and height of the single-track shuttle train including the catenary system. The proposed cross-section is shown in Figure 7-20. For emergency egress, with only one train in the tunnel, the tunnel would be available for egress. Passengers would exit the shuttle train and egress along the walkway towards the exiting direction of the tunnel to the rescue vehicle.



Note: To ensure minimal lighting and communications within the tunnel during emergencies, MV power equipment and associated cables shall be fire protected or implemented with full redundancy.

Figure 7-20: Proposed Cross-Section – Rail Tunnel

7.9.3 Tunnel Approaches, Road Connections and Terminus Facilities

The concept for the tunnel approaches on both shores including road connections and terminus facilities was discussed in the 2004 study. The general layout of these approaches with their entailed portal and terminus structures as well as the new roads required to connect the tunnel with the existing road system on both sides of the Strait as described in the 2004 study is still considered valid. A more detailed study of the approaches would be required at a later stage of the project.

7.10 Excavation Methods

The excavation method for the tunnel excavation across the Strait of Belle Isle has to be appropriate for the ground conditions and must be capable of addressing the various challenges that might be encountered on shore and underneath the Strait.

In very general terms, the geology for the Strait crossing is typified by more permeable sedimentary rock overlying less permeable gneiss. Considering that a tunnel excavation in the less permeable gneiss would result in a significantly longer tunnel alignment at great depth and, hence, significantly higher costs, it is assumed that the tunnel will be excavated in sedimentary rock of varying quality, from strong, good quality rock to highly fractured and weak rock. Challenges that have to be expected during tunnelling under the Strait include the following:

- Varying ground conditions including very strong, stable rock to weak, unstable rock, requiring different means of rock support and/or reinforcement.
- Locally poor ground conditions with highly fractured, unstable rock, related to fault zones or areas of weak shale.

- High water inflow into the tunnel through fractures in the rock mass, related to fault zones or other areas of poor ground conditions.
- High hydrostatic pressure of up to approximately 13 bar if faults or vertical fractures connect the tunnel to the water body of the Strait of Belle Isle (considering a tunnel elevation of 130 m below water level; actual pressure will depend on tunnel depth).
- Time-dependent rock deformations that might occur following the excavation, including squeezing and swelling of the rock.

7.10.1 Drill & Blast Excavation

Drill & blast excavation methods involve the controlled use of explosives. The method requires several steps involving multiple machines, resulting in a higher cycling time and typically in a slower advancement rate. Drilling equipment varies from handheld jackhammers and jack legs to large mobile jumbos and gantries, depending on the size of the tunnel heading. The typical drill & blast excavation scheme is shown in **Figure 7-21**. Typically, drill & blast techniques are more suitable for tunnels with larger diameter or with multiple access points, which allow development of multiple faces at once, resulting in higher operational efficiency.

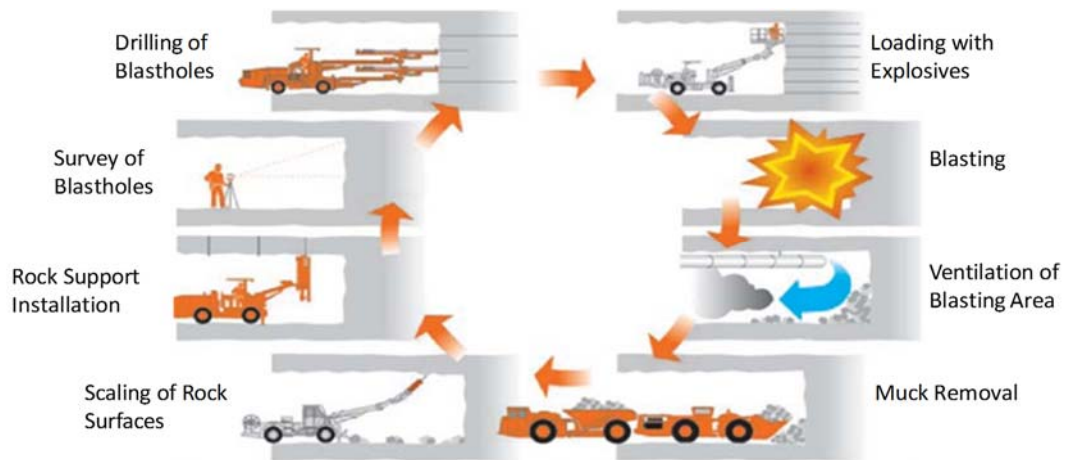


Figure 7-21: Typical Drill & Blast Excavation Scheme

The general advantage of drill & blast excavation is its flexibility regarding excavation size and ground support. In changing or difficult ground conditions such as faults or squeezing ground, the support as well as the excavation diameter can be adjusted to the requirements based on the encountered conditions. The advance rate for drill & blast excavations depends on the rock support required; minimal support installation in stable ground results in greater advance rates.

Drill & blast excavation is suitable for all rock types. Problems are typically related to over-excavation in areas of fractured rock or due to poor blasting operations as well as to the evacuation of gases from explosives after blasting. Issues such as noise or vibration due to the use of explosives do not apply to the Strait of Belle Isle excavation.

7.10.2 Mechanical Excavation (Roadheader)

Mechanical excavation includes the use of mechanical cutting equipment such as a roadheader (see **Figure 7-22**). A roadheader is a specialized mechanical equipment for the excavation of rock that uses a rotating cutting head system installed at the end of a hydraulic boom. The cutting head comprises typically two rotary cutters that are equipped with 5 to 6 rows of cutting bits which, by their rotation, cut and excavate rock. The roadheader moves on tracks. The energy required for rock excavation is supplied by an electrical power supply while the movements of the machine can be done either by a diesel engine or by an electric motor. Roadheaders are very flexible and mobile and can excavate various sizes and shapes of openings.

Removal of the excavated rock can be carried out with trucks or a conveyor belt. A typical roadheader excavation approach is shown in **Figure 7-23**.



Figure 7-22: Roadheader

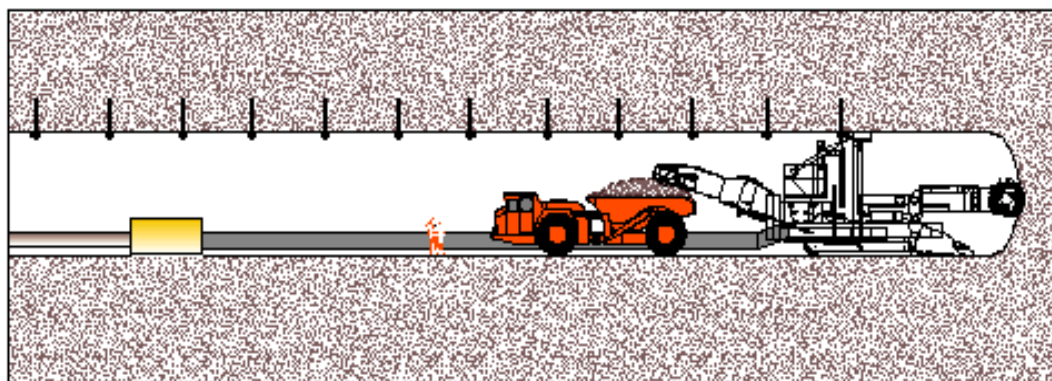


Figure 7-23: Typical Roadheader Excavation Scheme

The effective use of a roadheader depends largely on the rock type that is to be excavated and the characteristics of the rock mass. Highly abrasive rock types such as sandstone and gneiss with a high content of quartz are typically not suitable for the use of a roadheader due to the abrasiveness of the quartz. A further factor of influence is the strength of the rock; an unconfined compressive strength of 140 MPa is typically considered the upper range for efficiency.

Progress rates are typically influenced by the structural features of the rock mass. A rock that contains discontinuities such as bedding planes and joints which create a blocky rock mass is favorable for the use of a roadheader since the discontinuities provide natural breaking planes in the rock; the roadheader pulls the fractured rock apart rather than cutting through it. Fault zones, water inflow and the requirements for rock support will further impact the efficiency of the roadheader. Overall, progress rates are generally slower compared to TBM excavations.

Issues such as dust from the rock breakage have to be addressed with a proper ventilation system in the tunnel during excavation. A water spraying system can reduce dust.

7.10.3 Tunnel Boring Machine (TBM)

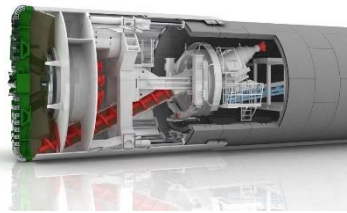

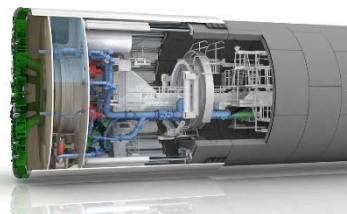
TBMs are available in a range of different machines; the selection of the appropriate machine for a tunnelling project should consider the geotechnical and hydrogeological conditions of the project site. A comparison of the most common machines and their typical applications is provided in **Table 7-13** below (images taken from Herrenknecht [28]). Hybrid (dual or multi-mode) machines that combine the advantages of individual TBMs are available to suit mixed or unpredictable ground conditions. These machines were developed to operate in both open and closed mode to address heterogeneous ground conditions but also withstand high hydrostatic pressures. As discussed above, this type of TBM was successfully used for projects under water pressures of up to 17 bar.

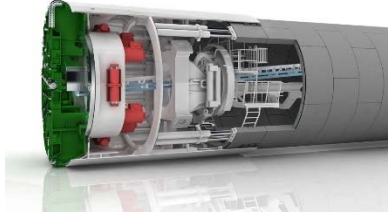
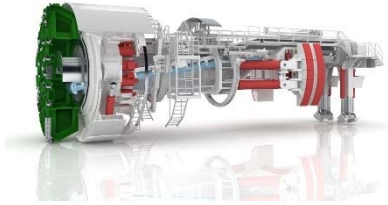
The general advantages of the use of a TBM are the limited disturbance to the surrounding ground and the production of smooth tunnel walls. Consideration must be given to the effects of cutter wear associated with plucking thinly bedded rock from the excavation face and with the abrasivity of the quartz rich gneiss and sandstone formations.

Based on the comparison in **Table 7-13** and pending further geotechnical investigation prior to construction of a tunnel fixed link at the Strait of Belle Isle, the use of a pressurized face machine, for example a Mixshield TBM, is currently considered the most suitable solution.

Mixshield machines are suitable for the expected difficult ground conditions varying between strong rock and potentially weak faults zones with high water inflow, under which condition the machine would operate in closed mode with pressurized face. Considering the tunnel elevation of approximately 130 m below sea level, the machine has to be capable of sustaining the corresponding hydrostatic pressure of 13 bars. Recent projects such as the Eurasia Tunnel project as well as the Lake Mead Tunnel have shown that Mixshield TBMs have the capabilities to address these pressures.

Table 7-13: Overview of Common Tunnel Boring Machines (TBMs)

| | |
|---|---|
| <p>Earth Pressure Balance (EPB) Machine</p> <ul style="list-style-type: none"> • Typically used in soft cohesive ground with low permeability; for all excavation diameters. • Special solutions available for use in heterogenous or unstable ground and increased water pressure. • Excavation under controlled positive face support pressure using soil paste. • Excavated material entering the excavation chamber is turned into paste and used as tunnel face support; controlled pressure conditions in the excavation chamber prevent uncontrolled soil inflow and settlements/heave. • Soil enhancement possible with injections of soil conditioning material into the surrounding ground. • Tunnel lining/rock support consists of pre-cast concrete segments installed behind the machine. • Machine advances with the use of hydraulic cylinders that push the machine forward from the previously installed concrete liner ring. |  |
| <p>Slurry Machine</p> <ul style="list-style-type: none"> • Typically used in all ground conditions, including complex ground and high permeability; typically for small to medium excavation diameters (microtunnelling) • Excavation under controlled positive face support pressure using slurry suspension. • Hydraulic tunnel face support to control difficult geology and high water pressure; controlled pressure conditions in the excavation chamber stabilize the tunnel face, prevent uncontrolled soil inflow and settlements/heave. Hydraulic pressures of up to 17 bar have been addressed in recent projects. • Tunnel lining/rock support consists of pre-cast concrete segments installed behind the machine. • Machine advances with the use of hydraulic cylinders that push the machine forward from the previously installed concrete liner ring. |  |
| <p>Mixshield Machine</p> <ul style="list-style-type: none"> • Typically used in heterogenous ground conditions with high permeability and high water pressure; for all excavation diameters. • Excavation under controlled positive face support pressure using slurry suspension and pressurized air cushion. • Hydraulic tunnel face support to control difficult geology and high water pressure; controlled pressure conditions in the excavation chamber stabilize the tunnel face, prevent uncontrolled soil inflow and settlements/heave. • Tunnel lining/rock support consists of pre-cast concrete segments installed behind the machine. • Machine advances with the use of hydraulic cylinders that push the machine forward from the previously installed concrete liner ring. |  |

| | |
|---|---|
| <p>Single Shield Machine</p> <ul style="list-style-type: none"> • Typically used in soft to hard rock or stable, non-groundwater bearing soil; for all excavation diameters. Shielded and sealed machine not usable in high water pressure conditions. • Excavation under atmospheric pressure. • Machine can be protected against jamming in the rock, for example in squeezing rock, if design accounts for an excavation diameter larger than shield skin diameter. • Can be equipped for use in rock with high water inflow; an injection system allows drilling and grouting of the rock ahead of the excavated face to control water inflow. • Shielding provides great safety for workers and machine and provides improved support of weak ground until initial support can be installed. • Tunnel lining/rock support consists typically of pre-cast concrete segments installed behind the machine. • Machine advances with the use of hydraulic cylinders that push the machine forward from the previously installed concrete liner ring. |  |
| <p>Open Face (Main Beam) Machine</p> <ul style="list-style-type: none"> • Typically used in hard, stable rock with medium to high rock strength; suited for short faulted zones; for all excavation diameters. Usable in high water pressure conditions. • Excavation under atmospheric pressure. • Low potential of getting stuck in squeezing or swelling ground. • No segmental lining; excavated rock walls are unsupported; in fractured rock, immediate rock support can be installed behind the machine. • Flexible for installation of optional or additional rock support/reinforcement elements, thus minimizing ground displacements and stabilization requirements. • Can be equipped for probing and grouting ahead of the tunnel face if required in poor rock conditions or high water inflow • Tunnel lining/support with rock anchors, ring beams, shotcrete, etc. and/or subsequent concrete liner. • Machine advances with the use of hydraulic cylinders that push the machine forward from grippers that push against the excavated tunnel walls. • Tunnelling progress depends on the time for installation of rock support. High advance rates in good quality rock masses. • Roof shield above the unit behind the cutterhead provides protection for workers and machine against breaking rock. |  |

8. Updated Assessment of Preferred Fixed Link Option

The updated assessment focusses on the preferred option identified in the 2004 study to evaluate if the previous assessment is still valid or if the preferred option should be revised. Based on economic and technical considerations, the preferred option as defined in the 2004 study was a single-track rail tunnel excavated by TBM. Alternatively, a single-lane road tunnel excavated by TBM could be considered.

8.1 Updated Risk Assessment

As in 2004, construction risks and operational risks were considered for the risk assessment of the fixed link options. The main hazards that contribute to the risks associated with the Strait of Belle Isle fixed link options emerge from the difficult environment at the project location. The hazards include deep water, icebergs and sea ice, marine traffic, long winter season, snow and ice, high winds and fog but also the difficult ground conditions. Other hazards that relate to the operation of the fixed links include events such as accidents, vehicle breakdown and fire. The risks associated with these hazards result from the probability that a hazard occurs and the potential consequences. For this study, only a high-level risk assessment was undertaken. A detailed risk assessment should be conducted for the preferred option at a later project stage.

8.1.1 Construction Risks

TBM technology has continued to advance in the past years to address challenging ground conditions and high hydrostatic pressures and, compared to 2004, the general risk associated with TBM excavations has decreased due to experience gained from numerous large tunnel projects. However, certain risks related to the geotechnical conditions, including locally poor ground conditions, high water pressures and increased water inflows, remain and need to be addressed during tunnelling.

The use of an open face machine allows the installation of rock support adjusted to the specific requirements for individual tunnel sections. The machine also allows probing and grouting ahead of the tunnel face to address unstable ground and control water inflow.

The recent advances in the development of dual mode TBMs has led to machines that are capable to operate under high hydrostatic pressures of up to 17 bar which is higher than the pressures expected for tunneling underneath the Strait of Belle Isle. A TBM designed to operate in different modes, such as a mixshield machine to address unstable ground and high water inflow/pressure combined with a hard rock TBM (as was used for deep tunneling under Lake Mead with water pressure up to 17 bar), can reduce the risk associated with the water inflow and unstable ground while also enabling the excavation of competent hard rock. The use of a dual mode machine would require the installation of a segmental concrete lining.

Construction risks also include that the machine can get stuck due to time-dependent rock deformations (such as squeezing or swelling ground) or due to mechanical problems. A potentially required recovery of the TBM through a shaft drilled from ground surface would not be possible at the Strait of Belle Isle.

Construction risks associated with drill & blast tunnel excavation relate mainly to the geotechnical conditions, including the expected faults zones with poor, fractured rock and high water inflows. A tunnel alignment at 50 m depth below the seabed would reduce the potential inflows due to the expected decrease of rock permeability in greater depths. However, a lower alignment also increases the hydrostatic pressure in the tunnel if faults extend to the seabed. Significant water inflow can be controlled only by grout injections into the surrounding rock; poor rock conditions must be addressed with appropriate support measures.

Considering that the impact of the excavation on the surrounding ground is high with drill & blast techniques, a deeper alignment would also allow for a greater rock cover above the tunnel to mitigate the risk associated with reduced crown thickness due to overbreak and rockfall from the tunnel roof. However, a deeper alignment also increases the length of the tunnel and hence the construction costs.

Mechanical tunnel excavation using a roadheader would need to address the same risks associated with geotechnical conditions as drill & blast tunnels. However, the impact of a roadheader excavation on the surrounding rock is significantly less than with drill & blast operations.

8.2 Updated Cost Estimate

8.2.1 Capital Costs Estimate

As in 2004, the 2017 updated estimate for the construction costs of a TBM bored tunnel was carried out using the Hatch tunnel estimating database (TED 2001). This database contains up-to-date information on labour and equipment requirements and advance rates from similar TBM bored tunnelling projects which were used in the development of a contractor style estimate. Where applicable, labour rates for Newfoundland were used. The costs include the connecting roads on each side of the Strait (but not the costs for a new highway along the north shore of the Gulf of St. Lawrence nor any upgrade of Highway 430), and the marshalling areas.

To allow a comparison of the costs, the 2017 construction cost estimates include the same assumptions as in 2004. In summary, the principal assumptions used in preparation of the estimates comprise the following:

- Earth pressure balance or mixshield type TBM, for the one TBM option, used to mine the tunnel starting from the Newfoundland side of the Strait or, for the two TBM option, one machine would launch from Newfoundland and one machine would launch from Labrador.
- Bolted precast concrete linings installed behind the TBM as it advances. The concrete elements were assumed to be manufactured in the Province of Newfoundland and Labrador.
- Labour conditions and wages as outlined in the collective agreement between the Construction Labour Relations Association of Newfoundland and Labrador Inc (CLRA)

and LIUNA, Local 1208, Construction, Rock and Tunnel and General Workers Union, June 2017.

- TBM advance rates of 14 m per day and 17.7 m per day where rock conditions are good, for highway and railway tunnels respectively.
- TBM advance rates of 5.4 m per day and 7.1 m per day in faulted zones which are assumed to occur over a 1400 m length of the tunnel alignment, for highway and railway tunnels respectively.
- 8-week long learning curve at commencement of tunnelling where advance rates are 50% of those achieved when the crews are experienced.
- 40% contingency applied to civil elements of the work.
- 20% contingency applied to mechanical and electrical elements of the work.
- 15% contractor's overheads and profit applied to the estimate.
- 17% applied to the overall estimate for design, construction management, and owner's costs.
- \$20 to \$23 million for feasibility study and environmental assessment.

The construction costs for the single bore road tunnel (1 TBM) option is estimated to be \$2,064 million and the costs for the single bore rail tunnel (1 TBM) is estimated to be \$1,675 million, both in 2017 dollars. A detailed cost breakdown of both estimates is provided in Appendix C.

The construction costs for the rail tunnel option include estimates for three shuttle trains.

Camp cost and duration are not included in the estimated costs above and should be considered in the more detail phase.

8.2.2 Operating & Maintenance Costs (O&M Costs)

At a pre-feasibility study level, the O&M costs will be similar for all three tunnel concepts (single road tunnel, twin road tunnels, and single rail tunnel).

8.2.2.1 Road Tunnel

As in 2004, the updated O&M expenditures associated with the operation of a road tunnel can be categorized as follows:

- Management and operation of the tunnel control building.
- Traffic supervision costs. Closed circuit television would be used for monitoring of the tunnels, with full time monitoring taking place at the surface within the tunnel control building (costs doubled for twin tunnels).
- Emergency truck costs. Required for the removal of broken down vehicles from the tunnel, and for carrying emergency fire fighting equipment. One emergency truck will be required at each portal, and these should be stationed close to the portals but away from

the traffic flow. Fully trained operators and an assistant will be required for each truck on a 24-hour basis (same costs for single tunnel and for twin tunnels).

- Energy costs associated with the operation of the control centre, tunnel vehicles, tunnel ventilation equipment, and drainage pumps (costs doubled for twin tunnels).
- Electrical maintenance costs associated with the inspection and maintenance of the power distribution system (includes switchgear, transformers, wiring, and cabling), tunnel lighting, communication and signal systems (costs doubled for twin tunnels). A platform truck is required for maintenance of the tunnel lighting system (same cost for single tunnel and for twin tunnels).
- Mechanical maintenance costs associated with the inspection and maintenance of the tunnel ventilation system, emergency diesel generators, and drainage pumps (costs doubled for twin tunnels).
- Structure maintenance costs associated with the inspection, cleaning, and maintenance of the roadway, drainage sumps, and tunnel structure. A street cleaner vehicle is required for cleaning of the roadway, and a washing truck for frequent cleaning of the tunnel walls and soffit (costs doubled for twin tunnels).

Estimated operating and maintenance costs for the single road tunnel option are \$7.7 million per annum and for the twin road tunnel option \$12.1 million per annum, both in 2017 dollars. Appendix D1 provides a breakdown of these costs.

8.2.2.2 *Rail Tunnel*

As in 2004, the updated O&M expenditures associated with the operation of a rail tunnel can be categorized as follows:

- Emergency rescue vehicles cost: (Rail Tunnel) Required for rescue of passengers during an emergency event. One rail mounted diesel powered rescue vehicle will be required at each portal. Fully trained operators will be required for each rescue vehicle on a 12-hour daily basis (same costs for single tunnel and for twin tunnels). Energy costs associated with the operation of tunnel lighting, equipment, and drainage pumps. Most of these costs will be for the supply of electrical power and would, therefore, vary with fluctuations in market costs for electricity.
- Electrical maintenance costs associated with the inspection and maintenance of the power distribution system (includes switchgear, transformers, wiring, and cabling), tunnel lighting, communication and signal systems.
- Mechanical maintenance costs associated with the inspection and maintenance of the tunnel ventilation system, emergency diesel generators, and drainage pumps.
- Structure maintenance costs associated with the inspection, cleaning and maintenance of the permanent way, drainage sumps, and tunnel structure.
- Staffing costs associated with loading vehicles on to the shuttle, three staff per shoreline.

Estimated operating costs for the rail tunnel alone are \$1.0 million per annum in 2017 dollars. The estimated operating costs for the train shuttle are \$7.7 million for a total of \$8.7 million per annum in 2017 dollars. A breakdown of these costs is provided in Appendix D2.

8.2.3 Contingency

For tunnelling projects, typical contingencies at a pre-feasibility stage range from 40% to 75%. The selection of contingency level is particularly related to the existence or lack of geotechnical investigation. Typically, at this stage no investigation has taken place. For this project, a contingency of 40% has been chosen for the tunnel (civil) costs based on the availability of much improved geotechnical information due to the geotechnical investigations and surveys that have taken place for the Nalcor Energy project. In addition, the use of a TBM reduces the sensitivity to unpredicted ground conditions and it is, therefore, justifiable that the lower end of the range of contingency levels is used.

8.3 Cost Estimate Classification

The cost estimate prepared for the Conceptual Design Submittal is a Class 4 estimate as defined by the Association for the Advancement of Cost Engineering (AACE) [26]. As shown in **Figure 8-1**, an AACE Class 4 estimate is considered accurate to +30% to -20% based on:

- Level of Project Definition – Conceptual Design
- Methodology
- Preparation Effort

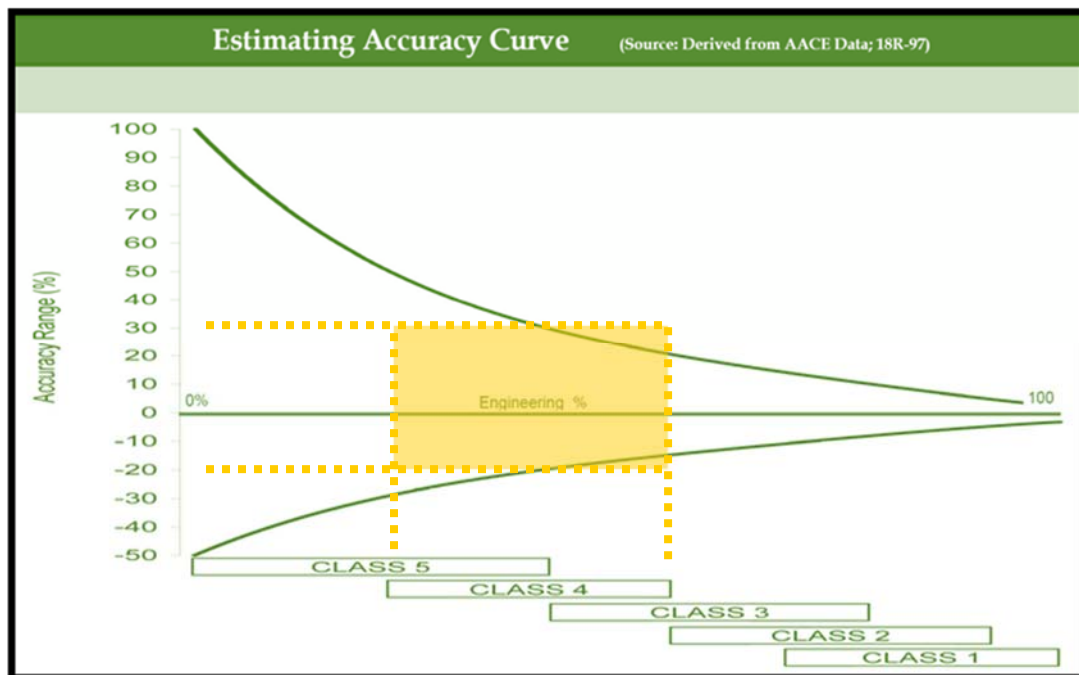


Figure 8-1: Estimate Accuracy

Figure 8-2 provides the cost estimate accuracy matrix that includes details regarding the specific characteristics of the classification levels such as maturity level of a project, purpose of cost estimate and associated accuracy ranges.

| ESTIMATE CLASS | Primary Characteristic | Secondary Characteristic | | |
|----------------|---|---|--|---|
| | MATURITY LEVEL OF PROJECT DEFINITION DELIVERABLES <small>Expressed as % of complete definition</small> | END USAGE <small>Typical purpose of estimate</small> | METHODOLOGY <small>Typical estimating method</small> | EXPECTED ACCURACY RANGE <small>Typical variation in low(L), and high(H) ranges^[a]</small> |
| Class 5 | 0% to 2% | Conceptual planning | Capacity factored, parametric models, judgment, or analogy | L: -20% to -50% H: +30% to +100% |
| Class 4 | 1% to 15% | Screening options | Equipment factored or parametric models | L: -15% to -30% H: +20% to +50% |
| Class 3 | 10% to 40% | Funding authorization | Semi-detailed unit costs with assembly level line items | L: -10% to -20% H: +10% to +30% |
| Class 2 | 30% to 75% | Project control | Detailed unit cost with forced detailed take-off | L: -5% to -15% H: +5% to +20% |
| Class 1 | 65% to 100% | Fixed price bid check estimate | Detailed unit cost with detailed take-off | L: -3% to -10% H: +3% to +15% |

Notes: [a] The state of technology, availability of applicable reference cost data and many other risks affect the range markedly. The +/- values represent typical percentage variation of actual costs from the cost estimate after application of contingency (typically at a 50% level of confidence) for given scope.

Figure 8-2: Cost Estimate Classification Matrix [26]

8.3.1 **Markups**

General contractor profit was applied at 15% based on industry averages for tunnel construction.

Indirect costs are generally inclusive of items such as supervision, survey, vehicles, quality assurance, insurance, safety, site office expenses, ablutions and miscellaneous costs. The calculated indirect costs for the project are equivalent to 12% of the direct costs.

Estimate contingency was applied to the aggregate of direct costs, indirect costs, and profit at 15% per AACE Class 4 standards based on the estimate level of effort and design percent completion.

8.3.2 **Escalation Rate**

Tunnel Work: Labour rates were not escalated from effective wages 2017 to the anticipated construction midpoint, 2024.

Surface Work: Unit costs were applied per the following:

- ENR September 2017 index value basis: 10823
- A final escalation of the unit costs will be performed for the final 100% estimate based on published cost data available closer to the time of bid.

8.3.3 **Cost Resources**

Cost data was obtained from the following resources and utilized where appropriate based on the estimate methodology for individual cost components:

- Tunnel & Shafts:
 - Hatch Proprietary Software - Tunnel Estimating Database (TED)
 - Historical project data
 - Vendor Materials / Equipment Quotations
 - Newfoundland and Labrador 2017 Prevailing Wages
- Surface Work:
 - R.S. Means
 - Historical Bid Data
 - Vendor Quotes on Equipment and Materials where appropriate

8.4 **Updated Construction Schedule**

A major factor in the schedule for a TBM bored tunnelling operation is the procurement of the TBM and manufacture and supply of precast concrete lining rings in advance of commencement of tunnelling operations. Experience on other major projects has demonstrated that TBM procurement / manufacture can typically take up to 15 months, and such a period has been assumed for the scheduling of the tunnel options.

The updated schedule for construction of the single road tunnel option comprises 14 years, including assumed 3 years for planning activities and 2 years for tunnel design. This schedule assumes that the tunnelling work would be undertaken by a single TBM operating from the Newfoundland side.

The schedule for construction of the railway option was assumed to be similar to that for the road tunnel with an additional 11 months because of the greater tunnelling length due to the flatter exit ramp grades. Using a single TBM results in an overall construction schedule of 15 years.

The updated schedules are shown in **Figure 8-3** and **Figure 8-4** below for the single bore road and rail tunnel option, both assuming the use of one TBM. The schedules for all road and rail tunnel options included in this report are provided in Appendix E.

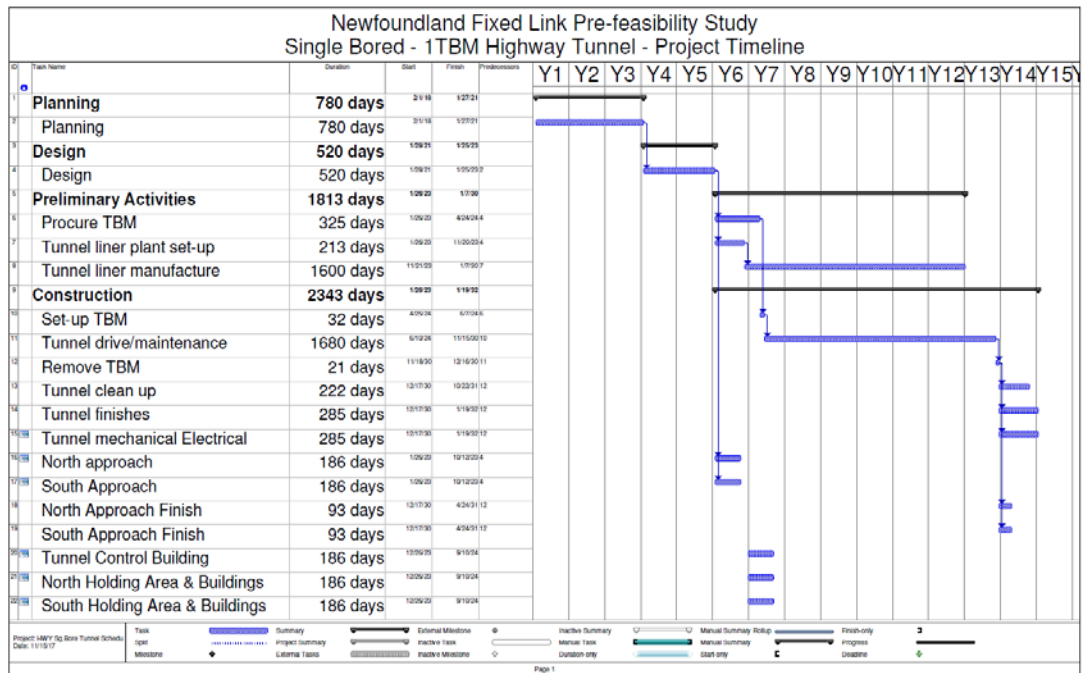


Figure 8-3: Project Schedule for TBM bored Road Tunnel (1 TBM)

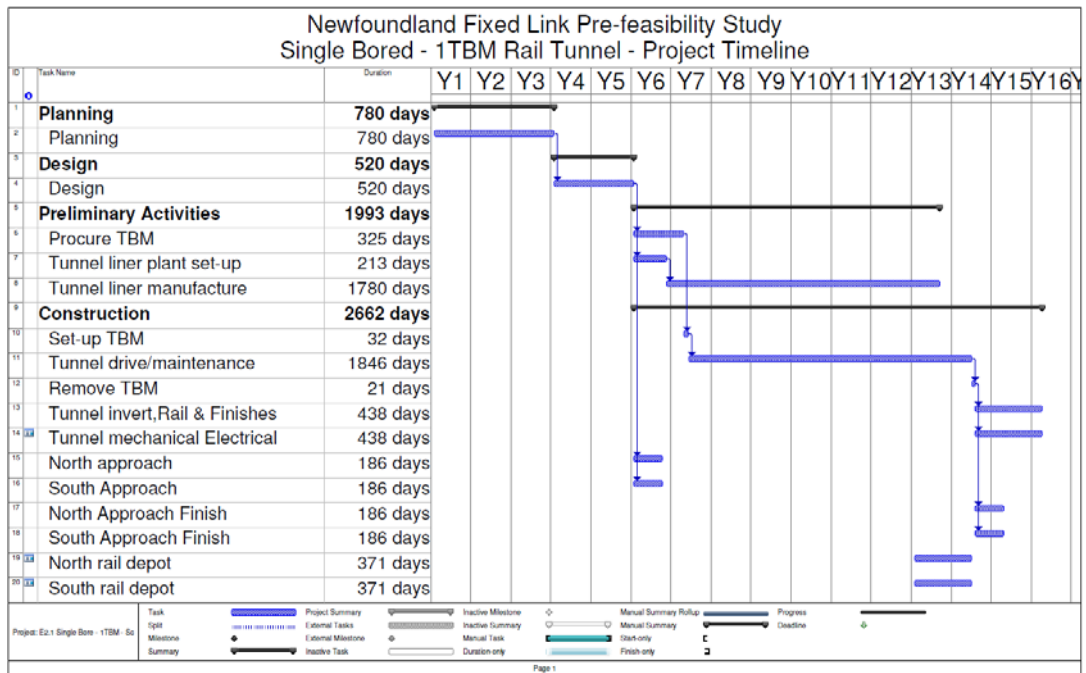


Figure 8-4: Project Schedule for TBM bored Rail Tunnel (1 TBM)

8.5 Cost and Schedule Comparison

In the following **Table 8-1** and **Table 8-2** the 2004 and 2017 costs and schedules are summarized for comparison. As in 2004, the 2017 schedule comprises 3 years of planning and 2 years of detailed design for a total of 5 years prior to start of construction. For all options, the three years of planning were allowed for additional studies, field investigations, environmental assessment and other planning activities. Costs and schedule details for all road and rail options are provided in the Appendices C, D and E.

Table 8-1: Comparison of Fixed Link Road Options

| Option | Tunnel Dimensions | Construction Costs | | Annual Operating Cost | | Project Duration (years) | |
|---|---------------------------------------|--------------------|----------|-----------------------|----------|--------------------------|------|
| | | \$M-2004 | \$M-2017 | \$M-2004 | \$M-2017 | 2004 | 2017 |
| TBM Single Bore Tunnel 1 TBM (7days/week) | 11 m Ø 21 km Length | 1,559 | 2,064 | 6.8 | 7.7 | 12.2 | 14 |
| TBM Single Bore Tunnel 2 TBM (7days/week) | 11 m Ø 21 km Length | N/A | 2,162 | N/A | 7.7 | N/A | 10.8 |
| TBM Twin Bore Tunnel 1 TBM per Bore | 11 m Ø 21 km Length (each bore) | N/A | 3,967 | N/A | 12.1 | N/A | 14 |

Table 8-2: Comparison of Fixed Link Rail Options

| Option | Tunnel Dimensions | Construction Costs | | Annual Operating Cost | | Project Duration (years) | |
|---|-------------------------|--------------------|----------|-----------------------|----------|--------------------------|------|
| | | \$M-2004 | \$M-2017 | \$M-2004 | \$M-2017 | 2004 | 2017 |
| TBM Single Bore Tunnel 1 TBM (7days/week) | 7.6 m Ø 30 km Length | 1,144 | 1,675 | 7.6 | 8.7 | 12.5 | 15.5 |
| TBM Single Bore Tunnel 2 TBM (7days/week) | 7.6 m Ø 30 km Length | 1,184 | 1,764 | 7.6 | 8.7 | 10.7 | 12 |

The graph in **Figure 8-5** illustrates the capital costs for the various options reviewed in this study in comparison with the capital cost estimates from the 2004 study.

Based on the Construction Cost Index published by the Engineering News Record (ENR), a ratio of 1.54 applies when comparing costs in 2017 to costs in 2004. Comparing the cost for the rail tunnel of \$1,144M in 2004 and \$1,675M in 2017 results in a ratio of 1.46, which is similar to the ENR ratio.

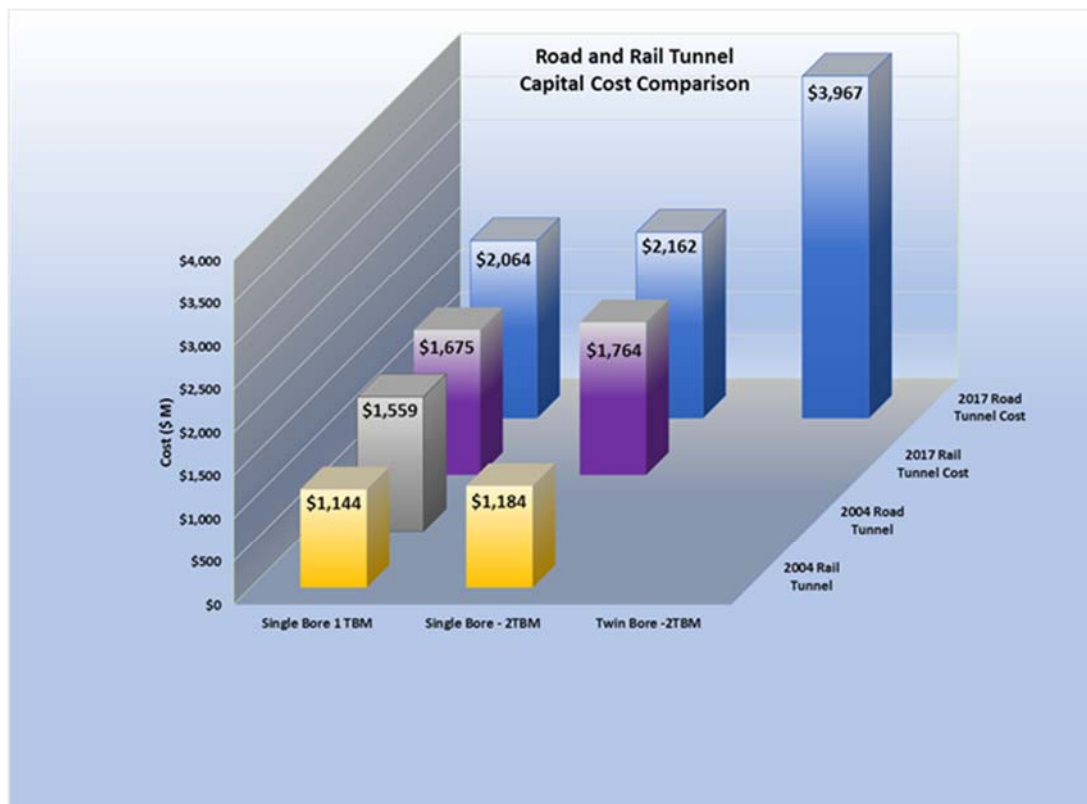


Figure 8-5: Capital Costs Comparison

The increased project duration time compared to the 2004 schedule for both the road and the rail tunnel is the result of the increased lengths of the tunnel exit ramps which are caused by the greater depth below the sea bed of the tunnel, resulting in longer excavation times. For the road tunnel, a total alignment length of 21 km was assumed; the rail tunnel was assumed to have a total length of 30 km. A refinement of the recommended tunnel depth below sea bed and a detailed study of the most suitable alignment location of the tunnel exit ramps at a later project stage will impact the tunnel costs as well as the schedule. For the purpose of this study update, the exit ramps were assumed to be in line with the tunnel which resulted in long ramps due to the surface topography at the shores. An adjustment of the tunnel exit ramps to follow a more suitable topography will most likely reduce the ramp lengths and hence the tunnelling schedule.

8.6 Recommended Option

Based on the assessment of capacity requirements, construction and operational risks as well as the updated construction and operational cost estimates and construction schedules, a single-track rail tunnel excavated with a TBM is still considered the most suitable option to address the transportation requirements at the Strait of Belle Isle.

9. Regulatory Requirements and Environmental Implications

Regulatory requirements and environmental implications and the required process of obtaining approval for a fixed link tunnel project were discussed in the Pre-Feasibility Study in 2004 and a list of the applicable legislation, standards and permits on federal, provincial and municipal level was provided (refer to 2004 Pre-Feasibility Study [1]). A complete assessment of the requirements and implications would be undertaken as the project moves forward.

Potential environmental implications related to the construction of a tunnel under the Strait of Belle Isle would have to be addressed in an Environmental Assessment. Compared to surface fixed link alternatives, no to very little impact on the environment in the Strait of Belle Isle is associated with the construction of a subsurface fixed link. Potential impact of the tunnel construction on marine life could be related to noise and vibrations during construction.

An impact of the tunnel construction and operation on the environment is expected on the onshore portions of the tunnel, including the areas for tunnel approaches, tunnel portals, parking spaces and amenities for travellers during waiting times, marshalling areas, etc. This impact would be significantly larger for a rail tunnel than a road tunnel due to the required terminal facilities, loading areas, etc.

In addition, the required construction of roadways to connect the tunnel approaches with the highway system should be considered. Implications concern, for example:

- Impact of emissions during construction such as noise, dust and exhaust fumes on workers, wild life and vegetation.
- Transport and disposal of excavated material.
- Impact of land use for required surface construction on existing vegetation or animal habitats or on potential rare species or archaeological sites.

The impact of the changing environmental conditions on the fixed link, which include in particular the observed tendency of less sea ice coverage across the Strait of Belle Isle over past years but also the tendency for a larger number of icebergs floating through the Strait as noted in past years, affect mostly surface options. The subsurface tunnel options would not be affected by changing ice and iceberg conditions in the Strait.

10. Ferry Service across the Strait of Belle Isle

As requested by the client, an update of the upgraded Strait of Belle Isle ferry crossing was carried out to account for an anticipated increased need for passenger transportation across the Strait as an alternative to a fixed link implementation.

10.1 Existing Ferry Service

The existing ferry service across the Strait of Belle Isle between Blanc-Sablon (QC) and St. Barbe (NL) is currently provided by Labrador Marine Inc. The ferry service includes passengers and vehicles only and is maintained with the MS Apollo, a Ro-Ro/Passenger ship that is certified for 85 vehicles and 240 passengers.

The ferry service between Blanc-Sablon and St. Barbe is currently provided year-round with reduced services during the winter months. An alternative route between Blanc-Sablon and Corner Brook (NL) is used in the event that St. Barbe becomes inaccessible due to ice conditions.

The current schedule (**Table 10-1**) provides 38 one-way crossings per week during the summer months from mid June to mid September (19 crossings each from Blanc-Sablon and from St. Barbe). The fall and spring schedule of 28 one-way crossings per week (14 crossings in each direction) applies from mid September to early January and from late April to mid June. The winter schedule includes 14 one-way crossings per week (7 in each direction) between early January and late April. Dangerous goods transports are carried out once per week from spring to fall; passenger numbers are limited on crossings that carry dangerous goods. In total, the 2017 schedule includes approximately 1374 one-way crossings (687 in each direction).

The crossing time for the 36 km long route between Blanc-Sablon and St. Barbe is 1 hour 45 minutes. The crossing time for the alternative winter route between Blanc-Sablon and Corner Brook is 12 hours (for 302 km).

Table 10-1: Strait of Belle Isle Ferry Crossings (2017 Schedule)

| | Winter | Spring | Summer | Fall |
|----------------------------|-----------------------------|--------------------------------|--------------------------------|--------------------------------|
| Crossings (one-way) | 15 weeks; Jan 8 - Apr 22 | 8 weeks; Apr 23 - Jun 18 | 12.5 weeks; Jun 19 - Sep 13 | 16.5 weeks; Sep 14 - Jan 7 |
| Per Week: | 14 7 per direction | 28 14 per direction | 38 19 per direction | 28 14 per direction |
| Per Day: | 2 1 per direction | 3 to 5 1 to 3 per direction | 5 to 7 2 to 4 per direction | 3 to 5 1 to 3 per direction |

10.2 Upgraded Ferry Service

Options to upgrade the existing ferry services include:

- Increased numbers of crossings per day using the existing vessel. This option requires longer operating hours per day.

- Increased numbers of passengers per crossing. This option requires a larger ship with a greater capacity for transporting passengers and vehicles.
- Increased numbers of crossings per day and reduced waiting time using two vessels. This option requires the acquisition of a second vessel.
- Crossings during winter months to provide year-round services. This option is already implemented with one daily return crossing between January and April. An alternative route is provided between Blanc-Sablon and Corner Brook during adverse ice conditions in St. Barbe harbor. However, the winter crossings are impacted by the sea ice conditions which can lead to a temporary suspension of the ferry service. This was experienced for example in March 2015 when the ferry was trapped in ice for 10 days in the St. Barbe harbor; in April 2017, the ferry remained docked in Blanc-Sablon due to extreme sea ice conditions in the Strait.

Assuming ferry services with a single vessel with scheduled sailings at 5 hour intervals in each direction and start of services at 6am during the summer season and at 8am from fall to spring, an upgraded schedule as summarized in **Table 10-2** was developed. The total number of crossings would increase from 1374 (2017 schedule) to 1789 one-way crossings (increase of 30%). A traveler would experience crossing times between approximately 2 hours (1 hour 45 min travel time plus time to disembark) and 7 hours if the traveler misses a sailing.

Table 10-2: Upgraded Ferry Schedule (One Vessel)

| | Winter | Spring | Summer | Fall |
|----------------------------|----------------------------|-----------------------------|------------------------------|-----------------------------|
| Crossings (one-way) | 15 weeks Jan 8 - Apr 22 | 8 weeks; Apr 23 - Jun 18 | 13 weeks; Jun 19 - Sep 13 | 16 weeks; Sep 14 - Jan 7 |
| Per Week: | | | | |
| 2017 Schedule | 14 | 28 | 38 | 28 |
| Upgraded Schedule | 21 | 35 | 49 | 35 |
| Per Day: | | | | |
| 2017 Schedule | 2 | 3 to 5 | 5 to 7 | 3 to 5 |
| Upgraded Schedule | 3 | 5 | 7 | 5 |

Assuming ferry services with two vessels with scheduled sailings at 2.5 hour intervals in each direction and similar service hours as above, the total number of one-way crossings would increase from 1374 (2017 schedule) to 3578 (increase of 60%). Crossing times would be between approximately 2 hours (1 hour 45 min for the travel plus time to disembark) and 4.5 hours if the traveler misses a particular sailing.

Note that the upgraded ferry schedule does not account for actual arrival times and vehicle types arriving at a certain time. This information was not provided to Hatch. A more detailed study of vehicle arrivals and types would be required to evaluate required capacities throughout the day and year.

11. Economic Analysis - Rail Tunnel Option

This section sets out the economic and business case analysis. Two traffic cases are described, both for a tunnel option with a railway shuttle:

- The Base Case with 40% diversion of traffic from the Gulf Ferries.
- The Higher Traffic Case with 60% diversion of traffic from the Gulf Ferries.

The assumed diversion percentages were provided by the Harris Centre.

Combined with these two traffic cases, two construction cases are addressed:

- Construction by one TBM operated from the Newfoundland side of the Strait, resulting in a construction period of 10 years.
- Construction by two TBMs, one operated from the Newfoundland side and one operated from the Labrador side of the Strait, resulting in a construction period of 7 years.

For both construction cases, the planning and design period is assumed to be 5 years. The operating period for the one TBM option is 30 years; the operating period for the option using two TBMs is 33 years. This results in economic life cycles of 45 years which were studied for both construction cases.

11.1 Traffic Prediction

The traffic predictions are based on the following assumptions:

1. All present traffic using the Strait of Belle Isle Ferry will divert to the new Fixed Link facility.
2. Present traffic for the peak operating period for 2016 is presented in the following **Table 11-1**.

Table 11-1: Present Traffic on Belle Isle Ferry

| Strait of Belle Isle Ferry | 2016 (May to October) |
|----------------------------|-----------------------|
| Passengers | 83,815 |
| Passenger Vehicles (TEU) | 35,798 |
| Commercial Vehicles (TEU) | 18,079 |

3. A one-time surge of traffic of 30% for the opening year has been assumed to reflect the all-year round operation of the new facility and the increased frequency of service.
4. Traffic diversion from the Gulf Ferries has been analyzed based on 40% and 60% diversion rates from the present traffic.
5. The present traffic for the Gulf Ferries is based on the following **Table 11-2** that summarizes the private and commercial traffic data presented in **Table 7-4**, **Table 7-5** and **Table 7-6** (refer to Section 7.4).

Table 11-2: Present Traffic on Gulf Ferries

| Gulf Ferries | 2016 |
|---------------------------|---------|
| Passengers | 328,528 |
| Passenger Vehicles (TEU) | 130,530 |
| Commercial Vehicles (TEU) | 267,274 |

6. Growth factors have been applied to the traffic data as follows:

- Strait of Belle Isle Ferry – all traffic +2.5% per annum
- Gulf Ferries – passengers -1.0% per annum
- Gulf Ferries – passenger vehicles -1.0% per annum
- Gulf Ferries – commercial vehicles +1.6% per annum

Based on these assumptions, the resulting traffic predictions for the first year of operation (Year 16 for 1 TBM and Year 13 for 2 TBMs) for the investigated diversion rates and construction alternatives are presented in **Table 11-3**.

Table 11-3: Predicted Traffic for First Operating Year of Fixed Link

| | 1 TBM | | 2 TBMs | |
|---------------------|---------|---------|---------|---------|
| | 40% | 60% | 40% | 60% |
| Diversion Rate | | | | |
| Passengers | 276,567 | 331,953 | 268,120 | 325,201 |
| Passenger Vehicles | 114,824 | 136,830 | 111,115 | 133,795 |
| Commercial Vehicles | 175,788 | 245,802 | 166,723 | 233,481 |

11.2 Tolls and Revenue Forecasts

It has been assumed that the tolls for the Fixed Link will align with the tolls for the existing Strait of Belle Isle Ferry. Blended rates have been developed for passengers, passenger vehicles, and commercial vehicles, based on the distribution of ferry user types and related tolls for the existing Strait of Belle Isle Ferry traffic. The resulting toll rates divided by ferry user types are presented in 2017 dollars in **Table 11-4**. The rate development as well as sensitivity tests addressing toll rates at 150% and 200% of the present ferry rates are shown in detail in Appendix F,

Table 11-4: Blended Toll Rates for the Fixed Link

| | Tolls (\$2017) |
|---------------------|----------------|
| Passengers | \$11.30 |
| Passenger Vehicles | \$47.66 |
| Commercial Vehicles | \$120.75 |

Revenue forecasts for each operating year have been developed by multiplying the annual traffic by the appropriate toll value. These revenue forecasts have been developed for each proposed diversion rate and construction case. The revenues and costs are presented in **Table 11-5**.

Table 11-5: Total Revenues and Costs¹⁾

| | Diversion Rate | 1 TBM | 2 TBM |
|------------------------------------|----------------|-------|-------|
| Total Revenue | 40% | 1,309 | 1,433 |
| Total Revenue | 60% | 1,754 | 1,926 |
| Operating Costs | | 261 | 287 |
| Net Revenue (less Operating Costs) | 40% | 1,048 | 146 |
| Net Revenue (less Operating Costs) | 60% | 1,493 | 1,638 |
| Total Subsidy (30 years) | | 240 | 264 |
| Net Revenue with Subsidy Saving | 40% | 1,288 | 1,410 |
| Net Revenue with Subsidy Saving | 60% | 1,733 | 1,902 |
| Total Capital Cost | | 1,675 | 1,764 |

¹⁾ Costs are in \$(2017) millions

The operating costs have been used with a constant yearly amount, with replacements of equipment covered by annual depreciation amounts based on the normal life of the specific equipment. The capital costs have been distributed over the planning, design and construction periods based on experience from other projects of this scale.

The present government annual subsidy provided for the Strait of Belle Isle Ferry has been assumed to be \$8.0 million based on inflating the value provided in 2004. This subsidy would be saved on the opening of the Fixed Link and has been either not included or included in the analysis to present the effect of this saving. It should be noted that this saving would likely be exceeded by the compensatory subsidy increase that would be required on the Gulf Ferries due to the loss of the diverted traffic.

11.3 Economic Evaluation

All costs and revenues provided in this section are expressed in terms of Constant Canadian dollars valued at 2017. Inflation projections and conversions into current dollar values, or nominal currency, are considered in the financial evaluation in Section 12.

Three indicators of economic value are employed: Net Present Value (NPV), Internal Rate of Return (IRR), and Benefit Cost Ratio (BCR). The applications of these indicators are described as follows:

- NPV is calculated using a real social discount rate of 7.5%, which is the recommended rate of the Federal Treasury Board assuming annual inflation at 2.5% over the project time horizon (i.e. 10% nominal). The net cash flow for each year (positive or negative) is discounted to reflect the equivalent value in 2017, and all the years are summed to one total NPV.
- IRR is calculated internally, before cash flows are discounted; it is the effective discount rate that would produce a null NPV.

- BCR is calculated using the same cash flow inputs as the NPV, except that activities which are meant to produce benefits are separated and summed independently from those that are meant to cause cost; the ratio of the summed benefits to the summed costs is the BCR. Values greater than one are desirable and suggest economic justification. Values less than unity are undesirable; however, they are not necessarily conclusive in themselves, without considering external factors (such as benefits to the overall provincial economy, encouragement of local investments, diversification of industry, encouragement of tourism, etc.) that are not quantified in the analysis.

The results of the economic analyses for the rail tunnel option are presented in **Table 11-6**.

Table 11-6: Economic Indicators

| Subsidy | 1 TBM | | | | 2 TBMs | | | |
|---|--------------|-------|----------|-------|--------------|-------|----------|-------|
| | Not included | | Included | | Not included | | Included | |
| Diversion Rate | 40% | 60% | 40% | 60% | 40% | 60% | 40% | 60% |
| Net Present Value (NPV) of Cash Flows (\$ millions) | -677 | -625 | -645 | -593 | -780 | -715 | -739 | -674 |
| Internal Rate of Return (IRR) (%) | -2.05 | -0.52 | -1.18 | -0.16 | -1.83 | -0.33 | -0.99 | -0.34 |
| Benefit/Cost Ratio (BCR) | 0.63 | 0.89 | 0.77 | 1.03 | 0.65 | 0.93 | 0.80 | 1.08 |

The following observations apply to the analysis results shown in **Table 11-6**:

- NPVs are all negative, which places the investigated alternatives in the realm of the public sector because quantifiable benefits are insufficient to sustain viability.
- All IRRs are negative, rendering this indicator not useful for this economic evaluation.
- BCRs are mostly less than unity, but at reasonable levels for public infrastructure, suggesting that project justification would likely have to depend on factors that are external to this study.

12. Financial Analysis - Rail Tunnel Option

12.1 Selection of Key Target Parameters

Based on the preceding discussion, the following key parameters are selected for use in the financial analysis:

The long-term inflation rate of 2.5%, and nominal and real social discount rates of 10% and 7.5% respectively have been introduced and employed in the economic analysis reported previously.

For the purpose of examining debt financing, the rate employed for the base case is 8% real (10.5% nominal), to be applied in amortization calculations over the operating life of 30 years, and including a risk premium because of the nature of the project. Short-term debt to finance construction work is estimated to be 5%.

If the project were to involve equity participation, then an acceptable long-term equity return would be 22.5% pre-tax. This is based on after-tax return on equity of 13.5% (high end, with risk premium), assuming an effective corporate tax rate of 40%. If a project were to be financed 25% with equity, then the blended target rate of return would be 13.5%. If the debt rate were to be reduced by 2%, then the blended target rate of return would drop to 12%.

At the pre-feasibility stage of the project, these parameters are considered in line with the risk and reward expectations of prospective investors. However, internal rates of return below 12% are not likely to attract private capital without significant external favorable considerations.

The project in its entirety will most likely not reach this target rate of return. However, this aspect should be considered in respect of a strategy that would segment the costs and risks to privatize a portion of the project. For example, if the project were to be built with public financing, then the operation could be submitted to tender or some other competitive process to franchise it for pecuniary considerations. The pricing for this would be influenced by the target returns from operations.

12.2 Financial Analysis Results

For the financial analysis, the constant dollar estimates for project costs and revenues were translated into nominal values. In carrying out this conversion, the cost of financing during construction is determined by calculating interest on current year costs plus previous year payments plus interest accumulated in prior years. The project costs translated into current dollars, representing the total cost of the project, are shown in **Table 12-1** for both the longer and shorter construction periods.

Table 12-1: Project Costs in Current \$, IRRs and Suggested Grants Required

| | 1 TBM | 2 TBMs |
|---|--------|--------|
| Total Cost (\$2017) (millions) | 1,675 | 1,764 |
| Total Cost (\$Current) (millions) | 2,188 | 2,213 |
| Interest during Construction (millions) | 578 | 398 |
| Total to Finance (millions) | 2,767 | 2,611 |
| Internal Rate of Return (40%) | +0.41% | +0.62% |
| Internal Rate of Return (60%) | +1.97% | +2.20% |
| Grant required for 8.0% IRR (40%)(millions) | 1,595 | 1,655 |
| Grant required for 8.0% IRR (60%)(millions) | 1,370 | 1,430 |

The analysis results show that the total project costs (Total to Finance) are approximately 50% to 65% higher than the Total Costs (\$2017) because of inflation and interest expense – \$2.6 billion versus \$1.8 billion (for 2 TBMs) or \$2.8 billion versus \$1.7 billion (for 1 TBM). The advantage of a faster construction phase is apparent; a slight cost disadvantage in constant dollar terms becomes an advantage in real world conditions. The difference in the total costs to finance is worth \$156 million.

The total amount to finance is \$2,767 million (for 1 TBM), or \$2,611 million (for 2 TBMs). Incorporating the post operation cash flows, the Internal Rate of Return ranges from 0.41% to 2.2%. These values remain consistent with the conclusion of the economic analysis, that the project is not bankable on its own merits. Proceeding with the project will, hence, require access to free capital made available to suit purposes other than the gain of investment.

Various financing scenarios involving grants or contributions were considered to identify financial conditions under which the project could proceed. The main finding is that a total contribution in the order of \$1.5 billion, or more, would be required for the project to be self-sufficient. For example, to provide a return of 8%, grants ranging from \$1.370 billion to \$1.655 billion would be required. This is still below the private finance threshold considered currently applicable, except perhaps for long-term debt; however, it illustrates the level of public support that is required for advancing the project. It is of interest that the 2 TBMs option has a lower financed construction cost and a longer revenue generating period but still requires a higher grant. This is the result of the proponent being relieved of much of the financing burden under a grant scenario since the construction cost is almost completely funded by the grant. Therefore, the construction financing advantage presented by the 2 TBMs option is negated when a grant is used to cover most of the construction costs as they occur. Hence, the 1 TBM option requires a smaller grant due to its lower overall costs in a mostly finance cost relieved environment.

The public sector involvement is considered the fundamental pre-condition for advancing the project beyond the present pre-feasibility stage. Once the level of support is decided, the models developed for this project can be employed to explore more financing variations and to examine potential revenue and cost relationships.

13. Economic Analysis - Road Tunnel Option

This section sets out the economic and business case analysis for the road tunnel option. Two traffic cases are described:

- The Base Case with 40% diversion of traffic from the Gulf Ferries.
- The Higher Traffic Case with 60% diversion of traffic from the Gulf Ferries.

The assumed diversion percentages were provided by the Harris Centre.

Combined with these two traffic cases, two construction cases are addressed:

- Construction by one TBM operated from the Newfoundland side of the Strait, resulting in a construction period of 9 years.
- Construction by two TBMs, one operated from the Newfoundland side and one operated from the Labrador side of the Strait, resulting in a construction period of 6 years.

For both construction cases, the planning and design period is assumed to be 5 years. The operating period for the one TBM option is 30 years; the operating period for the option using two TBMs is 33 years. This results in economic life cycles of 44 years which were studied for both construction cases.

13.1 Traffic Prediction

The traffic predictions are identical for the road and rail tunnel options (refer to Section 11.1).

13.2 Tolls and Revenue Forecasts

It has been assumed that the tolls for the Fixed Link will align with the tolls for the existing Strait of Belle Isle Ferry. Blended rates have been developed for passengers, passenger vehicles, and commercial vehicles, based on the distribution of ferry user types and related tolls for the existing Strait of Belle Isle Ferry traffic. The resulting toll rates divided by ferry user types are presented in 2017 dollars in **Table 13-1**. The rate development as well as sensitivity tests addressing toll rates at 150% and 200% of the present ferry rates are shown in detail in Appendix G.

Table 13-1: Blended Toll Rates for the Fixed Link

| | Tolls (\$2017) |
|---------------------|----------------|
| Passengers | \$11.30 |
| Passenger Vehicles | \$47.66 |
| Commercial Vehicles | \$120.75 |

Revenue forecasts for each operating year have been developed by multiplying the annual traffic by the appropriate toll value. These revenue forecasts have been developed for each proposed diversion rate and construction case. The revenues and costs are presented in **Table 13-2**.

Table 13-2: Total Revenues and Costs¹⁾

| | Diversion Rate | 1 TBM | 2 TBM |
|------------------------------------|----------------|-------|-------|
| Total Revenue | 40% | 1,289 | 1,411 |
| Total Revenue | 60% | 1,728 | 1,898 |
| Operating Costs | | 229 | 252 |
| Net Revenue (less Operating Costs) | 40% | 1,059 | 1,159 |
| Net Revenue (less Operating Costs) | 60% | 1,499 | 1,646 |
| Total Subsidy (30 years) | | 240 | 264 |
| Net Revenue with Subsidy Saving | 40% | 1,299 | 1,423 |
| Net Revenue with Subsidy Saving | 60% | 1,739 | 1,910 |
| Total Capital Cost | | 2,064 | 2,164 |

¹⁾ Costs are in \$(2017) millions

The operating costs have been used with a constant yearly amount, with replacements of equipment covered by annual depreciation amounts based on the normal life of the specific equipment. The capital costs have been distributed over the planning, design and construction periods based on experience from other projects of this scale.

The present government annual subsidy provided for the Strait of Belle Isle Ferry has been assumed to be \$8.0 million, based on inflating the value provided in 2004. This subsidy would be saved on the opening of the Fixed Link and has been either not included or included in the analysis to present the effect of this saving. It should be noted that this saving would likely be exceeded by the compensatory subsidy increase that would be required on the Gulf Ferries due to the loss of the diverted traffic.

13.3 Economic Evaluation

All costs and revenues provided in this section are expressed in terms of Constant Canadian dollars valued at 2017. Inflation projections and conversions into current dollar values, or nominal currency, are considered in the financial evaluation in Section 14.

As for the rail tunnel option, three indicators of economic value are employed: Net Present Value (NPV), Internal Rate of Return (IRR), and Benefit Cost Ratio (BCR). The applications of these indicators are described as follows:

- NPV is calculated using a real social discount rate of 7.5%, which is the recommended rate of the Federal Treasury Board assuming annual inflation at 2.5% over the project time horizon (i.e. 10% nominal). The net cash flow for each year (positive or negative) is discounted to reflect the equivalent value in 2017, and all the years are summed to one total NPV.
- IRR is calculated internally, before cash flows are discounted; it is the effective discount rate that would produce a null NPV.

- BCR is calculated using the same cash flow inputs as the NPV, except that activities which are meant to produce benefits are separated and summed independently from those that are meant to cause cost; the ratio of the summed benefits to the summed costs is the BCR. Values greater than one are desirable and suggest economic justification. Values less than unity are undesirable; however, they are not necessarily conclusive in themselves, without considering external factors (such as benefits to the overall provincial economy, encouragement of local investments, diversification of industry, encouragement of tourism, etc.) that are not quantified in the analysis.

The results of the economic analyses for the road tunnel option are presented in **Table 13-3**.

Table 13-3: Economic Indicators

| Subsidy | 1 TBM | | | | 2 TBMs | | | |
|---|--------------|-------|----------|-------|--------------|------|----------|-------|
| | Not included | | Included | | Not included | | Included | |
| Diversion Rate | 40% | 60% | 40% | 60% | 40% | 60% | 40% | 60% |
| Net Present Value (NPV) of Cash Flows (\$ millions) | -919 | -863 | -884 | -829 | -1,041 | -972 | -997 | -928 |
| Internal Rate of Return (IRR) (%) | -2.88 | -1.43 | -2.06 | -0.79 | -2.63 | -1.2 | -1.83 | -0.56 |
| Benefit/Cost Ratio (BCR) | 0.51 | 0.73 | 0.63 | 0.84 | 0.54 | 0.76 | 0.66 | 0.88 |

The following observations apply to the analysis results shown in **Table 13-3**:

- NPVs are all negative, which places the investigated alternatives in the realm of the public sector because quantifiable benefits are insufficient to sustain viability.
- All IRRs are negative, rendering this indicator not useful for this economic evaluation.
- BCRs are mostly less than unity, but at reasonable levels for public infrastructure, suggesting that project justification would likely have to depend on factors that are external to this study.

14. Financial Analysis - Road Tunnel Option

14.1 Selection of Key Target Parameters

The selection of the key target parameters is described in Section 12.1. These parameters are identical for the road and rail tunnel options.

14.2 Financial Analysis Results

For the financial analysis, the constant dollar estimates for project costs and revenues were translated into nominal values. In carrying out this conversion, the cost of financing during construction is determined by calculating interest on current year costs plus previous year payments plus interest accumulated in prior years. The project costs translated into current dollars, representing the total cost of the project, are shown in **Table 14-1** for both the longer and shorter construction periods.

Table 14-1: Project Costs in Current \$, IRRs and Suggested Grants Required

| | 1 TBM | 2 TBMs |
|---|--------|--------|
| Total Cost (\$2017) (millions) | 2,064 | 2,162 |
| Total Cost (\$Current) (millions) | 2,632 | 2,657 |
| Interest during Construction (millions) | 692 | 451 |
| Total to Finance (millions) | 3,324 | 3,107 |
| Internal Rate of Return (40%) | +0.44% | +0.19% |
| Internal Rate of Return (60%) | +1.04% | +1.28% |
| Grant required for 8.0% IRR (40%)(millions) | 2,041 | 2,104 |
| Grant required for 8.0% IRR (60%)(millions) | 1,820 | 1,880 |

The analysis results show that the total project costs (Total to Finance) are approximately 60% to 67% higher than the Total Costs (\$2017) because of inflation and interest expense – \$3.1 billion versus \$2.2 billion (for 2 TBMs) or \$3.3 billion versus \$2.1 billion (for 1 TBM). The advantage of a faster construction phase is apparent; a slight cost disadvantage in constant dollar terms becomes an advantage in real world conditions. The difference in the total costs to finance is worth \$217 million.

The total amount to finance is \$3,324 million (for 1 TBM), or \$3,107 million (for 2 TBMs). Incorporating the post operation cash flows, the Internal Rate of Return ranges from 0.44% to 1.28%. These values remain consistent with the conclusion of the economic analysis, that the project is not bankable on its own merits. Proceeding with the project will, hence, require access to free capital made available to suit purposes other than the gain of investment.

Various financing scenarios involving grants or contributions were considered to identify financial conditions under which the project could proceed. The main finding is that a total contribution in the order of \$2 billion, or more, would be required for the project to be self-sufficient. For example, to provide a return of 8%, grants ranging from \$1.820 billion to \$2.104 billion would be required. This is still below the private finance threshold considered currently applicable, except perhaps for long-term debt; however, it illustrates the level of public support that is required for advancing the project. It is of interest that the 2 TBMs option has a lower financed construction cost and a longer revenue generating period but still requires a

higher grant. This is the result of the proponent being relieved of much of the financing burden under a grant scenario since the construction cost is almost completely funded by the grant. Therefore, the construction financing advantage presented by the 2 TBMs option is negated when a grant is used to cover most of the construction costs as they occur. Hence, the 1 TBM option requires a smaller grant due to its lower overall costs in a mostly finance cost relieved environment.

The public sector involvement is considered the fundamental pre-condition for advancing the project beyond the present pre-feasibility stage. Once the level of support is decided, the models developed for this project can be employed to explore more financing variations and to examine potential revenue and cost relationships.

15. Financing Considerations using Public-Private-Partnership

The government of Newfoundland is operating deficits, and is facing rising costs associated with the maintenance of its existing infrastructure assets. Notwithstanding this, the province has a tremendous need for new infrastructure to enable economic growth.

Given this context, it is worth considering the potential to deliver the “Fixed Link” project via a Public-Private Partnership (P3).

P3s deliver several real, tangible benefits. The single most important way that they do this is by enabling the risks most associated with cost and schedule overruns (i.e. construction and maintenance) to be transferred from government to private sector partners who are better able to manage them. While government pays a cost premium to private financiers to achieve this risk transfer, the predictability of cost it creates is valuable.

P3 structures entail a non-traditional approach to government procurement. Traditional government procurements seek to encourage many competitive bids to deliver a 100% defined output. By contrast, the best P3 procurement processes are focused on outcomes/performance indicators that government hopes to realize from a given infrastructure project. A much smaller number of pre-qualified consortia (involving talented designers, builders, financiers, and maintainers) are then invited to propose how they would achieve these desired outcomes. When done right, government ensures that responses are sufficiently similar to allow for comparison, but still leaves lots of room for innovation.

P3 structures also have fiscal profiles that are beneficial to government. Rather than paying capital costs as they arise, these structures allow government to recognize costs over a longer period. In almost all cases, substantial completion payments (which represent a considerable portion of the total capital costs) are only paid once a government is satisfied that assets have been built to the pre-agreed standard. Similarly, ongoing “availability payments” are only made when the asset has achieved pre-agreed performance standards.

The following **Table 15-1** shows various types of P3 structures and the risk transfer which (ideally) is achieved through their usage.

It is worth noting that, for those models that do not involve private ownership and operation of infrastructure (i.e. DB, DBF, DBFM), there is a robust community of investors that has proven more than willing to participate in Canada. Since the inception of Infrastructure Ontario, close to \$3 billion worth of provincial infrastructure has been financed via a P3. Half of this money has come from investment funds, 31% from insurance companies, 12% from banks, and 7% from pension funds. The federal government, in the form of P3 Canada, has also been willing to invest in these types of projects.

Table 15-1: Risk Transfer to Private Sector by P3 Structure

| P3 Procurement Type | Design Risk | Construction Risk | Financial Risk | Maintenance Risk | Operating Risk | Traffic & Revenue Risk |
|---|-------------|-------------------|----------------|------------------|----------------|------------------------|
| Design-Build (DB) | ✓ | ✓ | | | | |
| Design-Build-Finance (DBF) | ✓ | ✓ | ✓ | | | |
| Design-Build-Finance-Maintain (DBFM) | ✓ | ✓ | ✓ | ✓ | | |
| Design-Build-Finance-Operate-Maintain (DBFOM) | ✓ | ✓ | ✓ | ✓ | ✓ | |
| Design-Build-Finance-Operate-Maintain (DBFOM) + Toll Concession | ✓ | ✓ | ✓ | ✓ | ✓ | ✓ |

Design-Build: The winning concession bids a fixed fee from which they must deliver the infrastructure asset based on a generally pre-determined schedule. The single concession is tasked with completing the design and construction for the project, and also takes on all associated risks with doing so. Sponsors take on the financing, operating and maintenance of the asset once completed. This type of development is more prevalent in pure private sector projects, but is increasingly gaining prominence in public sector procurements.

Design-Build-Finance: Under DBF contracts the winning concession takes on the additional responsibility of financing the project for the sponsor. Depending on the structure of the deal, the Sponsor can make periodic payments to the concession at specified completion points (milestone payments), payment once the project has been delivered (substantial completion payments), or some combination of both. Since the concession takes on additional financing responsibility, there is an impetus for them to deliver the project in a time conscious manner.

Design-Build-Finance-Maintain: The winning concession of this type of contract assumes maintenance responsibilities for the useful life of asset, or for some other unspecified period. Maintenance contracts are most effective for Government if they are well set up to manage the concession over the life of the maintenance contract. This includes being able to set delivery, service and quality thresholds.

Design-Build-Finance-Operate-Maintain: The winning concession takes on the additional responsibility to operate the asset. Generally this component is added when the concession is able to provide a breakthrough service through cost, scale and technological efficiencies. Both maintenance and operating components provide concessions incentive to deliver assets of the highest design and construction quality given the long-term cost associated with maintaining and operating.

The Strait of Belle Isle Fixed Link project has characteristics very similar to other large DBFM public-private partnerships (as we will demonstrate through case studies below).

A different set of considerations arises when government is looking for the private sector to operate and maintain infrastructure assets. Under DBFOM delivery models, projects are either partly or wholly financed by debt repaid with project revenues. If future revenues by themselves are insufficient to raise all capital needed, projects are often supplemented by public sector grants or in-kind (e.g., right-of-way) contributions. In most cases, private partners will be required to make equity investments. For this reason, DBFOM concessions are often attractive to public transportation agencies, as they can provide access to new sources of financing, transfer long-term life-cycle performance risk to the private sector, and deliver similar schedule and cost-efficiency benefits as DB and DBOM procurements.

Private investors are typically looking for three things when deciding where to invest their infrastructure allocations:

- A) Cash flow certainty. They want to know that the contract they are signing with government and the regulatory regime they will face will guarantee a reasonably predictable return on their investment.

- B) A positive track record of performance. The private sector prefers dealing in assets and services that have known risk profiles, that will allow them to better predict a likely return under various scenarios.
- C) A sizable equity investment. They want to be able to deploy large tranches of capital.

Based on their typical preferences, if we were to visually plot various types of investors against these three dimensions, the result might look like shown in **Figure 15-1**:

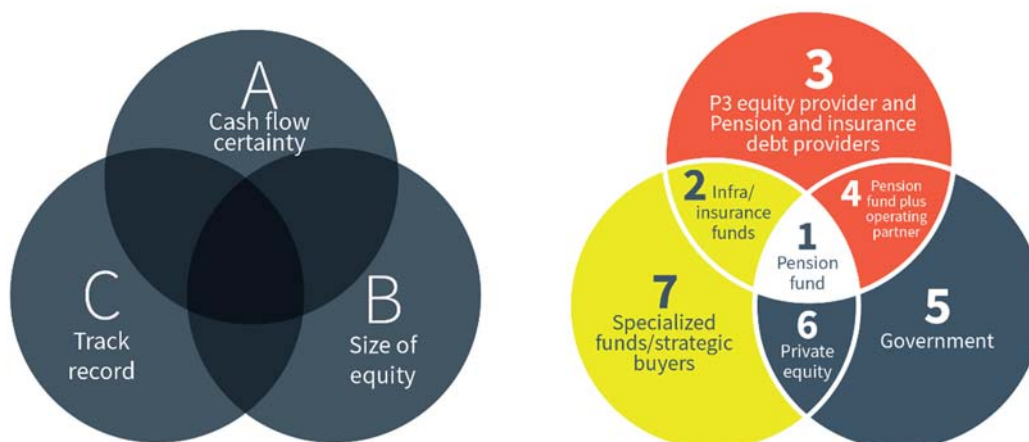


Figure 15-1: What the private sector looks for in projects to finance (left) and the kind of investment options that typically exist (right)

Many government projects fall squarely into the bottom right-hand space of this figure – Area 5 – Government. These projects require large sums of money, often have uncertain risks related to ridership volumes, revenues, operating costs, and regulatory regimes. Investors often participate in these projects on an equity basis when governments promise to retain responsibility for these various risks.

It is worth evaluating the potential to deliver the Strait of Belle Isle Fixed Link project via a DBFOM approach (with an associated tolling concession) provided:

- 1) The government is receptive to tolling both passenger and freight traffic;
- 2) The federal and provincial governments are willing to pay for the majority of the initial capital costs (N.B. most toll roads and tunnels make an operating profit, but this toll revenue rarely pays for the initial capital costs); and,
- 3) Projected tolling revenue is likely to enable the private consortium to achieve a sufficient return on their investment.

Should these conditions be met, numerous large institutional investors would be extremely interested in participating to the project. Pension plans, insurance companies, infrastructure funds, and sovereign wealth investors have billions to spend annually on these types of projects, and often struggle to spend their full allocation.

Given recent announcements, the federal government could also play a role in this approach. It could either ear-mark some monies from the planned \$187 billion in infrastructure grants for the project, or the project could be delivered via the Canadian Infrastructure Bank (CIB). The premise of the CIB is that there are countless infrastructure projects, each of which has the potential to enhance Canadian productivity while delivering returns to investors. But these projects, being risky (per the above) require some first loss capital to convince investors to proceed. To this end, Ottawa has set aside \$35 billion to invest in these projects. If project risks are realized – Ottawa is the first investor who suffers a loss. If the project yields revenue, the CIB achieves a return similar to other investors.

P3s: Impressive International Record

It has been over 20 years since countries such as Australia, United States and the United Kingdom began to use P3s as a regular method of procurement for public infrastructure projects. While the UK pioneered the P3 concept in the early 1990s, Canada is now widely acknowledged to be the key source of international best practice – the model and inspiration for emerging P3 programs around the globe. Especially since the onset of the financial crisis, the Canadian market has emerged as one of the world's most consistently productive – a market characterized by a strong pipeline, efficient procurement, vigorous competition in supply, and a supportive political environment.

In these countries, dozens of transportation projects are now in a mature phase of operation or under construction. As empirical experience has accumulated, there is clear evidence about specific advantages of the P3 model in terms of reducing the risks and enhancing the economic benefits associated with large-scale infrastructure and transportation investments. Experience shows that allocating risk in this way through the P3 model leads to two major benefits in project delivery: significant cost and time savings in construction; and, substantially greater focus on, and innovation to achieve, minimization of the costs infrastructure delivery across the life-cycle of the assets.

Below are some relevant examples where the P3 model has been used to successfully delivery large-scale transportation projects. It should be noted that these examples refer to P3 projects executed in densely populated areas as opposed to the rather remote area for the proposed Strait of Belle Isle Fixed Link.

Eglinton Crosstown LRT, Toronto, Ontario, Canada – DBFM (\$8.25 Billion)

Project Highlights:

The Eglinton Crosstown LRT is a light rail line under construction in Toronto, Ontario. Stage 1 consists of a 19-km corridor that will include a 10-km underground portion; 25 stops and stations, linking to bus routes, three subway stations and various GO Transit lines; and a new maintenance and storage facility. An extension (east or west) to the Crosstown LRT is anticipated, but not yet planned.



Key Takeaways:

- Procurement began as a standard TTC run procurement program, but was subsequently taken over by the regional rail authority (Metrolinx). Contract is scheduled as a DBFM with TTC remaining as operator for the line for 30-years after financial close.
- Two separate tunnel construction contracts, both let ahead of the DBFM, created significant complexity. There was an entire framework created for who is responsible for what, and the hand-off of the tunnels to the ultimate DBFM contractor.
- Metrolinx vowed to hand over tunnels that had not yet been built to a DBFM contractor. The process included a series of tests to determine if the DBB-constructed tunnels were fit for hand-over to the DBFM contractor.
- Vehicles and the two tunnel contracts were purchased outside the DBFM, and were effectively DBBs.
- TTC would likely operate the extension through an expansion of their current operating contract on the original Crosstown LRT.
- TTC is a logical choice in this scenario to operate the extension for the following reasons: strong labour union would object to another independent operator; longevity and deep understanding of the City of Toronto, and the hard infrastructure of the Crosstown LRT; and, internationally recognized for their safety record.

Ottawa LRT, Ottawa, Ontario, Canada – DBFM (\$2.1 Billion)

Project Highlights:

Ottawa's LRT system will be built in two stages. The first stage includes a new 12.5-km line along the existing Bus Route System (BRT) corridor and 13 stations, including three underground stations. The second stage (package one) includes adding 13-km of rail and 10 LRT stations. The future extension (Stage 2) is approved by council, and consists of a package of three extensions that will add over 39-km of rail and 23 new stations to the O-Train system by 2023.



Key Takeaways:

- In anticipation of building an extension to the Ottawa LRT, the City of Ottawa has sole sourced the operations contract for the extension (valued at \$600 M) to the incumbent RTG.
- In exchange for the sole sourced contract, RTG is precluded from bidding on the DBFM contract to build and maintain the extension component of the Ottawa LRT.
- Sole sourcing operations of the extension to RTG serves a dual purpose. It helps to maintain competitive tension for the build and maintain component of the extension while also ensuring that service levels across the line are maintained through use of one operator.
- To ensure consistent service across entire system, the following components were sole sourced to RTG as part of their operating component: system design, vehicles (38 additional light rail vehicles), supply of materials, and administration.

Port of Miami Tunnel, Miami, Florida, USA – DBFOM (\$1.4 Billion)

Project Highlights:

The Port of Miami Tunnel (POMT) project created a new, direct-access roadway connection from South Florida's Interstate highway network to the Port of Miami (POM). Project consisted of three integrated components: twin bored tunnels underneath the Main Shipping Channel between Dodge Island and Watson Island; widening of the existing MacArthur Causeway Bridge; and connections between the tunnel and the existing Port road network.



Key Takeaways:

- Extensive geotechnical sampling gave bidders sufficient confidence about the technical feasibility of the project which resulted in more competitive and cost-effective bids from private sector.
- Instead of receiving revenue from direct user tolls, the concessionaire will receive availability payments from FDOT throughout the duration of the contract to repay the up-front private sector financing of the design and construction of the tunnel plus the costs of operating and maintenance efforts once the facility is completed and placed into operation.
- Unique geotechnical risks associated with a tunnel of this size required risk-sharing between private developer and public sponsor with regards to potential changes in site conditions, such as unforeseen geotechnical conditions that increase project costs.
- Specific performance standards outlined by FDOT included measures of availability, service quality, and safety which helped create incentives for both timely completion of project construction and high operating and maintenance standards.
- FDOT has also included a High Traffic Payment as part of the availability payment, which compensated the concessionaire for higher maintenance costs if traffic levels greatly exceed the forecasts.
- Project highlights how availability payments can be used effectively in place of tolling when user fees are not an attractive option.

Elizabeth River Tunnels Project, Virginia, USA – DBFOM (\$2.1 Billion)

Project Highlights:

Project consists of a new two-lane tunnel adjacent to the existing Midtown Tunnel under the Elizabeth River, maintenance and safety improvements to the existing Midtown and Downtown tunnels, extending the MLK from London Boulevard to Interstate 264, and interchange modifications at Brambleton Avenue and Hampton Boulevard.



Key Takeaways:

- Fulling integrated DBFOM solution incorporated all the advantages of a DB solution and more (e.g. fixed price, date certain delivery of the asset; involvement of the O&M provider during the project development/design stage) which reduced risk of delay, leading to 6-month saving in delivering the new Midtown Tunnel.
- Project financing structure included: \$637.5 million Private Activity Bonds (PABs), \$441.4 million TIFIA loan, \$581 million State subsidy, \$251 million private equity, and \$150 million toll revenue during construction.
- Delivery method enabled seamless transfer of VDOT tunnel staff into ERC operating company, as well as rollout of all-electronic tolling (AET) solution during construction in order to reduce the toll rate once the project was finally completed.
- The agreement with Virginia DOT envisions that approximately 17.5% of the \$1.45-billion construction cost will be funded by tolling the existing free tunnels during the five-year construction period. The large savings on capitalized interest in the bond financing helped to reduce the starting toll from \$2.89 in an earlier plan.
- Joint workshops co-chaired by each party (developer, contractor, and VDOT) helped build a high degree of respect and trust before contract was mobilized.
- High political risk related to a change in state administration during project development state was mitigated by working with both gubernatorial administrations to adapt the contract to gain public approval, while meeting the state's goals and team's business targets.
- Full scale-marketing and media relations campaign was undertaken to inform citizens about tolling procedures and fares related to the implementation of AET for the first time in Hampton Roads, Virginia.

LBJ Express, Dallas County, USA – DBFOM (\$2.6 Billion)

Project Highlights:

The project involved the reconstruction of the main lanes and frontage roads along I-635, addition of six managed lanes (mostly subsurface) along I-635 from I-35E to US 75 and four managed lanes west and east of that stretch, and an addition of six elevated managed lanes along I-35E from Loop 12 to the I-35E/I-635 interchange.



Key Takeaways:

- The managed lanes are dynamically priced following a six-month introductory fixed-price schedule. HOV2+ users receive a 50 percent discount during peak operating periods.
- Funding sources include: Private Activity Bonds (PABs) - \$606 million; TIFIA loan - \$850 million; Equity contribution - \$682 million; Toll revenues during construction - \$17 million; Public funds - \$490 million.
- The TIFIA loan will be repaid with project revenues, which include all income, tolls, revenues, rates, fees, charges, rentals, or other receipts derived by or related to the operation of the project.
- Innovative financing package includes PABs and TIFIA credit assistance.
- Met the project goal to not increase the width or height of the existing roadway by constructing the managed lanes as an open trench, rather than placing them in tunnels, and cantilevering the existing general-purpose lanes above them. This approach also resulted in significant cost savings.

Capital Beltway, Virginia, USA – DBFOM (\$2.1 Billion)

Project Highlights:

The project included 14 miles of two new lanes in each direction; first time introduction of High Occupancy Vehicles (HOV) lanes to the Capital Beltway and reliable transit options to the Beltway and Tysons Corner, Virginia; congestion-free network for carpools, vanpools, transit and toll-paying motorists; and construction of carpool ramps connecting I-95 with the Capital Beltway to create a seamless HOV network.



Key Takeaways:

- Innovative design significantly reduced the displacements and impacts, as well as project costs which involved the replacement of more than \$260- million worth of aging infrastructure, including 50 bridges and overpasses outside of the HOT lanes.
- Dynamic tolling based on real-time traffic conditions.
- The total length of the concession is 85 years - five years of construction and 80 years of operation.
- First time a Private Activity Bond was used for HOT lanes in the U.S. and the first time combined with TIFIA financing
- The TIFIA loan holds a subordinate lien on a pledge of the project's toll revenues and interest income, after operations and maintenance expenses, certain capital expenditures, senior debt service reserve, and debt service payments to senior lenders.
- The TIFIA loan is structured with five years of capitalized interest during construction followed by five years of partially capitalized interest during ramp-up; then current interest only for 15 years followed by 15 years of interest plus principal.

16. Conclusions

16.1 Study Findings

Based on the review of assumptions and background information used in the Pre-Feasibility Study in 2004 and new information available, a review of fixed link projects that were carried out worldwide in recent years and the updated construction cost estimate, a rail tunnel excavated by TBM is still considered the most technically and economically attractive option for a fixed transportation link between Labrador and Newfoundland.

To assess the required capacity of the tunnel, it was assumed that all traffic from the current Strait of Belle Isle Ferry and 60% of the current Gulf Ferries traffic would be diverted to the new Fixed Link tunnel. Based on the traffic volume data provided to Hatch and considering a 2.5% traffic increase per year over the next 42 years (including 12 years for planning, design and construction), a design peak hour traffic volume of 197 vehicles per hour and direction was estimated for the Fixed Link. Considering this relatively low traffic volume, a single-track shuttle train with alternating operation in both directions would provide sufficient capacity to address the design traffic volume over the next 42 years. The shuttle cars would be custom made and could be designed to address the requirements regarding vehicle types using the Fixed Link.

The rail shuttle option would include a single bore tunnel with a 7.6 m inner diameter. The recommended ground cover above the tunnel is approximately 20 m, resulting in a tunnel depth of approximately 130 m below the water level in the Strait of Belle Isle. An optimization of the required tunnel depth below sea bottom will be possible based on further geotechnical investigation results at a later stage of the project.

The tunnel length was estimated to be approximately 30 km, assuming a straight tunnel alignment below the Strait of Belle Isle and on shore and considering exit ramps with 2% grades. A refinement of the tunnel length will be possible by aligning the exit ramps to the topography on both shores as part of a later project stage.

The tunnel excavation is recommended to be executed using a pressurized face TBM to address the challenges of tunnelling through the different geological formations that will be encountered at the project site comprising various rock types, varying from hard gneiss to weak shale. A pressurized face TBM would also be capable of excavating through fault zones with increased water inflows and addressing high hydrostatic pressures that have to be expected at a depth of 130 m below water level.

The construction costs for the single-track tunnel option, considering the use of one TBM, would be in the order of \$1,675 million (2017) with a construction period of approximately 10 years, including approximately 7 years of TBM tunnel drive; additional 5 years would have to be considered for planning and design of the tunnel.

The construction costs for the single-lane road tunnel option, considering the use of one TBM, would be in the order of \$2,064 million (2017) with a construction period of approximately 9 years, which include approximately 6.5 years of TBM tunnel drive. As for the

rail tunnel, additional 5 years would have to be considered for planning and design of the tunnel.

The costs for the planning and design stage would be in the order of \$20 to \$23 million (2017), which would include the environmental assessment and additional geotechnical investigations at the project site. A contingency of 40% on the tunnel (civil) costs has been included in the cost estimate.

The economic assessment is based on construction and operational costs. However, the level of detail required for the conceptual design in the current stage of the project does not include costs such as camp costs. Detailed cost estimates should be evaluated in the next project stage and included in the final assessment of the most suitable solution.

Delivering the Strait of Belle Isle project under a P3 - DBFOM arrangement appears to be an appropriate financing methodology; P3 financing has been successfully used in many large-scale tunnel projects similar to the Strait of Belle Isle Fixed Link.

An upgraded Strait of Belle Isle ferry option would be a lower cost alternative to a fixed link. However, transportation services during the winter months would still be restricted due to the environmental conditions in the Strait.

16.2 Future Work

Future activities required to advance the Strait of Belle Isle Fixed Link from pre-feasibility stage to a full feasibility study were discussed in the 2004 study. These activities include in particular further geotechnical/geophysical investigations along the proposed alignment, including borehole investigations within the Strait. These investigations would be costly and technically challenging due to the water depth and the harsh environment. However, they would be required to determine the ground conditions underneath the Strait and hence reduce the geotechnical risks during tunnel construction and define the most suitable tunnel alignment at depth which will have an immediate impact on the construction costs.

Considering that the planned Strait of Belle Isle Fixed Link would be serving an area with a low population density and a relatively low projected traffic volume, future work should also include an analysis of financing considerations for remote areas.

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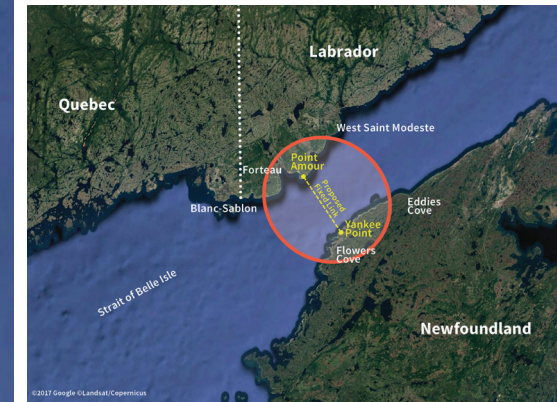
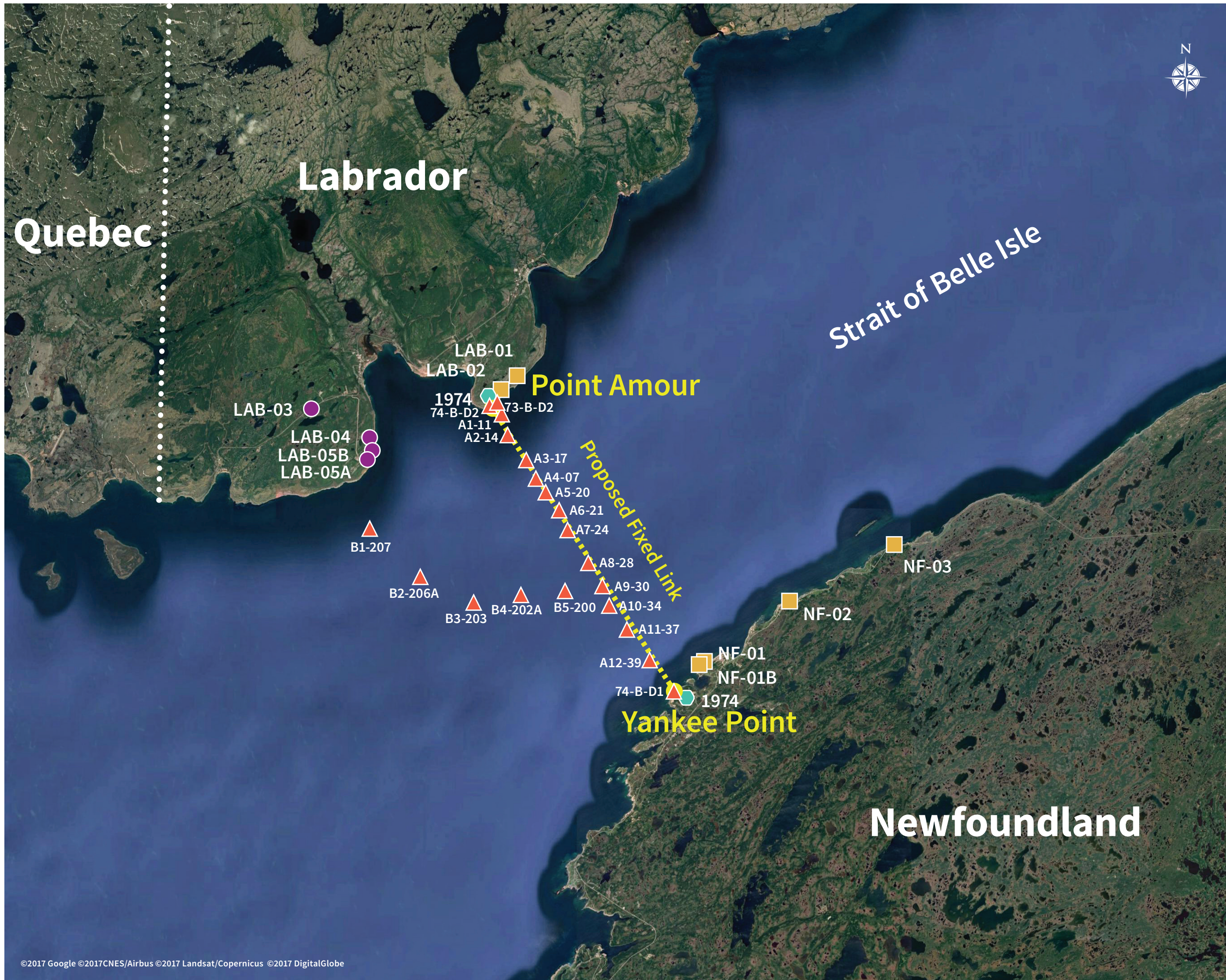
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APPENDIX A

Borehole Locations Strait of Belle Isle (Figure 1)



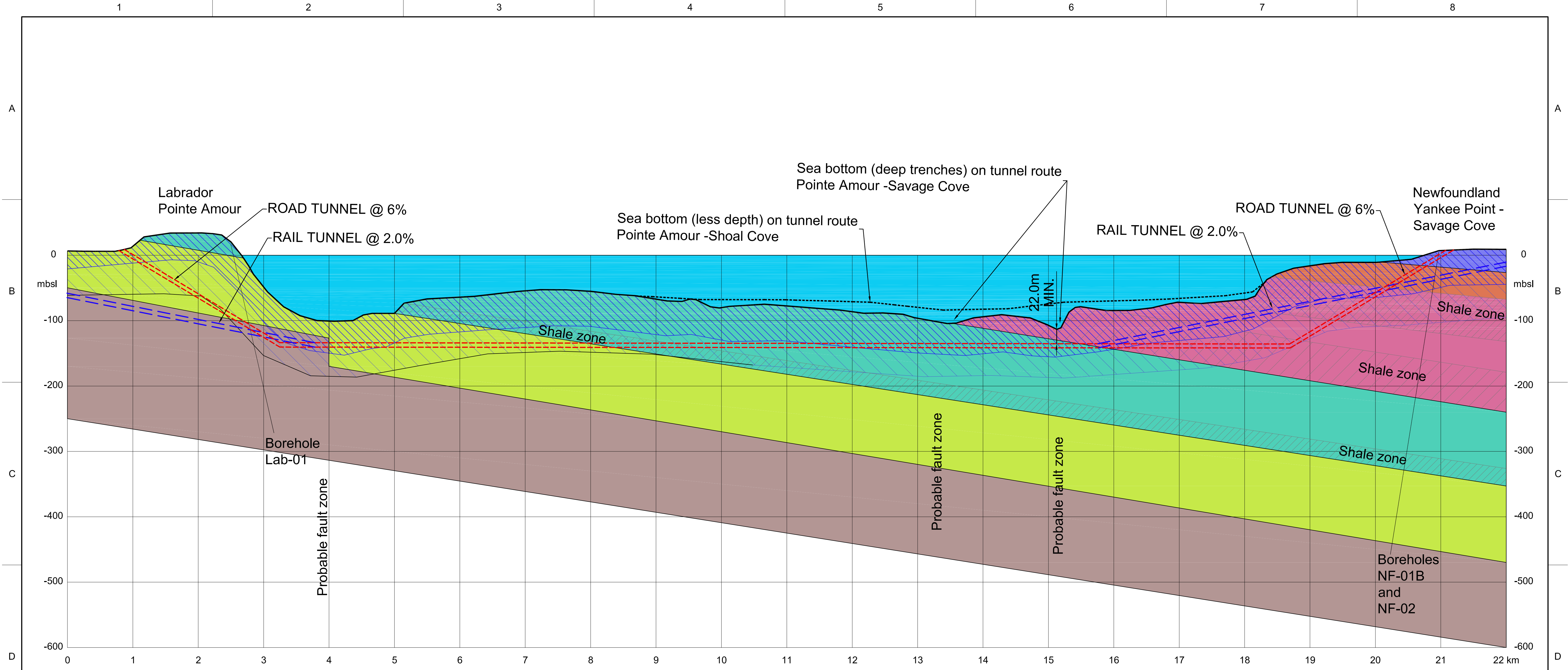
Legend

- 1974 Boreholes
 - ▲ 1980s Boreholes
 - 2009 Boreholes
 - 2011/2012 Boreholes
- Please note that the borehole locations are approximate.*


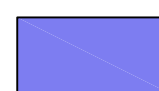
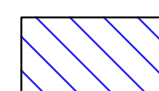




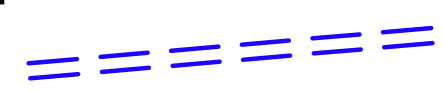


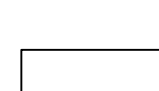
Figure A1: Borehole Locations
Strait of Belle Isle Fixed Link

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APPENDIX B
Schematic Geological Section Strait of Belle Isle
(Drawing 001)



DRAWING BASED ON DRAWING No. 5 PROVIDED IN: LANDSVIRKJUN POWER/MANNVIT ENGINEERING, 2010. LOWER CHURCHILL PROJECT, STRAIT OF BELLE ISLE, GEOTECHNICAL INVESTIGATIONS 2009, PRELIMINARY EVALUATION ON BOREHOLE DATA, REVISION 1, MARCH 2010, PREPARED FOR NALCOR ENERGY.

- | | | | | | | | |
|---|-------------------------------|---|---|---|---|---|---|
|  | Forteau limestone |  | Petite jardin -dolomite shale |  | High - medium permeability (Several Lugeon tests show >> 100 LU) |  Road | |
|  | Bradore sandstone |  | March point - dolomite shale |  | Medium - low permeability (LU values in the range of 10-30 LU) | |  Rail |
|  | Basement rock - granite gneis |  | Hawks bay - dolomite, limestone, sandstone, shale |  | Low - very low permeability (most values << 1 LU, occasional values 2-8 LU) | | |

| | | | | | | | | | | | | | | | | | | | |
|--|--|---------------|--|-------------------|--|---------------------|--|----|--|--------------|--|----------------|--|----------------|--|-----|--|--|--|
| <p>THIS DRAWING WAS PREPARED FOR THE EXCLUSIVE USE OF [NAME OF CLIENT] ("CLIENT") AND IS ISSUED PURSUANT TO [THE RELEVANT AGREEMENT BETWEEN CLIENT AND] HATCH LTD. ("HATCH"). UNLESS OTHERWISE AGREED IN WRITING WITH CLIENT OR SPECIFIED ON THIS DRAWING, (A) HATCH DOES NOT ACCEPT AND DISCLAIMS ANY AND ALL LIABILITY OR RESPONSIBILITY ARISING FROM ANY USE OF OR RELIANCE ON THIS DRAWING BY ANY THIRD PARTY OR ANY MODIFICATION OR MISUSE OF THIS DRAWING BY CLIENT, AND (B) THIS DRAWING IS CONFIDENTIAL AND ALL INTELLECTUAL PROPERTY RIGHTS ENJOYED OR REFERENCED IN THIS DRAWING REMAIN THE PROPERTY OF HATCH.</p> | | | | | | | | | | HATCH | | | DRAFTSPERSON RG OCT2017 DESIGNER CHECKER GM OCT2017 DESIGN COORD. RESP. ENG. LEAD DISC. ENG. ENG. MANAGER PROJ. MANAGER | | STRAIT OF BELLE ISLE FIXED LINK STRAIT OF BELLE ISLE SCHEMATIC GEOLOGICAL SECTION POINTE AMOUR - YANKEE POINT | | | | |
| DRAWING No. | | DRAWING TITLE | | REG. PROFESSIONAL | | A ISSUED FOR REPORT | | GM | | OCT2017 | | SCALE | | DWG. No. | | REV | | | |
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| REFERENCE DRAWINGS | | | | | | | | | | REVISIONS | | | | | | | | | |
| DRAWING APPROVAL STATUS: | | | | | | | | | | OR AS NOTED | | H354440-SK-001 | | SHEET SIZE: A1 | | | | | |

APPENDIX C

Construction Costs

APPENDIX C1

Road Tunnel

**Appendix C1-1
Road Tunnel - Single-Bore, 1 TBM
Construction Costs**

Newfoundland Fixed Link Pre-feasibility - Single Bored - 1TBM Highway Tunnel - Cost Summary

| BORED TUNNEL CONSTRUCTION COSTS | | |
|---|-------------|------------------------|
| ITEM | UNIT | MAIN TUNNEL |
| MOBILIZATION & DEMOBILIZATION | LS | 8,000,000 |
| TUNNELLING | LS | 600,306,358 |
| TUNNEL FINISHES | LS | 253,482,600 |
| NORTH APPROACH STRUCTURES | LS | 5,604,405 |
| SOUTH APPROACH STRUCTURES | LS | 6,979,072 |
| NORTH VENTILATION ADIT | LS | 11,002,540 |
| SOUTH VENTILATION ADIT | LS | 24,551,169 |
| ROAD FINISHES | LS | 4,304,750 |
| NORTH VEHICLE HOLDING AREA | LS | 6,960,000 |
| SOUTH VEHICLE HOLDING AREA | LS | 4,260,000 |
| TUNNEL DRAINAGE | LS | 9,554,554 |
| UTILITY DIVERSIONS | LS | 1,000,000 |
| MONITORING | LS | 1,000,000 |
| SUBTOTAL CIVIL | | \$937,005,448 |
| CIVIL CONTINGENCIES | | |
| CONTINGENCY | 40% | \$374,802,179 |
| TOTAL CIVIL | | \$1,311,807,628 |
| M&E AND FINISHING WORK | | |
| VENTILATION EQUIPMENT | LS | \$6,000,000 |
| VENTILATION BUILDINGS x 2 | LS | \$2,000,000 |
| FIRE SUPPRESSION SYSTEM | LS | \$4,000,000 |
| CONTROL CENTRE | LS | \$4,000,000 |
| SIGNALLING | LS | \$0 |
| LIGHTING | LS | \$2,959,546 |
| CCTV SYSTEM | LS | \$1,115,000 |
| GAS DETECTION | LS | \$945,000 |
| SUBSTATION, GENERATORS, UPS | LS | \$4,202,000 |
| SUBTOTAL M&E AND FINISHING | | \$25,221,546 |
| CONTINGENCIES | 20% | \$5,044,309 |
| TOTAL M&E AND FINISHING | | \$30,265,855 |
| TOTAL CIVIL, M&E AND FINISHING | | \$1,342,073,482 |
| ALLOWANCES | | |
| CONTRACTOR OH | 15% | \$201,311,022 |
| CONTRACTOR PROFIT | 15% | \$201,311,022 |
| CONSTRUCTION TOTAL | | \$1,744,695,527 |
| PRE-CONSTRUCTION AND SUPERVISION | | |
| FEASIBILITY STUDY | LS | \$17,000,000 |
| ENVIRONMENTAL ASSESSMENT | LS | \$6,000,000 |
| DESIGN | 5% | \$87,234,776 |
| CONSTRUCTION MANAGEMENT | 10% | \$174,469,553 |
| OWNERS COSTS | 2% | \$34,893,911 |
| PRE-CONSTRUCTION TOTAL | | \$319,598,240 |
| GRAND TOTAL | | \$2,064,293,767 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Set up | Parent Estimate ID: | 2631 |
| Tunnel Name: | Single Bore HWY T. | Project Phase: | Conceptual |
| Construction Activity: | Erect TBM Only | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | October 30, 2017 |
| Estimate Definition ID: | 7801 | Tunnel Characteristics ID: | 3393 |

Tunnel Characteristics

Finished Diameter: 11 m

Activity Details

Shift Arrangement 3 - 8 hour shifts x 7 days per week

Duration of Activity 4.5 Weeks

Total Number of Shifts 94.5

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 756.00 | 1.00 | 67,692 |
| Tunnel miner | 76.06 | \$/hr | 756.00 | 2.00 | 115,003 |
| Shaft bottom | 75.29 | \$/hr | 756.00 | 3.00 | 170,758 |
| Tunnel fitter | 69.92 | \$/hr | 756.00 | 1.00 | 52,860 |
| Tunnel electrician | 84.43 | \$/hr | 756.00 | 1.00 | 63,829 |
| Shaft top | 75.29 | \$/hr | 756.00 | 1.00 | 56,919 |
| Crane operator | 77.86 | \$/hr | 756.00 | 2.00 | 117,724 |
| Surface laborer | 75.22 | \$/hr | 756.00 | 1.00 | 56,866 |
| Equipment laborer | 72.88 | \$/hr | 756.00 | 1.00 | 55,097 |
| | | | | 13.00 | \$756,748 |
| Equipment | | | | | |
| Loco | 8,000.00 | \$/wk | 4.50 | 1.00 | 36,000 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 4.50 | 6.00 | 78,300 |
| Flat cars | 480.00 | \$/wk | 4.50 | 2.00 | 4,320 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 4.50 | 1.00 | 4,950 |
| Small tools | 2,600.00 | \$/wk | 4.50 | 1.00 | 11,700 |
| Shaft crane | 13,900.00 | \$/wk | 4.50 | 1.00 | 62,550 |
| Erection crane | 10,000.00 | \$/wk | 4.50 | 1.00 | 45,000 |
| Compressors | 1,500.00 | \$/wk | 4.50 | 1.00 | 6,750 |
| Generators | 3,100.00 | \$/wk | 4.50 | 1.00 | 13,950 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 4.50 | 1.00 | 36,000 |
| Loaders | 3,550.00 | \$/wk | 4.50 | 1.00 | 15,975 |
| | | | | | \$315,495 |
| Materials | | | | | |
| Temporary materials | 3,000.00 | \$/wk | 4.50 | 1.00 | 13,500 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|------------------|------------|----------------------|--------------------------|--------------|
| Thrust frame | 7,000.00 | \$/wk | 4.50 | 1.00 | 31,500 |
| | | | | | \$45,000 |
| Total Estimated Cost: | | | | | \$1,117,243 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Tunnel Drive | Parent Estimate ID: | 2632 |
| Tunnel Name: | Single Bore HWY T. | Project Phase: | Conceptual |
| Construction Activity: | TBM Tunneling | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | October 27, 2017 |
| Estimate Definition ID: | 7800 | Tunnel Characteristics ID: | 3393 |

Tunnel Characteristics

| | |
|-----------------------------------|----------------------------|
| Tunnel Length: | 21,000 m |
| Finished Diameter: | 11 m |
| Initial Support Type: | Pre-cast concrete segments |
| Initial Support Thickness: | 0 m |
| Final Lining Thickness: | 0.45 m |
| Grout Thickness: | 0.125 m |

Theoretical Excavation Volumes

| | |
|----------------------------------|------------------------|
| Total Neat Excavation: | 2,434,789 Cubic Metres |
| Initial Lining Volume: | 0 Cubic Metres |
| Final Lining Volume: | 339,928 Cubic Metres |
| Theoretical Grout Volume: | 99,166 Cubic Metres |

Normal Excavation/Support Cycle

| | |
|---------------------------------|------------|
| Excavation Cycle Length: | 1.5 Metres |
| Excavate: | 28 Minutes |
| Erect Support: | 36 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 64 Minutes |

Difficult Excavation/Support Cycle

| | |
|--|-------------|
| Length of Difficult Excavation: | 1400 Metres |
| Excavate: | 92 Minutes |
| Erect Support: | 74 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 166 Minutes |

Reduction Factors

| | |
|--------------------------------------|---------|
| Machine availability: | 80 % |
| Back up efficiency: | 55 % |
| Planned maintenance: | 5 % |
| Learning curve efficiency: | 40 % |
| Learning curve duration time: | 8 Weeks |

| | |
|----------------------------------|------------|
| Learning Curve Rate: | 5.6 m/day |
| Experienced Advance Rate: | 14.1 m/day |
| Difficult Advance Rate: | 5.4 m/day |

TBM Skidding Through Excavation

| | |
|------------------------------|----------|
| Duration of skidding: | 0 Weeks |
| Length of skidding: | 0 Metres |

Advance Rate and Shift Details

| | |
|---|-------------------------------------|
| Shift Arrangement: | 3 - 8 hour shifts x 7 days per week |
| Avg. Drive Advance per Shift: | 4.17 Metres |
| Avg. Drive Advance per Day: | 12 Metres |
| Avg. Drive Advance per Week: | 87 Metres |
| Duration of Tunneling (Incl. Skid): | 240.05 Weeks |
| Total number of shifts (Incl. Skid): | 5,041 |

| | Metres | Days |
|------------------------------|--------|-------|
| Learning Curve Drive: | 316 | 56 |
| Experienced Drive: | 19,284 | 1,367 |
| Difficult Drive: | 1,400 | 257 |
| Skidding Portion: | 0 | 0 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|--------------|-------|---------------|-------------------|---------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 40,332.00 | 1.00 | 3,611,327 |
| Tunnel miner | 76.06 | \$/hr | 40,332.00 | 3.00 | 9,202,956 |
| Tunnel laborer | 75.39 | \$/hr | 40,332.00 | 4.00 | 12,162,518 |
| Loco driver | 75.64 | \$/hr | 40,332.00 | 4.00 | 12,202,850 |
| Shaft bottom | 75.29 | \$/hr | 40,332.00 | 1.00 | 3,036,596 |
| TBM operator | 76.06 | \$/hr | 40,332.00 | 1.00 | 3,067,652 |
| Tunnel fitter | 69.92 | \$/hr | 40,332.00 | 1.00 | 2,820,013 |
| Tunnel electrician | 84.43 | \$/hr | 40,332.00 | 1.00 | 3,405,231 |
| Shaft top | 75.29 | \$/hr | 40,332.00 | 2.00 | 6,073,193 |
| Crane operator | 77.86 | \$/hr | 40,332.00 | 1.00 | 3,140,250 |
| Surface laborer | 75.22 | \$/hr | 40,332.00 | 4.00 | 12,135,092 |
| Equipment laborer | 72.88 | \$/hr | 40,332.00 | 4.00 | 11,757,585 |
| Shift Supervisor | 89.54 | \$/hr | 40,332.00 | 2.00 | 7,222,655 |
| | | | | 29.00 | \$89,837,917 |
| Equipment | | | | | |
| TBM | 340,000.00 | \$/m2 | 115.94 | 1.00 | 39,419,600 |
| TBM optional equipment. | 1,500,000.00 | \$/Nr | 1.00 | 1.00 | 1,500,000 |
| Loco | 8,000.00 | \$/wk | 240.05 | 4.00 | 7,681,600 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 240.05 | 56.00 | 38,984,120 |
| Flat cars | 480.00 | \$/wk | 240.05 | 8.00 | 921,792 |
| Person riders | 480.00 | \$/wk | 240.05 | 2.00 | 230,448 |
| Track | 200.00 | \$/m | 21,000.00 | 1.00 | 4,200,000 |
| Air pipe | 46.00 | \$/m | 21,000.00 | 1.00 | 966,000 |
| Water pipe | 40.00 | \$/m | 21,000.00 | 1.00 | 840,000 |
| Pump main | 78.00 | \$/m | 21,000.00 | 1.00 | 1,638,000 |
| Cabling | 125.00 | \$/m | 21,000.00 | 1.00 | 2,625,000 |
| Lighting | 46.00 | \$/m | 21,000.00 | 1.00 | 966,000 |
| Vent ducting | 46.00 | \$/m | 21,000.00 | 1.00 | 966,000 |
| Grout mixers | 10,960.00 | \$/wk | 240.05 | 1.00 | 2,630,948 |
| Grout pumps | 5,250.00 | \$/wk | 240.05 | 1.00 | 1,260,263 |
| Grout hoses & pipes | 300.00 | \$/wk | 240.05 | 2.00 | 144,030 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 240.05 | 2.00 | 528,110 |
| Small tools | 4,000.00 | \$/wk | 240.05 | 1.00 | 960,200 |
| Shaft crane | 13,900.00 | \$/wk | 240.05 | 1.00 | 3,336,695 |
| Compressors | 1,500.00 | \$/wk | 240.05 | 1.00 | 360,075 |
| Low pressure C/A system | 5,900.00 | \$/wk | 240.05 | 1.00 | 1,416,295 |
| Pipework and controls | 1,000.00 | \$/wk | 240.05 | 2.00 | 480,100 |
| Generators | 3,100.00 | \$/wk | 240.05 | 1.00 | 744,155 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 240.05 | 1.00 | 1,920,400 |
| Surface fans | 1,200.00 | \$/wk | 240.05 | 2.00 | 576,120 |
| Loaders | 3,550.00 | \$/wk | 240.05 | 2.00 | 1,704,355 |
| Other surface plant | 4,000.00 | \$/wk | 240.05 | 1.00 | 960,200 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--|-----------|--------|---------------|-------------------|---------------|
| Tunnel C/A system | 62,000.00 | \$/wk | 240.05 | 1.00 | 14,883,100 |
| | | | | | \$132,843,606 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 40,332.00 | 3,000.00 | 18,149,400 |
| Gas oil | 1.20 | \$/L | 48,000.00 | 1.00 | 57,600 |
| Lubrication materials | 140.00 | \$/wk | 240.05 | 1.00 | 33,607 |
| TBM spares, cutters | 390.00 | \$/m | 21,000.00 | 1.00 | 8,190,000 |
| Filters etc. | 465.00 | \$/wk | 240.05 | 1.00 | 111,623 |
| Hydraulic oil | 7.00 | \$/L | 32,000.00 | 1.00 | 224,000 |
| Other consumables | 200.00 | \$/wk | 240.05 | 1.00 | 48,010 |
| Tail seal grease | 200.00 | \$/m | 21,000.00 | 1.00 | 4,200,000 |
| | | | | | \$31,014,240 |
| Materials | | | | | |
| Concrete lining rings | 15,661.94 | \$/Nr | 14,000.00 | 1.00 | 219,267,194 |
| Gaskets | 170.00 | \$/m | 21,000.00 | 1.00 | 3,570,000 |
| Bolts | 20.00 | \$/Nr | 1,765.00 | 30.00 | 1,059,000 |
| Grout | 220.00 | \$/m3 | 99,166.00 | 1.00 | 21,816,520 |
| Grout plugs | 0.80 | \$/Nr | 1,765.00 | 7.00 | 9,884 |
| Packers | 15.00 | \$/Nr | 3,633.00 | 12.00 | 653,940 |
| Temporary materials | 3,000.00 | \$/wk | 240.05 | 1.00 | 720,150 |
| Other materials | 0.00 | \$/t | 0.00 | 1.00 | 0 |
| | | | | | \$247,096,688 |
| Subcontracts | | | | | |
| Soil disposal | 25.00 | \$/m3 | 2,434,789.00 | 1.50 | 91,304,588 |
| | | | | | \$91,304,588 |
| Total Estimated Cost: | | | | | \$592,097,038 |
| Total Estimated Cost per Metre: | | | | | \$28,195 |
| Total Estimated Cost per Week: | | | | | \$2,466,586 |
| Total Estimated Cost per Shift: | | | | | \$117,445 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | TBM Maintenance | Parent Estimate ID: | 2633 |
| Tunnel Name: | Single Bore HWY T. | Project Phase: | Conceptual |
| Construction Activity: | TBM Maintenance | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | October 30, 2017 |
| Estimate Definition ID: | 7802 | Tunnel Characteristics ID: | 3393 |

| <u>Tunnel Characteristics</u> | | <u>Activity Details</u> | |
|-------------------------------|------|-------------------------------|-------------------------------------|
| Finished Diameter: | 11 m | Shift Arrangement | 1 - 6 hour shifts x 1 days per week |
| | | Duration of Activity | 240 Weeks |
| | | Total Number of Shifts | 240 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|--------|---------------|-------------------|--------------------|
| Labor | | | | | |
| Working foreperson | 89.54 | \$/hr | 1,440.00 | 1.50 | 193,406 |
| Loco driver | 75.64 | \$/hr | 1,440.00 | 1.50 | 163,382 |
| Shaft bottom | 75.29 | \$/hr | 1,440.00 | 1.50 | 162,626 |
| TBM operator | 76.06 | \$/hr | 1,440.00 | 1.50 | 164,290 |
| Tunnel fitter | 69.92 | \$/hr | 1,440.00 | 1.50 | 151,027 |
| Tunnel electrician | 84.43 | \$/hr | 1,440.00 | 1.50 | 182,369 |
| Shaft top | 75.29 | \$/hr | 1,440.00 | 1.50 | 162,626 |
| Surface laborer | 75.22 | \$/hr | 1,440.00 | 1.50 | 162,475 |
| | | | | 12.00 | \$1,342,202 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 1,440.00 | 600.00 | 129,600 |
| Other consumables | 0.00 | \$/wk | 240.00 | 1.00 | 0 |
| | | | | | \$129,600 |
| Materials | | | | | |
| Temporary materials | 500.00 | \$/wk | 240.00 | 1.00 | 120,000 |
| | | | | | \$120,000 |
| Total Estimated Cost: | | | | | \$1,591,802 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Tunnel Clean up | Parent Estimate ID: | 2634 |
| Tunnel Name: | Single Bore HWY T. | Project Phase: | Conceptual |
| Construction Activity: | Tunnel Clean Up | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | October 30, 2017 |
| Estimate Definition ID: | 7804 | Tunnel Characteristics ID: | 3393 |

Tunnel Characteristics

| | |
|---------------------------------|---|
| Tunnel Length: | 21,000 m |
| Finished Diameter: | 11 m (Circular Tunnels) |
| Excavated Cross Section: | 0 m ² (Non-circular Tunnels) |
| Excavated Perimeter: | 0 m (Non-circular Tunnels) |

Productivity Cycle

| | |
|-----------------------------------|-------------|
| Section Length | 30 Metres |
| Vent Line Removal Time | 120 Minutes |
| Track Removal Time | 60 Minutes |
| Temp Lighting Removal Time | 60 Minutes |
| Clean Up Time | 120 Minutes |
| Total Cycle Time | 360 Minutes |

Reduction Factors

| | |
|-----------------------------------|---------|
| Learning Curve Efficiency: | 50 % |
| Back Up Efficiency: | 80 % |
| Learning Curve Duration: | 1 Weeks |

Shift Details

| | |
|--------------------------------|-------------------------------------|
| Shift Arrangement: | 3 - 8 hour shifts x 7 days per week |
| Avg. Advance per Shift: | 31.47 Metres |
| Avg. Advance per Week: | 661 Metres |
| Total number of hours: | 5,338 |

Clean Up Productivity Data

| | <u>Average Advance</u> | <u>Drive Length</u> | <u>Drive Duration</u> | | |
|-----------------------------------|------------------------|---------------------|-----------------------|----------|-------------|
| Learning Curve Portion: | 48.0 m/day | 336 Metres | 21 Shifts | 7 Days | 1.00 Weeks |
| Experienced Drive Portion: | 96.0 m/day | 20,664 Metres | 646 Shifts | 215 Days | 30.75 Weeks |
| Total: | 94.5 m/day | 21,000 Metres | 667 Shifts | 222 Days | 31.75 Weeks |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|--------------------|
| Labor | | | | | |
| Tunnel laborer | 75.39 | \$/hr | 5,338.00 | 6.00 | 2,414,591 |
| Shaft bottom | 75.29 | \$/hr | 5,338.00 | 1.00 | 401,898 |
| Shaft top | 75.29 | \$/hr | 5,338.00 | 1.00 | 401,898 |
| Crane operator | 77.86 | \$/hr | 5,338.00 | 1.00 | 415,617 |
| | | | | 9.00 | \$3,634,004 |
| Equipment | | | | | |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 31.75 | 1.00 | 34,925 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|---------------------|-----------|--------|---------------|-------------------|-----------|
| Hoists operator | 2,600.00 | \$/wk | 31.75 | 1.00 | 82,550 |
| Shaft crane | 13,900.00 | \$/wk | 31.75 | 1.00 | 441,325 |
| Compressors | 1,500.00 | \$/wk | 31.75 | 1.00 | 47,625 |
| Loaders | 3,550.00 | \$/wk | 31.75 | 1.00 | 112,713 |
| Other surface plant | 3,000.00 | \$/wk | 31.75 | 1.00 | 95,250 |
| Bobcat | 800.00 | \$/wk | 31.75 | 1.00 | 25,400 |
| | | | | | \$839,788 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 5,338.00 | 200.00 | 160,140 |
| | | | | | \$160,140 |

Total Estimated Cost: \$4,633,931
Total Estimated Cost per Metre: \$221
Total Estimated Cost per Week: \$145,951
Total Estimated Cost per Shift: \$6,945

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | TBM Removal | Parent Estimate ID: | 2635 |
| Tunnel Name: | Single Bore HWY T. | Project Phase: | Conceptual |
| Construction Activity: | TBM Removal | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | October 30, 2017 |
| Estimate Definition ID: | 7803 | Tunnel Characteristics ID: | 3393 |

Tunnel Characteristics

Finished Diameter: 11 m

Activity Details

Shift Arrangement 3 - 8 hour shifts x 7 days per week

Duration of Activity 3 Weeks

Total Number of Shifts 63

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 504.00 | 1.00 | 45,128 |
| Tunnel miner | 76.06 | \$/hr | 504.00 | 3.00 | 115,003 |
| Shaft bottom | 75.29 | \$/hr | 504.00 | 2.00 | 75,892 |
| Tunnel fitter | 69.92 | \$/hr | 504.00 | 1.00 | 35,240 |
| Tunnel electrician | 84.43 | \$/hr | 504.00 | 1.00 | 42,553 |
| Shaft top | 75.29 | \$/hr | 504.00 | 2.00 | 75,892 |
| Crane operator | 77.86 | \$/hr | 504.00 | 2.00 | 78,483 |
| Surface laborer | 75.22 | \$/hr | 504.00 | 2.00 | 75,822 |
| Equipment laborer | 72.88 | \$/hr | 504.00 | 1.00 | 36,732 |
| | | | | 15.00 | \$580,744 |
| Equipment | | | | | |
| Loco | 8,000.00 | \$/wk | 3.00 | 1.00 | 24,000 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 3.00 | 6.00 | 52,200 |
| Flat cars | 480.00 | \$/wk | 3.00 | 4.00 | 5,760 |
| Person riders | 480.00 | \$/wk | 3.00 | 1.00 | 1,440 |
| Booster fans | 1,500.00 | \$/wk | 3.00 | 1.00 | 4,500 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 3.00 | 1.00 | 3,300 |
| Other plant | 2,200.00 | \$/wk | 3.00 | 1.00 | 6,600 |
| Hoists operator | 3,000.00 | \$/wk | 3.00 | 1.00 | 9,000 |
| Shaft crane | 13,900.00 | \$/wk | 3.00 | 1.00 | 41,700 |
| 50T Crane | 7,000.00 | \$/wk | 3.00 | 1.00 | 21,000 |
| TBM Crane | 20,000.00 | \$/wk | 3.00 | 1.00 | 60,000 |
| Compressors | 1,500.00 | \$/wk | 3.00 | 1.00 | 4,500 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 3.00 | 1.00 | 24,000 |
| Surface fans | 1,200.00 | \$/wk | 3.00 | 1.00 | 3,600 |
| | | | | | \$261,600 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|-------|---------------|-------------------|------------------|
| Consumables | | | | | |
| Gas oil | 1.20 | \$/L | 15.00 | 1,000.00 | 18,000 |
| Other consumables | 500.00 | \$/wk | 3.00 | 1.00 | 1,500 |
| | | | | | <u>19,500</u> |
| Materials | | | | | |
| Temporary materials | 800.00 | \$/wk | 3.00 | 1.00 | 2,400 |
| | | | | | <u>2,400</u> |
| General Supplies | | | | | |
| Small tools | 700.00 | \$/wk | 3.00 | 1.00 | 2,100 |
| | | | | | <u>2,100</u> |
| Total Estimated Cost: | | | | | \$866,344 |

Initialisation

Project: Newfoundland Fixed Link Pre-feasibility Study

Section: North Approach

Option: Bored Highway Tunnel

Date: 31-Oct-17

Calculations by:

Surface gradient %

+ sloping same way as track/road
- sloping against track/road

Track/Road Gradient %

Ground elevation
at portal m

Bottom of slab
elevation at portal m

Total length= 328.571429 m

Total Cost=\$ 5.6 M

UNIT RATES (Sep. 2017)

Current construction index: 10823
Base index(April 2001): 6286

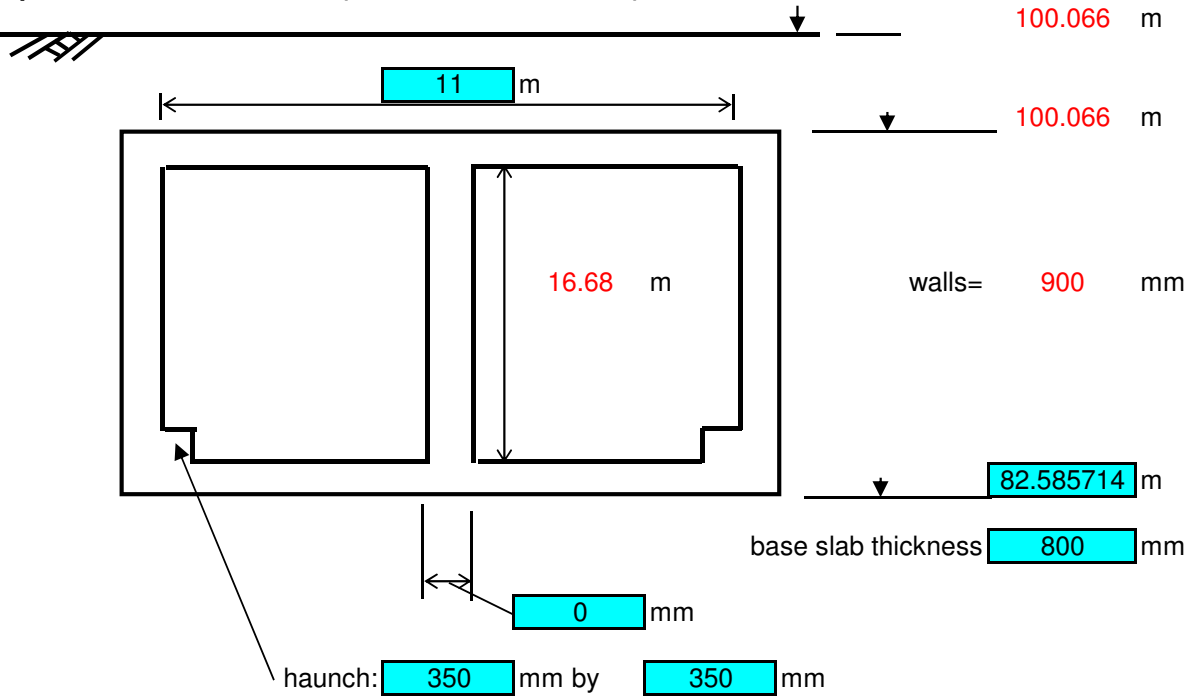
Materials

| Item | unit | Rate |
|-----------------------|--------|------|
| excavation | m3 | 60 |
| concrete | m3 | 220 |
| rebar | tonnes | 1600 |
| formwork/falsework | m2 | 241 |
| SP&L<=4.6m deep | m2 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0 |
| 20<SP&L<=25m deep | m2 | 0 |
| backfill + compact | m3 | 69 |
| surface reinstatement | m2 | 52 |

Note: Above unit rates include allowance for overheads and profit

Section Cut and Cover
 Length of section: 32.85714 m
 Section 1

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 7351.6 | m3 | |
| concrete= | 1331.01 | m3 | |
| rebar= | 159.7 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1096.114 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 1148.7 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 420.5714 | m2 | |

Calculated costs

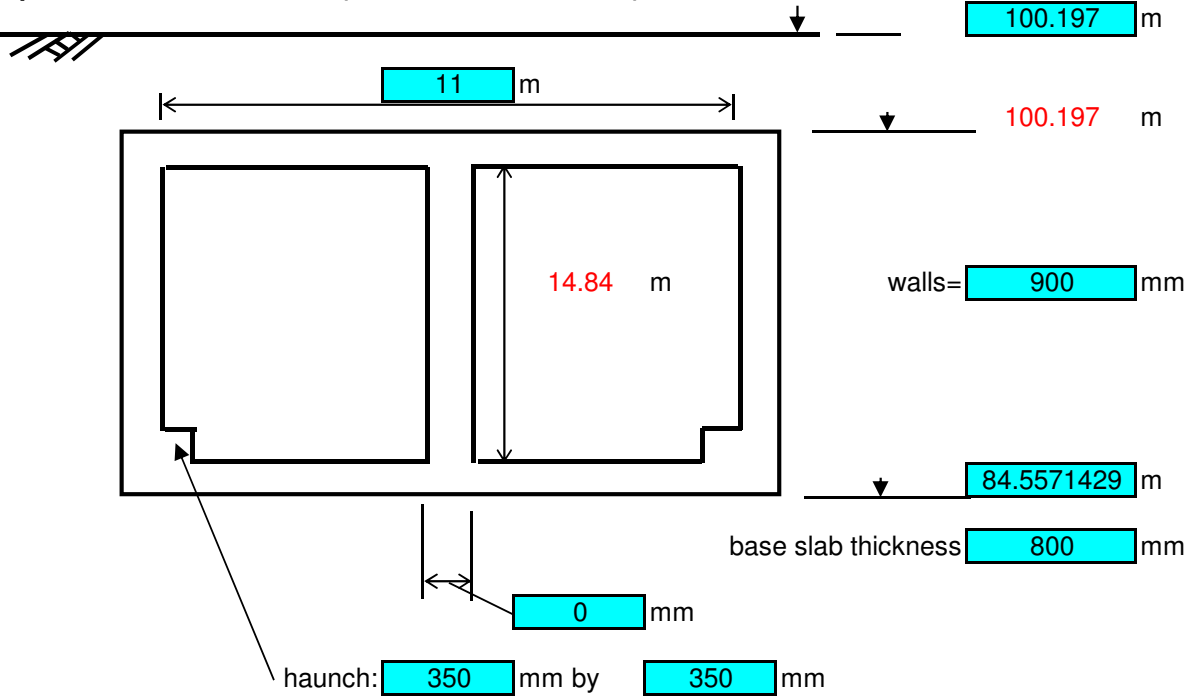
| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|-------------|
| excavation | m3 | 7351.6 | 60 | 441095.3143 |
| concrete | m3 | 1331.01 | 220.0 | 292822.2 |
| rebar | tonnes | 159.7 | 1600 | 255553.92 |
| formwork/falsework | m2 | 1096.114 | 241.0468 | 264214.8088 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 1148.7 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 420.5714 | 51.65288 | 21723.72529 |
| Total | | | | 1275409.968 |

Indirect costs, profit, and contingency

| | | |
|----|------------------------|------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | 1275410 |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | 1275410 |
| 25 | % contingency = \$ | 318852.4921 |
| | Total Cost = \$ | 1,594,262 |

Section Cut and Cover
 Length of section: 32.85714 m
 Section 2

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 6577.7 | m3 | |
| concrete= | 1222.187 | m3 | |
| rebar= | 146.7 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 975.2 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 1027.8 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 420.5714 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|-------------|
| excavation | m3 | 6577.7 | 60 | 394664.2286 |
| concrete | m3 | 1222.187 | 220.0 | 268881.1714 |
| rebar | tonnes | 146.7 | 1600 | 234659.9314 |
| formwork/falsework | m2 | 975.2 | 241.0468 | 235068.8107 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 1027.8 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 420.5714 | 51.65288 | 21723.72529 |

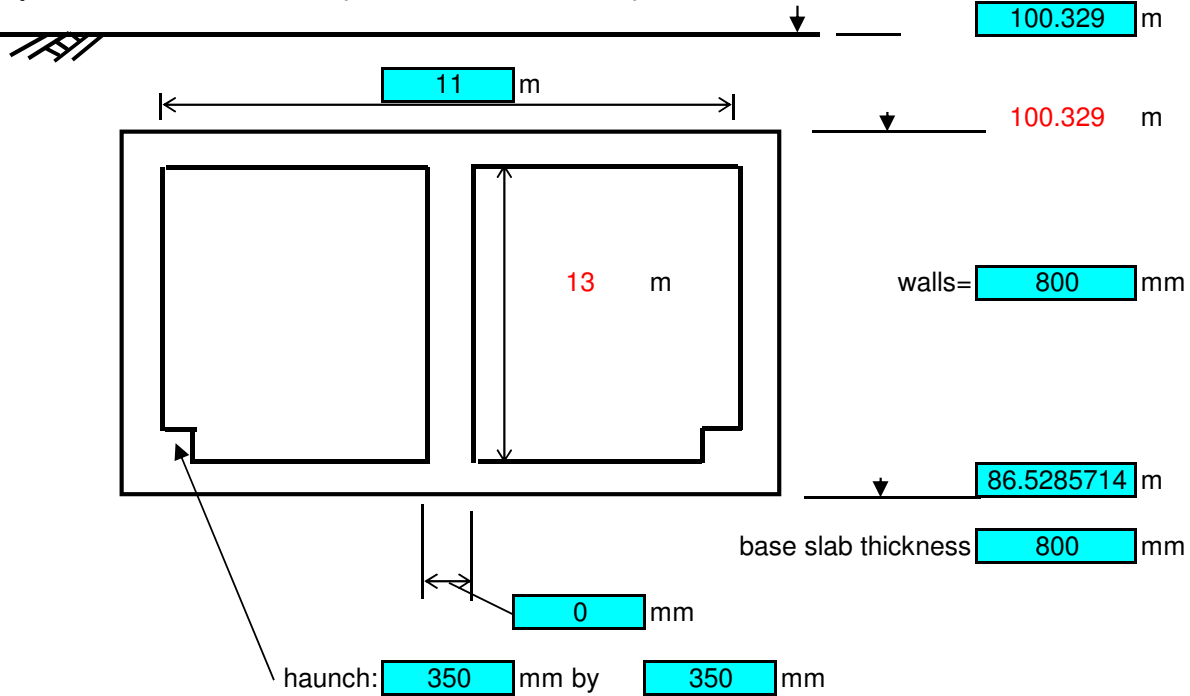
Total 1154997.867

Indirect costs, profit, and contingency

| | | |
|-----------|------------------------|-------------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | <u>1154998</u> |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | <u>1154998</u> |
| 25 | % contingency = \$ | 288749.4669 |
| | Total Cost = \$ | <u>1,443,747</u> |

Section Cut and Cover
 Length of section: 32.85714 m
 Section 3

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 5713.2 | m3 | |
| concrete= | 1022.679 | m3 | |
| rebar= | 122.7 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 854.2857 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 906.9 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 414 | m2 | |

Calculated costs

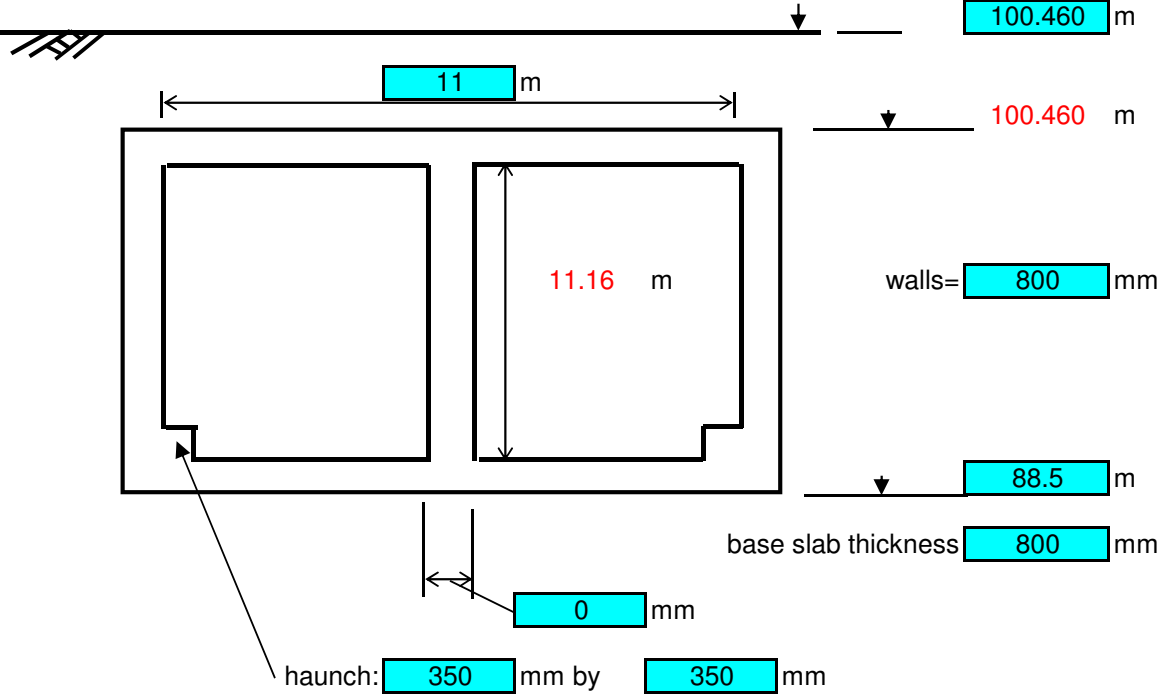
| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|--------------------|
| excavation | m3 | 5713.2 | 60 | 342792 |
| concrete | m3 | 1022.679 | 220.0 | 224989.2857 |
| rebar | tonnes | 122.7 | 1600 | 196354.2857 |
| formwork/falsework | m2 | 854.2857 | 241.0468 | 205922.8126 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 906.9 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 414 | 51.65288 | 21384.29208 |
| Total | | | | <u>991442.6761</u> |

Indirect costs, profit, and contingency

| | | |
|-----------|------------------------|-------------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | <u>991443</u> |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | <u>991443</u> |
| 25 | % contingency = \$ | 247860.669 |
| | Total Cost = \$ | <u>1,239,303</u> |

Section Cut and Cover
 Length of section: 32.85714 m
 Section 4

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|----------------|---|
| excavation= | 4951.4 | m ³ | |
| concrete= | 925.9471 | m ³ | |
| rebar= | 111.1 | tonnes | (assume 0.12t/m ³ of concrete) |
| formwork/falsework= | 733.3714 | m ² | |
| SP&L<=4.6m deep | 0.0 | m ² | |
| 4.6<SP&L<=6.7m deep | 0.0 | m ² | |
| 6.7<SP&L<=10.6m deep | 0.0 | m ² | |
| 10.6<SP&L<=13.7m deep | 785.9 | m ² | |
| 13.7<SP&L<=16.8m deep | 0.0 | m ² | |
| 16.8<SP&L<=20.0m deep | 0.0 | m ² | |
| 20<SP&L<=25m deep | 0.0 | m ² | |
| backfill= | 0 | m ³ | |
| surface reinstatement= | 414 | m ² | |

Calculated costs

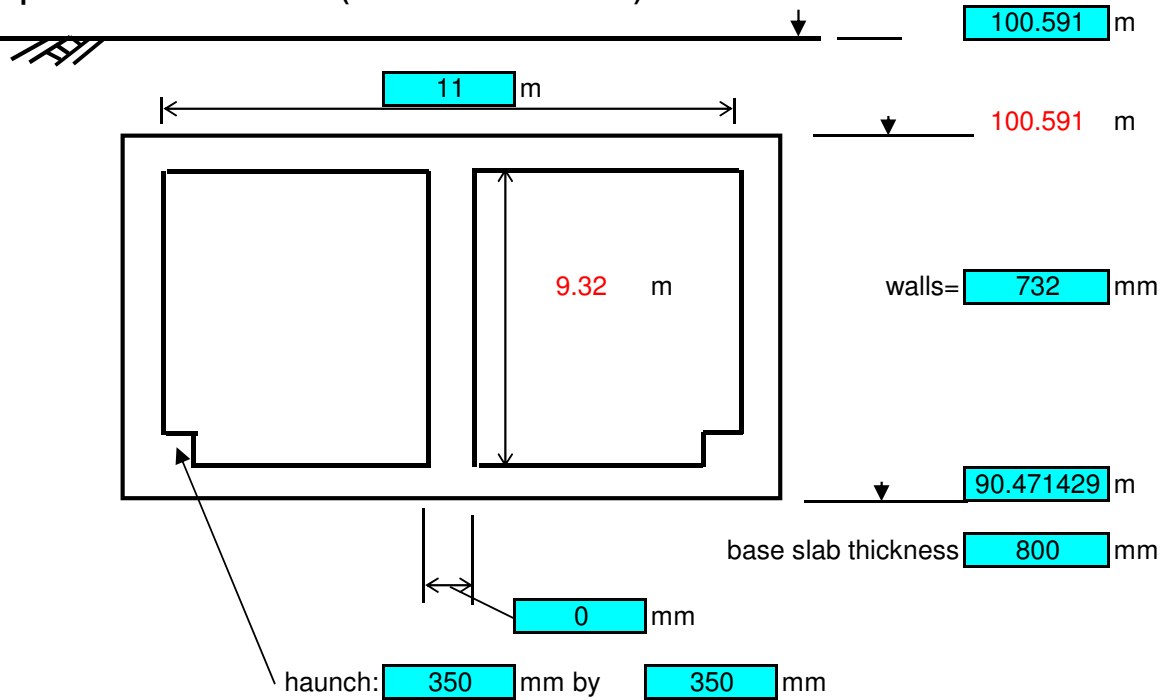
| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|--------------------|
| excavation | m3 | 4951.4 | 60 | 297086.4 |
| concrete | m3 | 925.9471 | 220.0 | 203708.3714 |
| rebar | tonnes | 111.1 | 1600 | 177781.8514 |
| formwork/falsework | m2 | 733.3714 | 241.0468 | 176776.8145 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 785.9 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 414 | 51.65288 | 21384.29208 |
| Total | | | | <u>876737.7294</u> |

Indirect costs, profit, and contingency

| | | |
|----|------------------------|-------------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | <u>876738</u> |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | <u>876738</u> |
| 25 | % contingency = \$ | 219184.4324 |
| | Total Cost = \$ | <u>1,095,922</u> |

Section Cut and Cover
 Length of section: 32.85714 m
 Section 5

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 4144.5 | m3 | |
| concrete= | 783.9938 | m3 | |
| rebar= | 94.1 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 612.4571 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 665.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 409.5314 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|-------------|
| excavation | m3 | 4144.5 | 60 | 248667.4834 |
| concrete | m3 | 783.9938 | 220.0 | 172478.6297 |
| rebar | tonnes | 94.1 | 1600 | 150526.8041 |
| formwork/falsework | m2 | 612.4571 | 241.0468 | 147630.8164 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 665.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 409.5314 | 51.65288 | 21153.4775 |

Total 740457.2112

Indirect costs, profit, and contingency

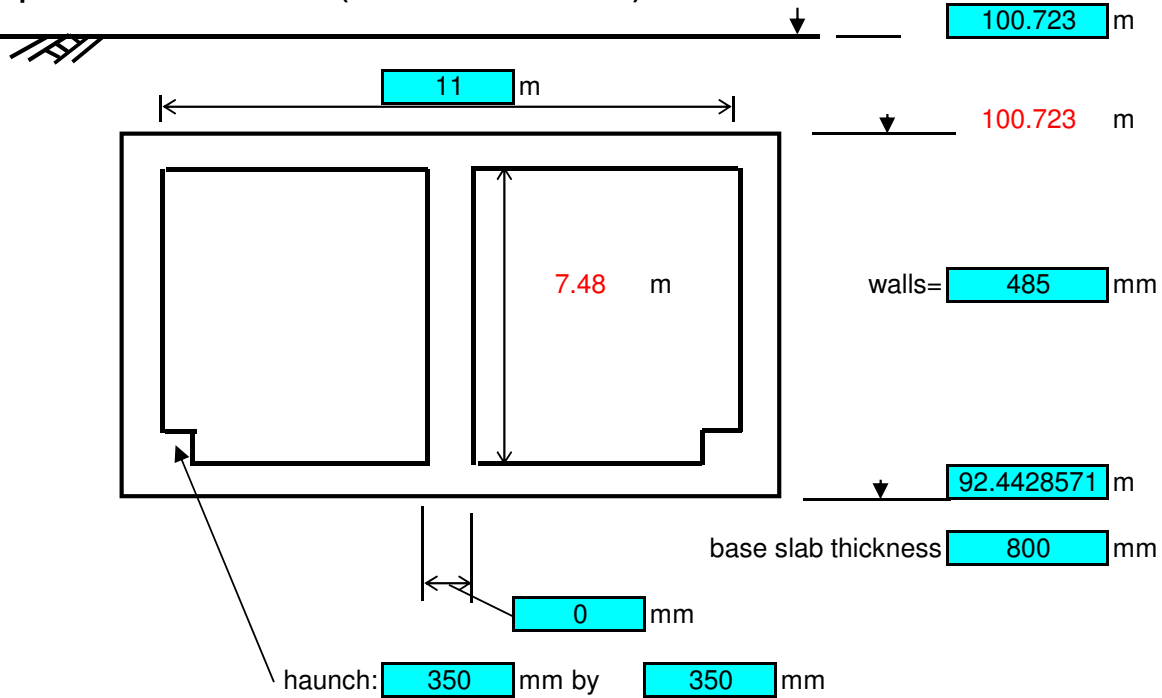
0 % indirect costs = \$ **0**
 Subtotal 2 = \$ **740457**

0 % profit = \$ **0**
 Subtotal 3 = \$ **740457**

25 % contingency = \$ **185114.3028**
Total Cost = \$ **925,572**

Section Cut and Cover
 Length of section: 32.85714 m Section 6

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 3256.5 | m3 | |
| concrete= | 561.0883 | m3 | |
| rebar= | 67.3 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 491.5429 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 544.1 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 393.3 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|-------------|
| excavation | m3 | 3256.5 | 60 | 195391.44 |
| concrete | m3 | 561.0883 | 220.0 | 123439.4229 |
| rebar | tonnes | 67.3 | 1600 | 107728.9509 |
| formwork/falsework | m2 | 491.5429 | 241.0468 | 118484.8183 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 544.1 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 393.3 | 51.65288 | 20315.07747 |

Total 565359.7095

Indirect costs, profit, and contingency

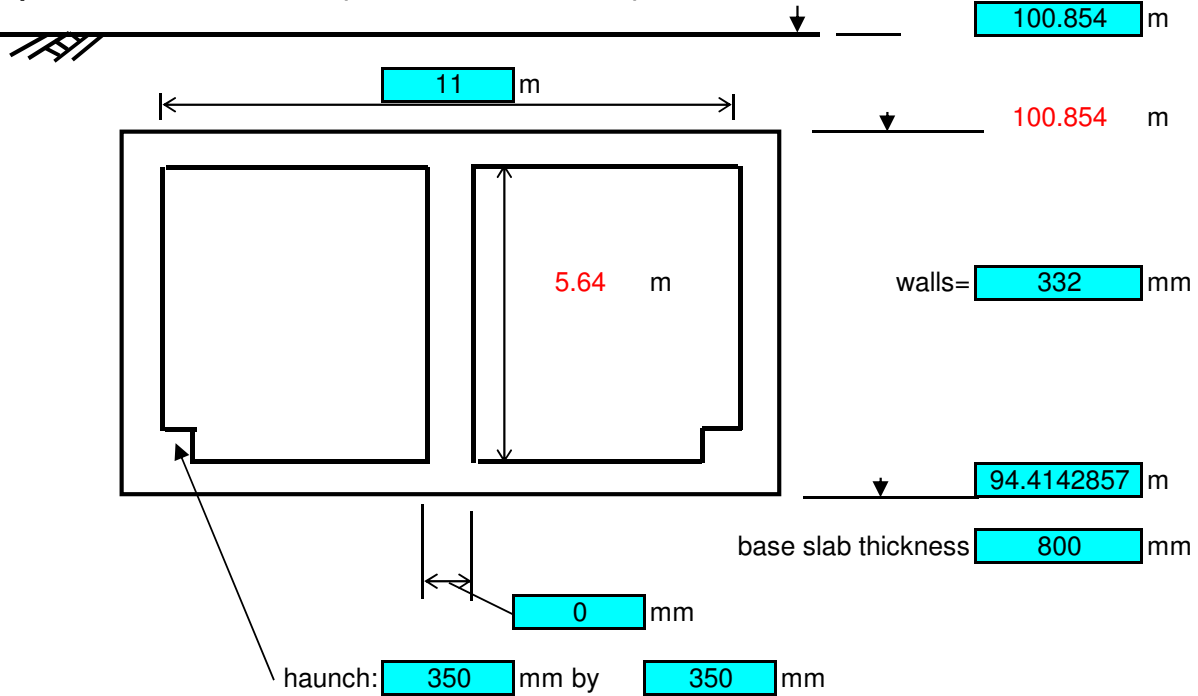
0 % indirect costs = \$ **0**
 Subtotal 2 = \$ **565360**

0 % profit = \$ **0**
 Subtotal 3 = \$ **565360**

25 % contingency = \$ **141339.9274**
Total Cost = \$ **706,700**

Section Cut and Cover
 Length of section: 32.85714 m
 Section 7

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 2468.1 | m3 | |
| concrete= | 437.6953 | m3 | |
| rebar= | 52.5 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 370.6286 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 423.2 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 383.2457 | m2 | |

Calculated costs

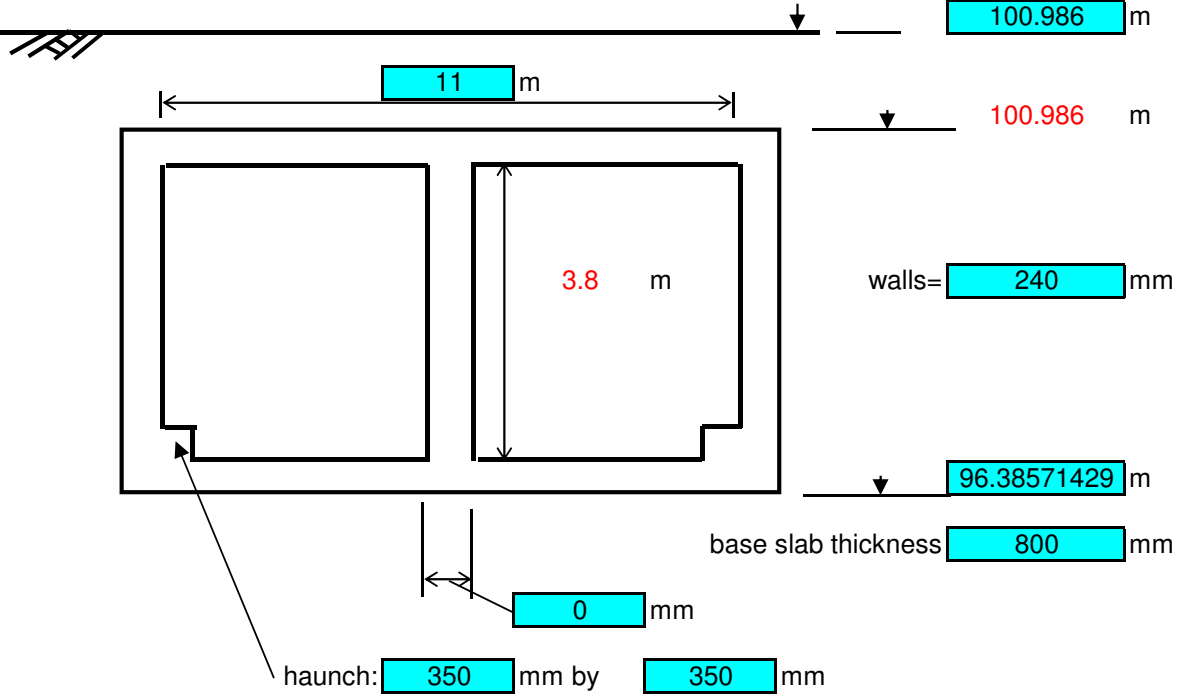
| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|--------------------|
| excavation | m3 | 2468.1 | 60 | 148086.144 |
| concrete | m3 | 437.6953 | 220.0 | 96292.95657 |
| rebar | tonnes | 52.5 | 1600 | 84037.48937 |
| formwork/falsework | m2 | 370.6286 | 241.0468 | 89338.82024 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 423.2 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 383.2457 | 51.65288 | 19795.74467 |
| Total | | | | <u>437551.1548</u> |

Indirect costs, profit, and contingency

| | | |
|-----------|------------------------|-----------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | <u>437551</u> |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | <u>437551</u> |
| 25 | % contingency = \$ | 109387.7887 |
| | Total Cost = \$ | <u>546,939</u> |

Section Cut and Cover
 Length of section: 32.85714 m
 Section 8

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 1735.1 | m3 | |
| concrete= | 369.7414 | m3 | |
| rebar= | 44.4 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 249.7143 | m2 | |
| SP&L<=4.6m deep | 302.3 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 377.2 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|-------------|
| excavation | m3 | 1735.1 | 60 | 104107.2 |
| concrete | m3 | 369.7414 | 220.0 | 81343.11429 |
| rebar | tonnes | 44.4 | 1600 | 70990.35429 |
| formwork/falsework | m2 | 249.7143 | 241.0468 | 60192.82214 |
| SP&L<=4.6m deep | m2 | 302.3 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 377.2 | 51.65288 | 19483.46612 |

Total 336116.9568

Indirect costs, profit, and contingency

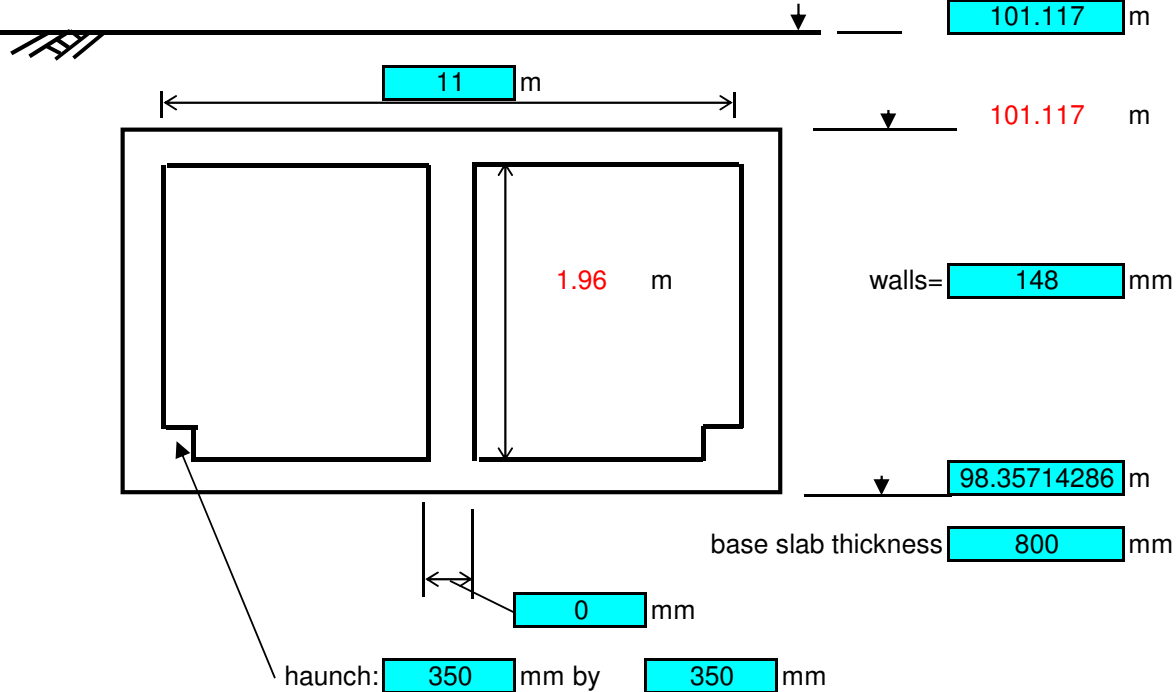
0 % indirect costs = \$ **0**
 Subtotal 2 = \$ **336117**

0 % profit = \$ **0**
 Subtotal 3 = \$ **336117**

25 % contingency = \$ **84029.23921**
Total Cost = \$ **420,146**

Section Cut and Cover
 Length of section: 32.85714 m Section 9

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 1024.4 | m3 | |
| concrete= | 324.0358 | m3 | |
| rebar= | 38.9 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 128.8 | m2 | |
| SP&L<=4.6m deep | 181.4 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 371.1543 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|-------------|
| excavation | m3 | 1024.4 | 60 | 61463.14971 |
| concrete | m3 | 324.0358 | 220.0 | 71287.88229 |
| rebar | tonnes | 38.9 | 1600 | 62214.87909 |
| formwork/falsework | m2 | 128.8 | 241.0468 | 31046.82405 |
| SP&L<=4.6m deep | m2 | 181.4 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 371.1543 | 51.65288 | 19171.18756 |

Total 245183.9227

Indirect costs, profit, and contingency

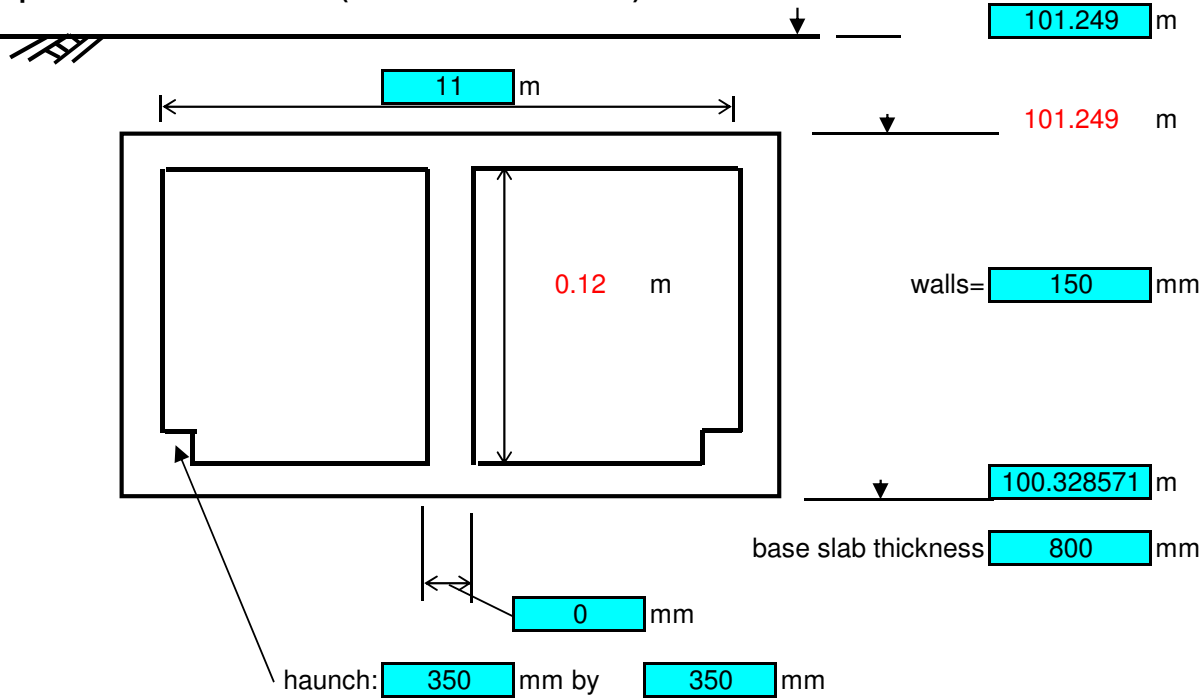
0 % indirect costs = \$ **0**
 Subtotal 2 = \$ **245184**

0 % profit = \$ **0**
 Subtotal 3 = \$ **245184**

25 % contingency = \$ **61295.98068**
Total Cost = \$ **306,480**

Section Cut and Cover
 Length of section: 32.85714 m
 Section 10

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 341.6 | m3 | |
| concrete= | 306.2614 | m3 | |
| rebar= | 36.8 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 7.885714 | m2 | |
| SP&L<=4.6m deep | 60.5 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 371.2857 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|----------|--------------------|
| excavation | m3 | 341.6 | 60 | 20494.97143 |
| concrete | m3 | 306.2614 | 220.0 | 67377.51429 |
| rebar | tonnes | 36.8 | 1600 | 58802.19429 |
| formwork/falsework | m2 | 7.885714 | 241.0468 | 1900.825962 |
| SP&L<=4.6m deep | m2 | 60.5 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 68.87051 | 0 |
| surface reinstatement | m2 | 371.2857 | 51.65288 | 19177.97623 |
| Total | | | | <u>167753.4822</u> |

Indirect costs, profit, and contingency

| | | |
|-----------|------------------------|-----------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | <u>167753</u> |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | <u>167753</u> |
| 25 | % contingency = \$ | 41938.37055 |
| | Total Cost = \$ | <u>209,692</u> |

Summary of Costs

Markup for adjacent 0 %

| Section | | Cost |
|------------------|-----------|------------------|
| 1 | \$ | 1,275,410.0 |
| 2 | \$ | 1,154,997.9 |
| 3 | \$ | 991,442.7 |
| 4 | \$ | 876,737.7 |
| 5 | \$ | 740,457.2 |
| 6 | \$ | 565,359.7 |
| 7 | \$ | 437,551.2 |
| 8 | \$ | 336,117.0 |
| 9 | \$ | 245,183.9 |
| 10 | \$ | 167,753.5 |
| Sub-total | \$ | 5,604,405 |

Initialisation

Project: Newfoundland Fixed Link Pre-feasibility Study

Section: South Approach

Option: Bored Highway Tunnel

Date: 1-Nov-17

Calculations by:

Surface gradient %

+ sloping same way as track/road
- sloping against track/road

Track/Road Gradient %

Ground elevation
at portal m

Bottom of slab
elevation at portal m

Total length= m

Total Cost=\$ M

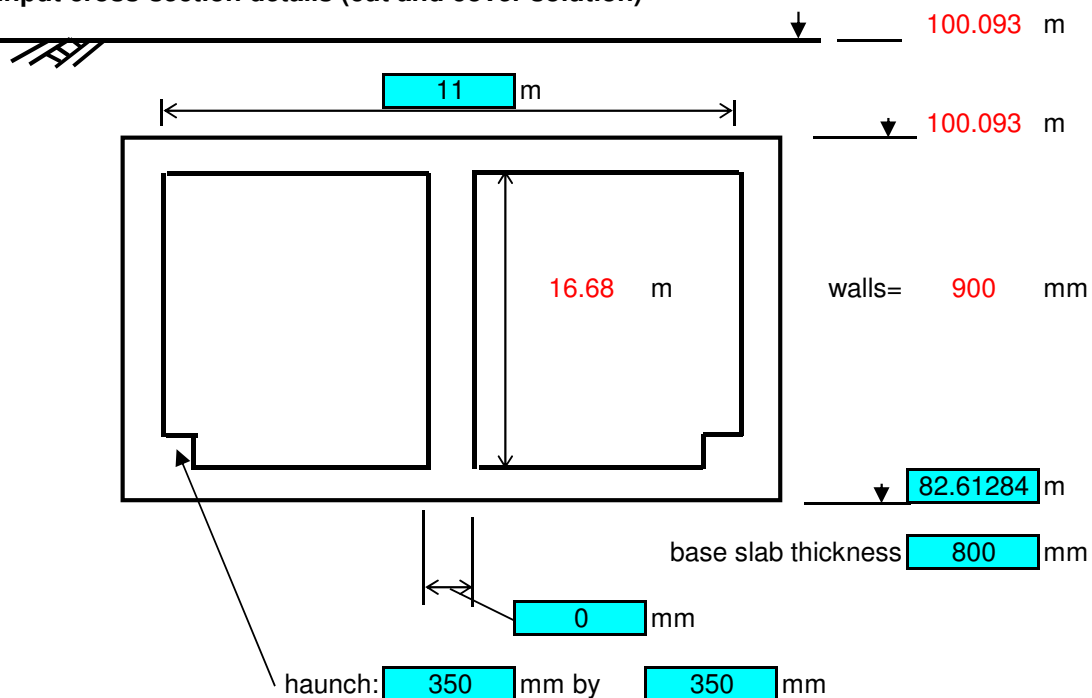
UNIT RATES

Materials

| Item | unit | Rate |
|-----------------------|-------------|-------------|
| excavation | m3 | 60 |
| concrete | m3 | 220 |
| rebar | tonnes | 1600 |
| formwork/falsework | m2 | 241 |
| SP&L<=4.6m deep | m2 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0 |
| 20<SP&L<=25m deep | m2 | 0 |
| backfill + compact | m3 | 69 |
| surface reinstatement | m2 | 52 |

Section Cut and Cover
 Length of section: 33.8 m Section 1

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 7553.9 | m3 | |
| concrete= | 1367.643 | m3 | |
| rebar= | 164.1 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1126.283 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 1180.3 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 432.1468 | m2 | |

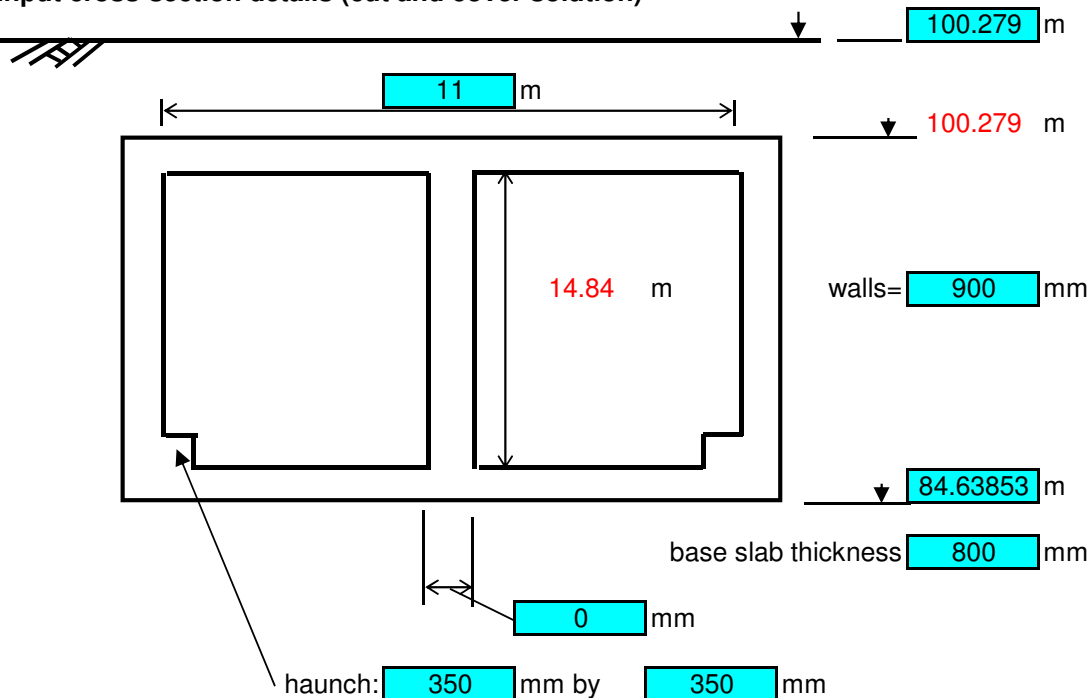
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 7553.9 | 60 | 453235.6 |
| concrete | m3 | 1367.643 | 220.0 | 300881.5 |
| rebar | tonnes | 164.1 | 1600 | 262587.5 |
| formwork/falsework | m2 | 1126.283 | 241 | 271434.1 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 1180.3 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 432.1468 | 52 | 22471.63 |

Total 1310610

Section Cut and Cover
 Length of section: 34 m Section 2

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 6758.8 | m3 | |
| concrete= | 1255.825 | m3 | |
| rebar= | 150.7 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1002.04 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 1056.1 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 432.1468 | m2 | |

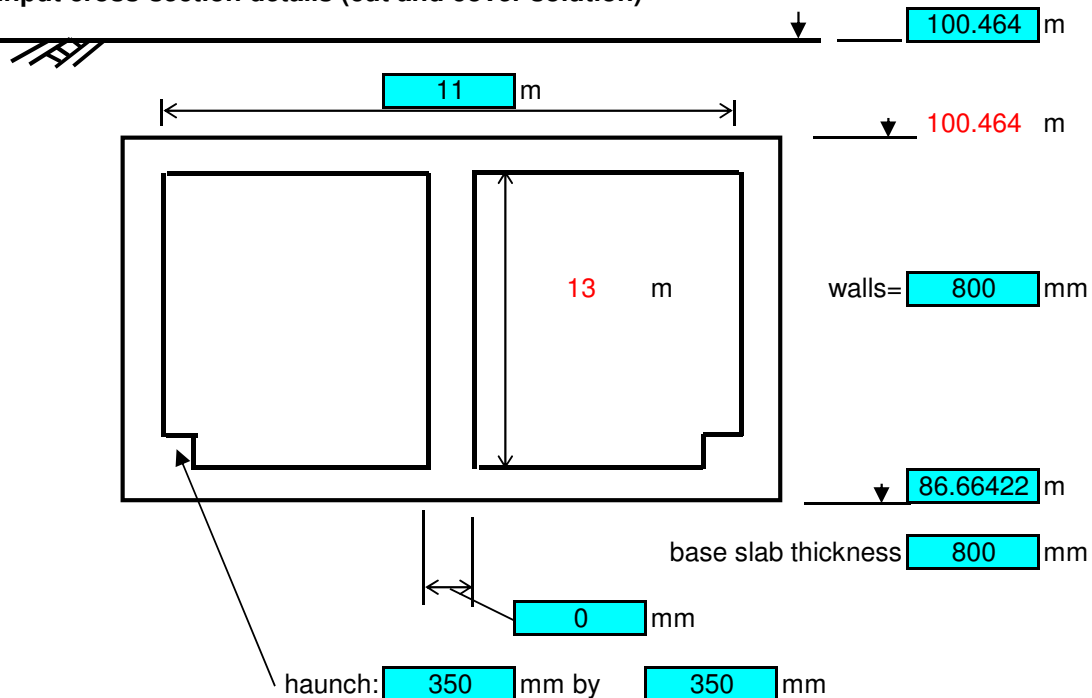
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 6758.8 | 60 | 405526.5 |
| concrete | m3 | 1255.825 | 220.0 | 276281.6 |
| rebar | tonnes | 150.7 | 1600 | 241118.5 |
| formwork/falsework | m2 | 1002.04 | 241 | 241491.7 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 1056.1 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 432.1468 | 52 | 22471.63 |

Total 1186890

Section Cut and Cover
 Length of section: 34 m Section 3

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 5870.4 | m3 | |
| concrete= | 1050.826 | m3 | |
| rebar= | 126.1 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 877.7982 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 931.8 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 425.3945 | m2 | |

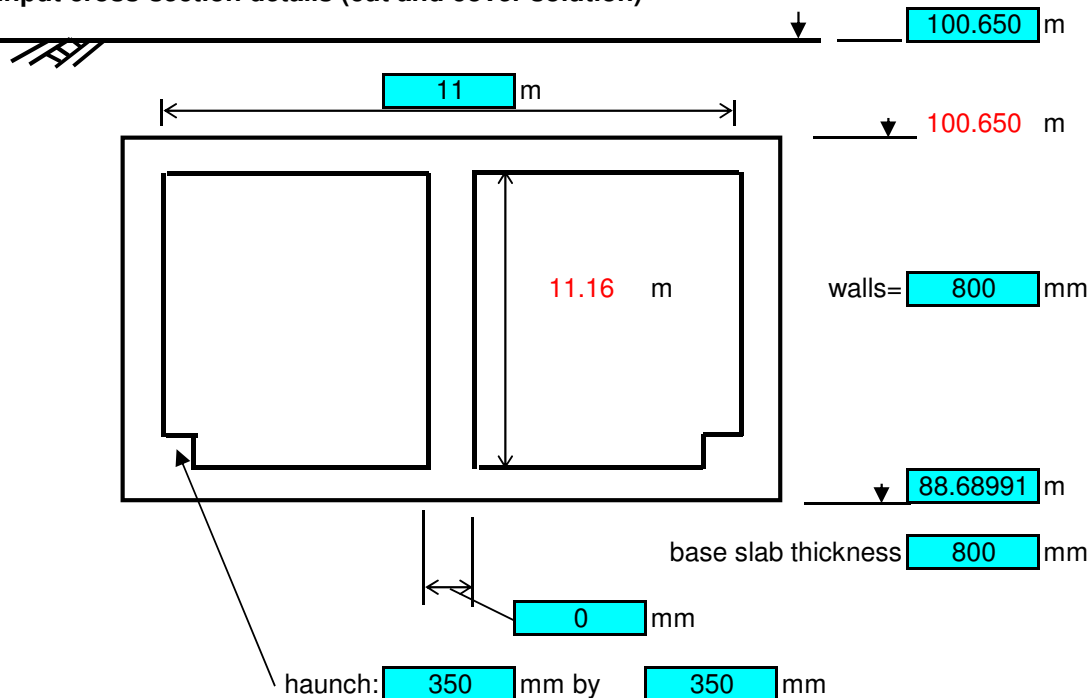
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 5870.4 | 60 | 352226.6 |
| concrete | m3 | 1050.826 | 220.0 | 231181.7 |
| rebar | tonnes | 126.1 | 1600 | 201758.5 |
| formwork/falsework | m2 | 877.7982 | 241 | 211549.4 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 931.8 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 425.3945 | 52 | 22120.51 |

Total 1018837

Section Cut and Cover
 Length of section: 33.76147 m Section 4

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 5087.7 | m3 | |
| concrete= | 951.4319 | m3 | |
| rebar= | 114.2 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 753.556 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 807.6 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 425.3945 | m2 | |

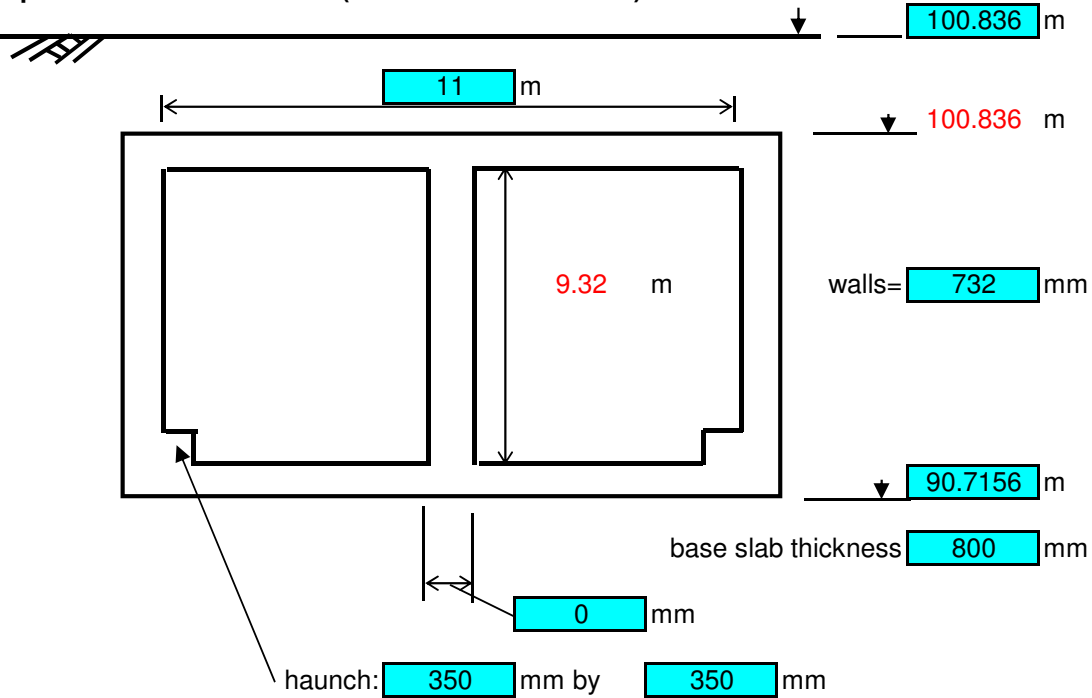
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 5087.7 | 60 | 305263.1 |
| concrete | m3 | 951.4319 | 220.0 | 209315 |
| rebar | tonnes | 114.2 | 1600 | 182674.9 |
| formwork/falsework | m2 | 753.556 | 241 | 181607 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 807.6 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 425.3945 | 52 | 22120.51 |

Total 900980.5

Section Cut and Cover
 Length of section: 33.76147 m Section 5

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 4258.5 | m3 | |
| concrete= | 805.5716 | m3 | |
| rebar= | 96.7 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 629.3138 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 683.3 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 420.8029 | m2 | |

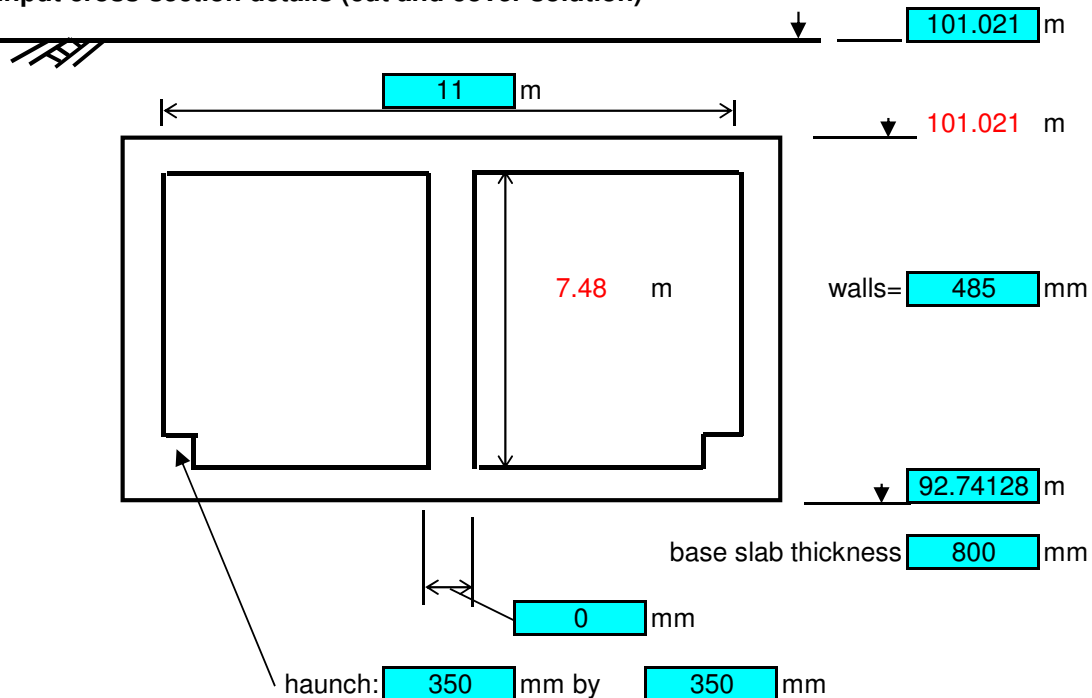
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 4258.5 | 60 | 255511.5 |
| concrete | m3 | 805.5716 | 220.0 | 177225.7 |
| rebar | tonnes | 96.7 | 1600 | 154669.7 |
| formwork/falsework | m2 | 629.3138 | 241 | 151664.6 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 683.3 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 420.8029 | 52 | 21881.75 |

Total 760953.4

Section Cut and Cover
 Length of section: 33.76147 m Section 6

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 3346.2 | m3 | |
| concrete= | 576.5311 | m3 | |
| rebar= | 69.2 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 505.0716 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 559.1 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 404.1248 | m2 | |

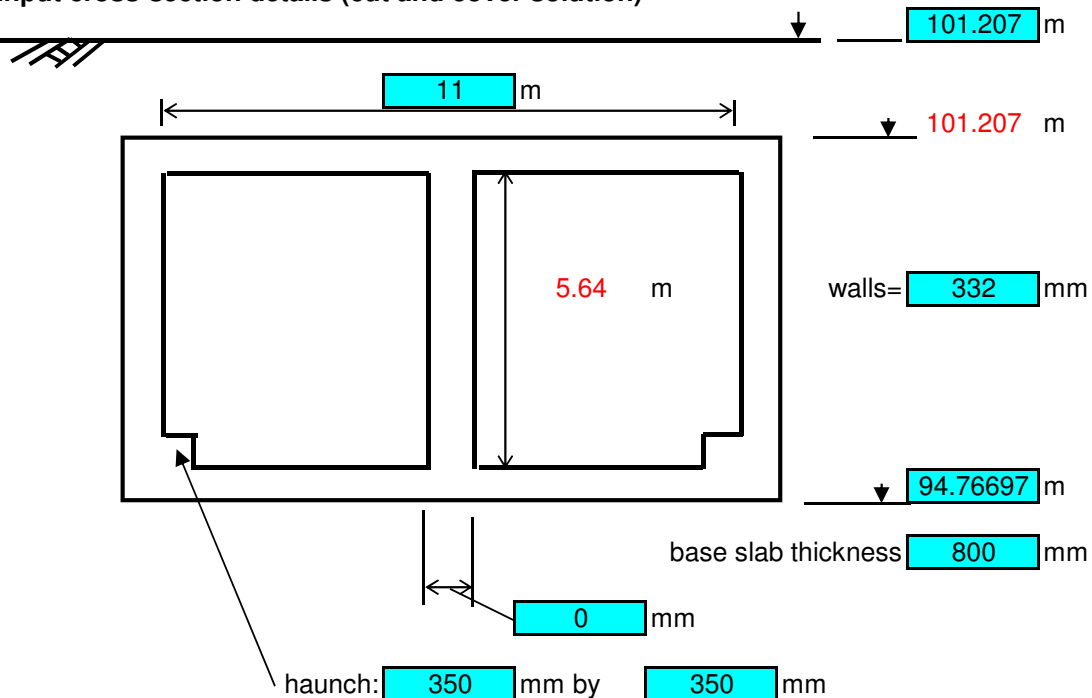
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 3346.2 | 60 | 200769.2 |
| concrete | m3 | 576.5311 | 220.0 | 126836.8 |
| rebar | tonnes | 69.2 | 1600 | 110694 |
| formwork/falsework | m2 | 505.0716 | 241 | 121722.2 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 559.1 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 404.1248 | 52 | 21014.49 |

Total 581036.7

Section Cut and Cover
 Length of section: 33.76147 m Section 7

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 2536.0 | m3 | |
| concrete= | 449.7419 | m3 | |
| rebar= | 54.0 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 380.8294 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 434.8 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 393.7938 | m2 | |

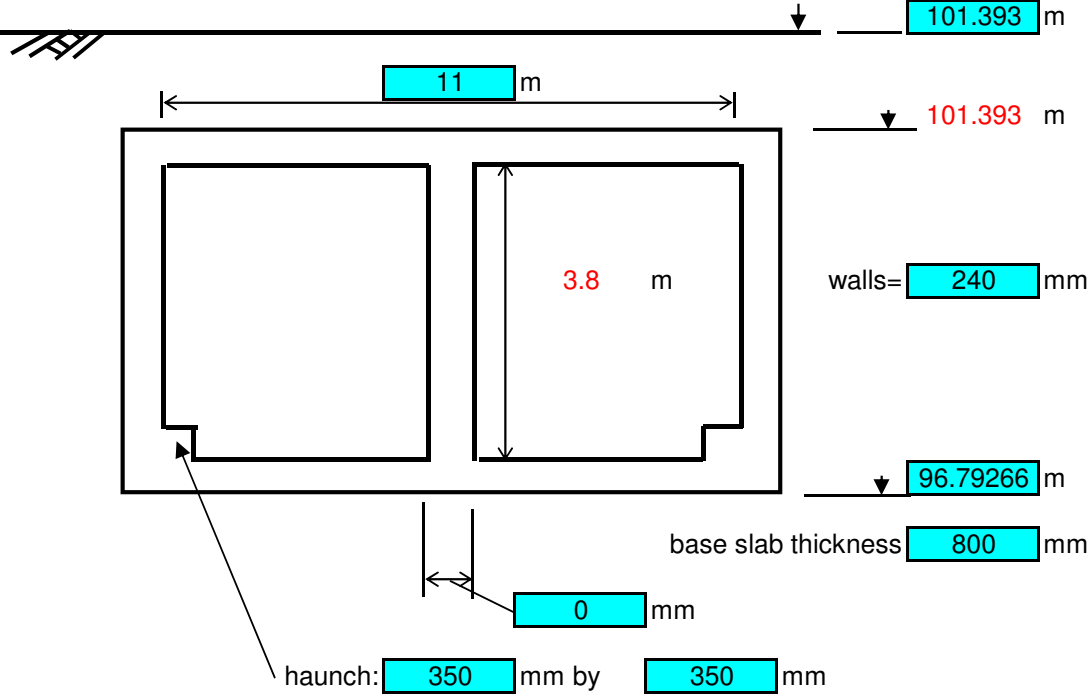
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 2536.0 | 60 | 152161.9 |
| concrete | m3 | 449.7419 | 220.0 | 98943.22 |
| rebar | tonnes | 54.0 | 1600 | 86350.45 |
| formwork/falsework | m2 | 380.8294 | 241 | 91779.88 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 434.8 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 393.7938 | 52 | 20477.28 |

Total 449712.7

Section Cut and Cover
 Length of section: 33.76147 m Section 8

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 1782.9 | m3 | |
| concrete= | 379.9178 | m3 | |
| rebar= | 45.6 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 256.5872 | m2 | |
| SP&L<=4.6m deep | 310.6 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 387.5817 | m2 | |

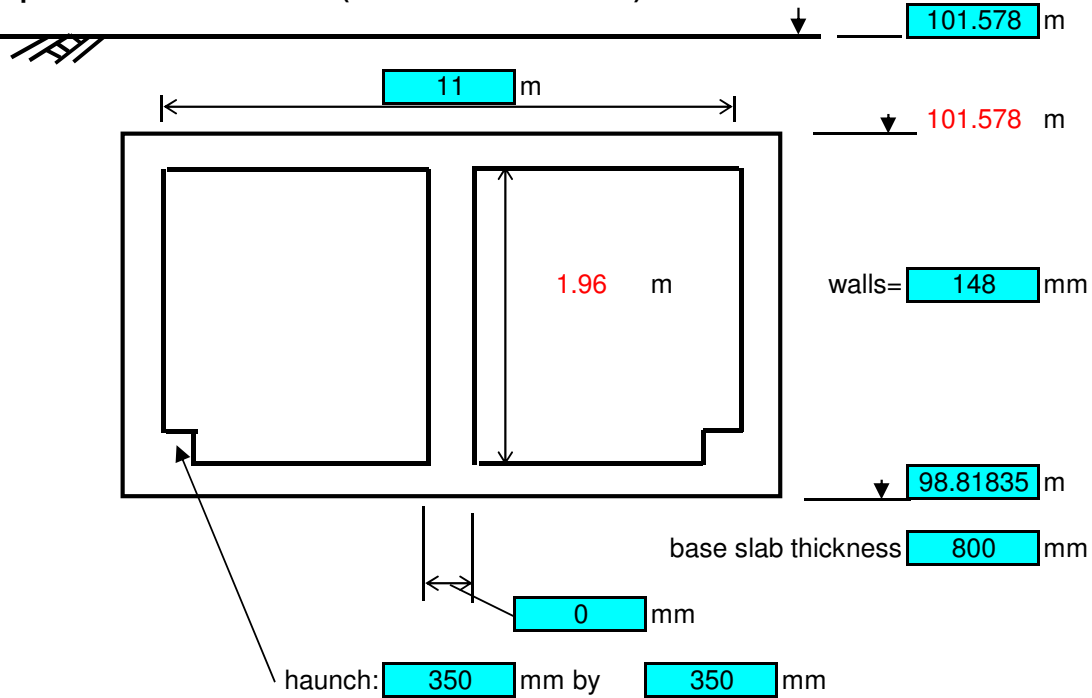
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 1782.9 | 60 | 106972.5 |
| concrete | m3 | 379.9178 | 220.0 | 83581.92 |
| rebar | tonnes | 45.6 | 1600 | 72944.22 |
| formwork/falsework | m2 | 256.5872 | 241 | 61837.5 |
| SP&L<=4.6m deep | m2 | 310.6 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 387.5817 | 52 | 20154.25 |

Total 345490.4

Section Cut and Cover
 Length of section: 33.76147 m Section 9

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 1052.6 | m3 | |
| concrete= | 332.9542 | m3 | |
| rebar= | 40.0 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 132.345 | m2 | |
| SP&L<=4.6m deep | 186.4 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 381.3695 | m2 | |

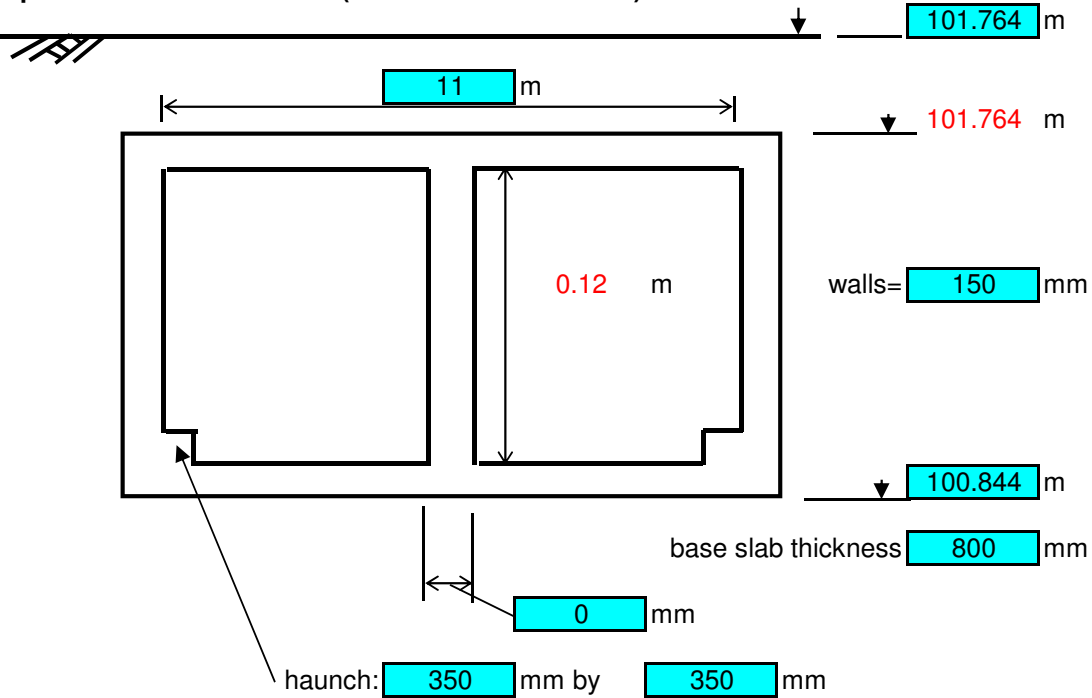
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 1052.6 | 60 | 63154.8 |
| concrete | m3 | 332.9542 | 220.0 | 73249.93 |
| rebar | tonnes | 40.0 | 1600 | 63927.22 |
| formwork/falsework | m2 | 132.345 | 241 | 31895.13 |
| SP&L<=4.6m deep | m2 | 186.4 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 381.3695 | 52 | 19831.22 |

Total 252058.3

Section Cut and Cover
 Length of section: 33.76147 m Section 10

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 351.0 | m3 | |
| concrete= | 314.6906 | m3 | |
| rebar= | 37.8 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 8.102752 | m2 | |
| SP&L<=4.6m deep | 62.1 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 381.5046 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 351.0 | 60 | 21059.05 |
| concrete | m3 | 314.6906 | 220.0 | 69231.94 |
| rebar | tonnes | 37.8 | 1600 | 60420.6 |
| formwork/falsework | m2 | 8.102752 | 241 | 1952.763 |
| SP&L<=4.6m deep | m2 | 62.1 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 381.5046 | 52 | 19838.24 |

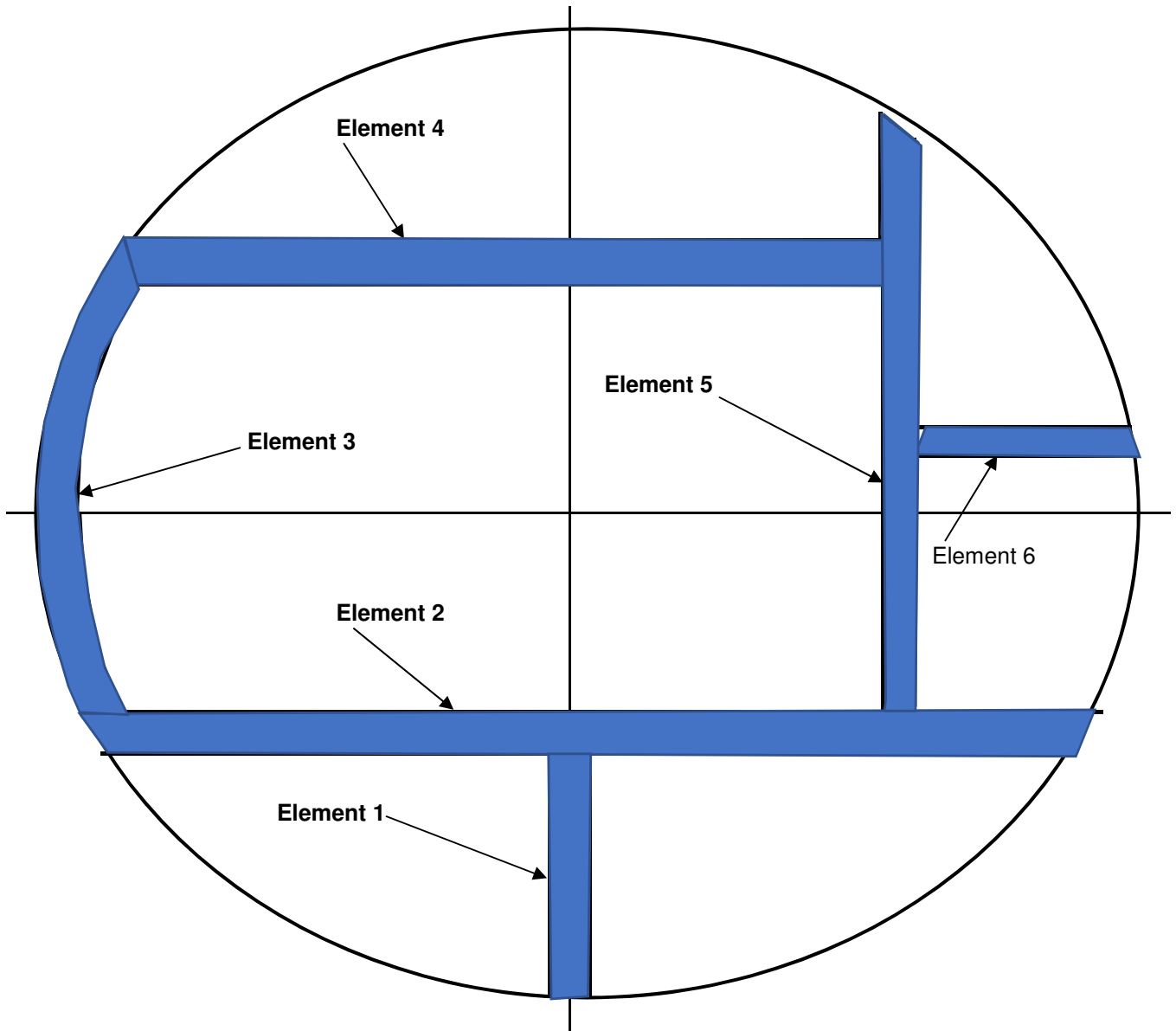
Total 172502.6

Summary of Costs

Markup for adjacent %

| Section | Cost |
|------------------|---------------------|
| 1 | \$ 1,310,610.3 |
| 2 | \$ 1,186,889.9 |
| 3 | \$ 1,018,836.7 |
| 4 | \$ 900,980.5 |
| 5 | \$ 760,953.4 |
| 6 | \$ 581,036.7 |
| 7 | \$ 449,712.7 |
| 8 | \$ 345,490.4 |
| 9 | \$ 252,058.3 |
| 10 | \$ 172,502.6 |
| Sub-total | \$ 6,979,072 |

Tunnel length= 21000 m



Assumed tunnel cross section

**Newfoundland Fixed Link Pre-feasibility Study
 Cost Estimating
 Single Lane Bored Highway Tunnel
 Tunnel Structural Finishes**

**Page 2 of 2
 Date: Oct. 31, 2017
 Calculation by:**

Quantity Take-off

| Concrete | | | | | Concrete | Rebar |
|----------|-----|-------|------|------|-------------------|-----------------|
| Element | Nr. | L(m) | b(m) | d(m) | Qty(m3) | Qty(m3) |
| 1 | 1 | 21000 | 0.5 | 2.5 | 26,250 | 3,150 |
| 2 | 1 | 21000 | 9.4 | 0.5 | 98,700 | 11,844 |
| 3 | 1 | 21000 | 0.3 | 4.8 | 30,240 | 3,629 |
| 4 | 1 | 21000 | 7.5 | 0.5 | 78,750 | 9,450 |
| 5 | 1 | 21000 | 0.3 | 6.6 | 41,580 | 4,990 |
| 6 | 1 | 21000 | 2.1 | 0.3 | 13,230 | 1,588 |
| | | | | | <u>288,750</u> m3 | <u>22,806</u> t |

Formwork/falsework

| Element | Nr. | L(m) | d(m) | Faces | Area(m2) |
|---------|-----|-------|------|-------|------------------|
| 1 | 1 | 21000 | 2.5 | 2 | 105000 |
| 2 | 2 | 21000 | 8.5 | 1 | 178500 |
| 3 | 1 | 21000 | 4.8 | 1 | 100800 |
| 4 | 2 | 21000 | 7 | 1 | 147000 |
| 5 | 2 | 21000 | 6.6 | 2 | 277200 |
| 6 | 1 | 21000 | 2.1 | 1 | 44100 |
| | | | | | <u>852600</u> m2 |

Rates

| | | |
|---------------|----|------|
| Concrete | m3 | 220 |
| Formwork | m2 | 180 |
| Reinforcement | t | 1600 |

Costs

| | | | | | | | |
|---------------|----|---------|----|----|------|---|-----------------------|
| Concrete | m3 | 288,750 | m3 | at | 220 | = | 63,525,000 |
| Formwork | m2 | 852,600 | m2 | at | 180 | = | 153,468,000 |
| Reinforcement | t | 22,806 | t | at | 1600 | = | 36,489,600 |
| | | | | | | | <u>\$ 253,482,600</u> |

**Main Electrical Components for Novaroute
Single TBM Bored Highway Tunnel Option**

Length of each tunnel= 21000 m
Tunnel width= 11 m

Number of tunnels= 1

| Item | Component | Unit | Qty | Unit Cost | Total Cost |
|------|--|------|--------|-----------|------------|
| | Lighting | | | | |
| | Threshold + transition (1st 700m) | m2 | 7280 | 315 | 2,293,200 |
| | Interior (balance) | m2 | 223300 | 120.00 | 26,796,000 |
| | Lighting subtotal | | | | 0 |
| | Substations, generators, UPS | | | | |
| | Substations | Ea | 2 | 1337000 | 2,674,000 |
| | Emergency generator | Ea | 1 | 955000 | 955,000 |
| | UPS (Battery system) | Ea | 1 | 573000 | 573,000 |
| | Substations, generators, UPS subtotal | | | | 4,202,000 |
| | CCTV system | | | | |
| | Cameras (every 60m) | Each | 350 | 3100 | 1,085,000 |
| | Control station | Each | 1 | 30000 | 30,000 |
| | CCTV system subtotal | | | | 1,115,000 |
| | Provide power for gas detection, ventilation etc. | | | | |
| | Power provision | m | 21000 | 45 | 945,000 |
| | Power subtotal | | | | 945,000 |
| | Lane control system | | | | |
| | Fibre optic display (every 200m) | Each | 105 | 12500 | 0 |
| | Lane control system subtotal | | | | 0 |

Total Electrical \$ 6,262,000

Prorating for Newfoundland - assuming *50% for single lane tunnel

for **21,000** m length of tunnel

| | | | | | |
|--------|----------------------------------|---------------------|-------|------------|-----------------------|
| 732 | m of threshold lighting costs \$ | 661,231.22 | 1,260 | lights | |
| 20,268 | m of interior lighting costs \$ | 1,812,524.13 | 3,445 | lights | |
| 21,000 | m of nighttime circuit costs \$ | 1,717,488.39 | 241 | lights | |
| 4,946 | lights cost \$ | 353,702.33 | | to instal | |
| 21,000 | m of conduit costs | <u>1,374,145.03</u> | | | |
| | | <u>5,919,091.09</u> | * | 0.5 | = \$ 2,959,546 |

**Newfoundland Fixed Link Pre-feasibility Study
Highway Tunnel - North Vent Adit
Tunnel Final Liner Cost Estimate**

Tunnel length= 600 m
Liner cross section area= 5.47 m²

Shift pattern

| Shifts | Hours | Days |
|--------|-------|------|
| 3 | 8 | 5 |

Advance rate= 30 m/day

Rebar ratio= 0.12 t/m³ of concrete

Concrete supply=\$ 190 /m³

Rebar supply=\$ 1200 /t

Initial form set-up time= 4 weeks

Durations

Number of days= 40 days
Number of hours= 960 hours
Number of weeks= 8 weeks

Labour

Crew size 15
Average labour rate \$ 76 /hour

Total labour cost=\$ 1,094,400

Equipment

Form \$ 1500000
Weekly cost of other equipment \$ 25000 (see TED 2370)

Total equipment cost=\$ 1,700,000

Materials

Concrete= 3279.8227 m³
Rebar= 393.57873

Concrete cost=\$ 623,166
Rebar cost=\$ 472,294

Total material cost=\$ 1,095,461

Cost Summary

| | |
|--------------|-------------------------|
| Labour | 1,094,400 |
| Equipment | 1,700,000 |
| Materials | 1,095,461 |
| Total | <u>3,889,861</u> |

Newfoundland Fixed Link Pre-feasibility Study
Highway Tunnel - North Vent Adit
Tunnel Drill and Blast Cost Estimate

Drill & blast excavation @ \$ 250 /m³

Tunnel length= 600 m

Tunnel excavated diameter= 6.5 m

Excavated volume= 19909.8 m³

Excavation cost=\$ 4,977,461

**Newfoundland Fixed Link Pre-feasibility Study
 Highway Tunnel - North Vent Adit Shaft
 Tunnel Drill and Blast Cost Estimate**

shaft excavated diameter= 5.5 m
 depth= 100 m
 shaft final diameter= 6.1 m

From graph

unlined shaft cost=\$ 17000 /m

Quantities

Concrete Base= 24 m3
 Shotcrete= 173 m3
 Rockbolts= 1728 m2
 final cast in place liner= 547 m3

Direct Costs

| | | | | \$ - Cost |
|-----------------------|------|---|----------------|---------------------------------|
| shaft excavation etc. | 100 | * | 17000 | 1700000 |
| Concrete Base | 24 | * | 150 | 3564 |
| Shotcrete | 173 | * | 500 | 86394 |
| Rockbolts | 1728 | * | 10 | 17279 |
| liner | 547 | * | 600 | 327982 (includes steel & forms) |
| Total Direct Cost= | | | <u>2135219</u> | |

Newfoundland Fixed Link Pre-feasibility Study
Highway Tunnel
TBM Bored Option - North Vent Adit

| | |
|----------------|-------------------|
| D&B Excavation | 4,977,461 |
| Liner | 3,889,861 |
| Shaft | 2,135,219 |
| | <u>11,002,540</u> |

**Newfoundland Fixed Link Pre-feasibility Study
Highway Tunnel - South Vent Adit
Tunnel Final Liner Cost Estimate**

Tunnel length= 2000 m
Liner cross section area= 5.47 m²

Shift pattern

| Shifts | Hours | Days |
|--------|-------|------|
| 3 | 8 | 5 |

Advance rate= 30 m/day

Rebar ratio= 0.12 t/m³ of concrete

Concrete supply=\$ 190 /m³

Rebar supply=\$ 1200 /t

Initial form set-up time= 4 weeks

Durations

Number of days= 87 days
Number of hours= 2080 hours
Number of weeks= 17 weeks

Labour

Crew size 15
Average labour rate \$ 76 /hour

Total labour cost=\$ 2,371,200

Equipment

Form \$ 1500000
Weekly cost of other equipment \$ 25000 (see TED 2370)

Total equipment cost=\$ 1,933,333

Materials

Concrete= 10932.742 m³
Rebar= 1311.9291

Concrete cost=\$ 2,077,221
Rebar cost=\$ 1,574,315

Total material cost=\$ 3,651,536

Cost Summary

| | |
|-----------|------------------|
| Labour | 2,371,200 |
| Equipment | 1,933,333 |
| Materials | 3,651,536 |
| Total | <u>7,956,069</u> |

**Newfoundland Fixed Link Pre-feasibility Study
Highway Tunnel - South Vent Adit
Tunnel Drill and Blast Cost Estimate**

Drill & blast excavation @ \$ 250 /m³

Tunnel length= 2000 m

Tunnel excavated diameter= 6.5 m

Excavated volume= 66366.1 m³

Excavation cost=\$ 16,591,536

**Newfoundland Fixed Link Pre-feasibility Study
 Highway Tunnel - South Vent Adit Shaft
 Tunnel Drill and Blast Cost Estimate**

shaft excavated diameter= 5.5 m
 depth= 0 m
 shaft final diameter= 6.1 m

From graph

unlined shaft cost=\$ 17000 /m

Quantities

Concrete Base= 24 m3
 Shotcrete= 0 m3
 Rockbolts= 0 m2
 final cast in place liner= 0 m3

Direct Costs

| | | | | \$ - Cost |
|-----------------------|----|---|-------|--------------------------|
| shaft excavation etc. | 0 | * | 17000 | 0 |
| Concrete Base | 24 | * | 150 | 3564 |
| Shotcrete | 0 | * | 500 | 0 |
| Rockbolts | 0 | * | 10 | 0 |
| liner | 0 | * | 600 | 0 |
| | | | | (includes steel & forms) |
| Total Direct Cost= | | | | <u>3564</u> |

Newfoundland Fixed Link Pre-feasibility Study
TBM Bored Highway Tunnel
TBM Bored Option - South Vent Adit

| | |
|----------------|-------------------|
| D&B Excavation | 16,591,536 |
| Liner | 7,956,069 |
| Shaft | 3,564 |
| | 24,551,169 |

Sump Sizing

Assume inflow to tunnel of **1** litres/m²/24hours

Tunnel length= **21000** m
Tunnel diameter= **11** m

24 hour inflow= **1995697** litres = **1996** m³
assume same again for firefighting= **1996** m³

Required sump capacity= **3991** m³

Assumed sump diameter= **6** m
Assumed number of sump structures= **2** m

Required length of each sump= **71** m

Piping

Assume **300** mm diameter steel pipe connecting each sump to the portal areas
Number of pipes= **1**
Assume sumps located at 1/3 and 2/3 of tunnel length

Total length of piping= **14300** m

Rates

Sump construction-\$ **17795** /m of sump
Pipe-\$ **400** /m of pipe
Pipe installation labour -\$ **36** /m of pipe (assume 6 person crew installing 100m/day)
Pipe installation equipment-\$ **20** /m of pipe (assume \$1000/day for equipment)
Pumps-\$ **125000** /pump

Costs

| Item | Unit | Qty | Rate | \$-Cost |
|-------------------|------|-------|--------|------------------|
| Sump construction | m | 142 | 17795 | 2,526,890 |
| Pipe | m | 14300 | 400 | 5,720,000 |
| Pipe installation | m | 14300 | 36 | 521,664 |
| Equipment | m | 14300 | 20 | 286,000 |
| Pumps | Nr | 4 | 125000 | 500,000 |
| Total | | | | 9,554,554 |

**Newfoundland Fixed Link Pre-feasibility Study
North Vehicle Holding Area**

Area= 300 m by 600 m

Earthworks

Assume 0.5 m depth cleared over entire area

Spoil excavation and removal @ \$ 30 /m³

Earthworks= 2,700,000

Surface

Assume surface @ 20 /m²

Surface= 3,600,000

Buildings etc.

| | | |
|------------------------------|--------|------------------|
| Public facilities building @ | 80000 | See CJT estimate |
| Site maintenance building @ | 220000 | See CJT estimate |
| Fire engines @ | 360000 | See CJT estimate |

Total cost 6,960,000

**Newfoundland Fixed Link Pre-feasibility Study
South Vehicle Holding Area**

Area= 300 m by 600 m

Earthworks

Assume 0 m depth cleared over entire area

Spoil excavation and removal @ \$ 30 /m³

Earthworks= 0

Surface

Assume surface @ 20 /m²

Surface= 3,600,000

Buildings etc.

Public facilities building @ 80000 See CJT estimate
Site maintenance building @ 220000 See CJT estimate
Fire engines @ 360000 See CJT estimate

Total cost 4,260,000

Appendix C1-2
Road Tunnel - Single-Bore, 2 TBMs
Construction Costs

Newfoundland Fixed Link Pre-feasibility - 2 TBM 2 sides Single Bored Highway Tunnel - Cost Summary

| BORED TUNNEL CONSTRUCTION COSTS | | |
|---|-------------|------------------------|
| ITEM | UNIT | MAIN TUNNEL |
| MOBILIZATION & DEMOBILIZATION | LS | 8,000,000 |
| TUNNELLING | LS | 646,428,999 |
| TUNNEL FINISHES | LS | 253,482,600 |
| NORTH APPROACH STRUCTURES | LS | 5,604,405 |
| SOUTH APPROACH STRUCTURES | LS | 6,979,072 |
| NORTH VENTILATION ADIT | LS | 11,002,540 |
| SOUTH VENTILATION ADIT | LS | 24,551,169 |
| ROAD FINISHES | LS | 4,304,750 |
| NORTH VEHICLE HOLDING AREA | LS | 6,960,000 |
| SOUTH VEHICLE HOLDING AREA | LS | 4,260,000 |
| TUNNEL DRAINAGE | LS | 9,554,554 |
| UTILITY DIVERSIONS | LS | 1,000,000 |
| MONITORING | LS | 1,000,000 |
| SUBTOTAL CIVIL | | \$983,128,090 |
| CIVIL CONTINGENCIES | | |
| CONTINGENCY | 40% | \$393,251,236 |
| TOTAL CIVIL | | \$1,376,379,326 |
| M&E AND FINISHING WORK | | |
| VENTILATION EQUIPMENT | LS | \$6,000,000 |
| VENTILATION BUILDINGS x 2 | LS | \$2,000,000 |
| FIRE SUPPRESSION SYSTEM | LS | \$4,000,000 |
| CONTROL CENTRE | LS | \$4,000,000 |
| SIGNALLING | LS | \$0 |
| LIGHTING | LS | \$2,959,546 |
| CCTV SYSTEM | LS | \$1,115,000 |
| GAS DETECTION | LS | \$945,000 |
| SUBSTATION, GENERATORS, UPS | LS | \$4,202,000 |
| SUBTOTAL M&E AND FINISHING | | \$25,221,546 |
| CONTINGENCIES | 20% | \$5,044,309 |
| TOTAL M&E AND FINISHING | | \$30,265,855 |
| TOTAL CIVIL, M&E AND FINISHING | | \$1,406,645,180 |
| ALLOWANCES | | |
| CONTRACTOR OH | 15% | \$210,996,777 |
| CONTRACTOR PROFIT | 15% | \$210,996,777 |
| CONSTRUCTION TOTAL | | \$1,828,638,734 |
| PRE-CONSTRUCTION AND SUPERVISION | | |
| FEASIBILITY STUDY | LS | \$17,000,000 |
| ENVIRONMENTAL ASSESSMENT | LS | \$6,000,000 |
| DESIGN | 5% | \$91,431,937 |
| CONSTRUCTION MANAGEMENT | 10% | \$182,863,873 |
| OWNERS COSTS | 2% | \$36,572,775 |
| PRE-CONSTRUCTION TOTAL | | \$333,868,585 |
| GRAND TOTAL | | \$2,162,507,319 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | 2TBM Set-up | Parent Estimate ID: | 7801 |
| Tunnel Name: | HWYTunnel 2TBMs Bore | Project Phase: | Conceptual |
| Construction Activity: | Erect TBM Only | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 02, 2017 |
| Estimate Definition ID: | 7807 | Tunnel Characteristics ID: | 3394 |

Tunnel Characteristics

Finished Diameter: 11 m

Activity Details

Shift Arrangement 3 - 8 hour shifts x 7 days per week

Duration of Activity 4.5 Weeks

Total Number of Shifts 94.5

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|--------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 756.00 | 2.00 | 135,384 |
| Tunnel miner | 76.06 | \$/hr | 756.00 | 4.00 | 230,005 |
| Shaft bottom | 75.29 | \$/hr | 756.00 | 6.00 | 341,515 |
| Tunnel fitter | 69.92 | \$/hr | 756.00 | 2.00 | 105,719 |
| Tunnel electrician | 84.43 | \$/hr | 756.00 | 2.00 | 127,658 |
| Shaft top | 75.29 | \$/hr | 756.00 | 2.00 | 113,838 |
| Crane operator | 77.86 | \$/hr | 756.00 | 4.00 | 235,449 |
| Surface laborer | 75.22 | \$/hr | 756.00 | 2.00 | 113,733 |
| Equipment laborer | 72.88 | \$/hr | 756.00 | 2.00 | 110,195 |
| | | | | 26.00 | \$1,513,497 |
| Equipment | | | | | |
| Loco | 8,000.00 | \$/wk | 4.50 | 2.00 | 72,000 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 4.50 | 12.00 | 156,600 |
| Flat cars | 480.00 | \$/wk | 4.50 | 4.00 | 8,640 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 4.50 | 2.00 | 9,900 |
| Small tools | 2,600.00 | \$/wk | 4.50 | 2.00 | 23,400 |
| Shaft crane | 13,900.00 | \$/wk | 4.50 | 2.00 | 125,100 |
| Erection crane | 10,000.00 | \$/wk | 4.50 | 2.00 | 90,000 |
| Compressors | 1,500.00 | \$/wk | 4.50 | 2.00 | 13,500 |
| Generators | 3,100.00 | \$/wk | 4.50 | 2.00 | 27,900 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 4.50 | 2.00 | 72,000 |
| Loaders | 3,550.00 | \$/wk | 4.50 | 2.00 | 31,950 |
| | | | | | \$630,990 |
| Materials | | | | | |
| Temporary materials | 3,000.00 | \$/wk | 4.50 | 2.00 | 27,000 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|-------|---------------|-------------------|-------------|
| Thrust frame | 7,000.00 | \$/wk | 4.50 | 2.00 | 63,000 |
| | | | | | \$90,000 |
| Total Estimated Cost: | | | | | \$2,234,487 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | 2 TBM HWY Tunnel | Parent Estimate ID: | 7800 |
| Tunnel Name: | HWYTunnel 2TBMs Bore | Project Phase: | Conceptual |
| Construction Activity: | TBM Tunneling | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 02, 2017 |
| Estimate Definition ID: | 7806 | Tunnel Characteristics ID: | 3394 |

Tunnel Characteristics

| | |
|-----------------------------------|-------------------|
| Tunnel Length: | 10,500 m |
| Finished Diameter: | 11 m |
| Initial Support Type: | Precast segmental |
| Initial Support Thickness: | 0 m |
| Final Lining Thickness: | 0.45 m |
| Grout Thickness: | 0.125 m |

Theoretical Excavation Volumes

| | |
|----------------------------------|------------------------|
| Total Neat Excavation: | 1,217,395 Cubic Metres |
| Initial Lining Volume: | 0 Cubic Metres |
| Final Lining Volume: | 169,964 Cubic Metres |
| Theoretical Grout Volume: | 49,583 Cubic Metres |

Normal Excavation/Support Cycle

| | |
|---------------------------------|------------|
| Excavation Cycle Length: | 1.5 Metres |
| Excavate: | 28 Minutes |
| Erect Support: | 36 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 64 Minutes |

Difficult Excavation/Support Cycle

| | |
|--|-------------|
| Length of Difficult Excavation: | 700 Metres |
| Excavate: | 92 Minutes |
| Erect Support: | 74 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 166 Minutes |

Reduction Factors

| | |
|--------------------------------------|---------|
| Machine availability: | 80 % |
| Back up efficiency: | 55 % |
| Planned maintenance: | 5 % |
| Learning curve efficiency: | 40 % |
| Learning curve duration time: | 8 Weeks |

| | |
|----------------------------------|------------|
| Learning Curve Rate: | 5.6 m/day |
| Experienced Advance Rate: | 14.1 m/day |
| Difficult Advance Rate: | 5.4 m/day |

TBM Skidding Through Excavation

| | |
|------------------------------|----------|
| Duration of skidding: | 0 Weeks |
| Length of skidding: | 0 Metres |

Advance Rate and Shift Details

| | Metres | Days |
|---|-------------------------------------|------|
| Shift Arrangement: | 3 - 8 hour shifts x 7 days per week | |
| Avg. Drive Advance per Shift: | 4.08 Metres | |
| Avg. Drive Advance per Day: | 12 Metres | |
| Avg. Drive Advance per Week: | 86 Metres | |
| Duration of Tunneling (Incl. Skid): | 122.42 Weeks | |
| Total number of shifts (Incl. Skid): | 2,571 | |
| Learning Curve Drive: | 316 | 56 |
| Experienced Drive: | 9,484 | 672 |
| Difficult Drive: | 700 | 129 |
| Skidding Portion: | 0 | 0 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|--------------|-------|---------------|-------------------|---------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 20,571.00 | 1.00 | 1,841,927 |
| Working foreperson | 89.54 | \$/hr | 20,571.00 | 2.00 | 3,683,855 |
| Tunnel miner | 76.06 | \$/hr | 20,571.00 | 3.00 | 4,693,891 |
| Tunnel laborer | 75.39 | \$/hr | 20,571.00 | 4.00 | 6,203,391 |
| Loco driver | 75.64 | \$/hr | 20,571.00 | 4.00 | 6,223,962 |
| Shaft bottom | 75.29 | \$/hr | 20,571.00 | 1.00 | 1,548,791 |
| TBM operator | 76.06 | \$/hr | 20,571.00 | 1.00 | 1,564,630 |
| Tunnel fitter | 69.92 | \$/hr | 20,571.00 | 1.00 | 1,438,324 |
| Tunnel electrician | 84.43 | \$/hr | 20,571.00 | 1.00 | 1,736,810 |
| Shaft top | 75.29 | \$/hr | 20,571.00 | 2.00 | 3,097,581 |
| Crane operator | 77.86 | \$/hr | 20,571.00 | 1.00 | 1,601,658 |
| Surface laborer | 75.22 | \$/hr | 20,571.00 | 4.00 | 6,189,402 |
| Equipment laborer | 72.88 | \$/hr | 20,571.00 | 4.00 | 5,996,858 |
| | | | | 29.00 | \$45,821,080 |
| Equipment | | | | | |
| TBM | 340,000.00 | \$/m2 | 115.94 | 1.00 | 39,419,600 |
| TBM optional equipment. | 1,500,000.00 | \$/Nr | 1.00 | 1.00 | 1,500,000 |
| Loco | 8,000.00 | \$/wk | 122.42 | 4.00 | 3,917,440 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 122.42 | 56.00 | 19,881,008 |
| Flat cars | 480.00 | \$/wk | 122.42 | 8.00 | 470,093 |
| Person riders | 480.00 | \$/wk | 122.42 | 2.00 | 117,523 |
| Track | 200.00 | \$/m | 10,500.00 | 1.00 | 2,100,000 |
| Air pipe | 46.00 | \$/m | 10,500.00 | 1.00 | 483,000 |
| Water pipe | 40.00 | \$/m | 10,500.00 | 1.00 | 420,000 |
| Pump main | 78.00 | \$/m | 10,500.00 | 1.00 | 819,000 |
| Cabling | 125.00 | \$/m | 10,500.00 | 1.00 | 1,312,500 |
| Lighting | 46.00 | \$/m | 10,500.00 | 1.00 | 483,000 |
| Vent ducting | 46.00 | \$/m | 10,500.00 | 1.00 | 483,000 |
| Grout mixers | 10,960.00 | \$/wk | 122.42 | 1.00 | 1,341,723 |
| Grout pumps | 5,250.00 | \$/wk | 122.42 | 1.00 | 642,705 |
| Grout hoses & pipes | 300.00 | \$/wk | 122.42 | 2.00 | 73,452 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 122.42 | 2.00 | 269,324 |
| Small tools | 4,000.00 | \$/wk | 122.42 | 1.00 | 489,680 |
| Shaft crane | 13,900.00 | \$/wk | 122.42 | 1.00 | 1,701,638 |
| Compressors | 1,500.00 | \$/wk | 122.42 | 1.00 | 183,630 |
| Low pressure C/A system | 5,900.00 | \$/wk | 122.42 | 1.00 | 722,278 |
| Pipework and controls | 1,000.00 | \$/wk | 122.42 | 2.00 | 244,840 |
| Generators | 3,100.00 | \$/wk | 122.42 | 1.00 | 379,502 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 122.42 | 1.00 | 979,360 |
| Surface fans | 1,200.00 | \$/wk | 122.42 | 2.00 | 293,808 |
| Loaders | 3,550.00 | \$/wk | 122.42 | 2.00 | 869,182 |
| Other surface plant | 4,000.00 | \$/wk | 122.42 | 1.00 | 489,680 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|-----------------------|-----------|--------|---------------|-------------------|---------------|
| Tunnel C/A system | 62,000.00 | \$/wk | 122.42 | 1.00 | 7,590,040 |
| | | | | | \$87,677,006 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 20,571.00 | 3,000.00 | 9,256,950 |
| Gas oil | 1.20 | \$/L | 48,000.00 | 1.00 | 57,600 |
| Lubrication materials | 140.00 | \$/wk | 122.42 | 1.00 | 17,139 |
| TBM spares, cutters | 390.00 | \$/m | 10,500.00 | 1.00 | 4,095,000 |
| Filters etc. | 465.00 | \$/wk | 122.42 | 1.00 | 56,925 |
| Hydraulic oil | 7.00 | \$/L | 32,000.00 | 1.00 | 224,000 |
| Other consumables | 200.00 | \$/wk | 122.42 | 1.00 | 24,484 |
| Tail seal grease | 200.00 | \$/m | 10,500.00 | 1.00 | 2,100,000 |
| | | | | | \$15,832,098 |
| Materials | | | | | |
| Concrete lining rings | 15,661.94 | \$/Nr | 7,000.00 | 1.00 | 109,633,597 |
| Gaskets | 170.00 | \$/m | 10,500.00 | 1.00 | 1,785,000 |
| Bolts | 20.00 | \$/Nr | 883.00 | 30.00 | 529,800 |
| Grout | 220.00 | \$/m3 | 49,583.00 | 1.00 | 10,908,260 |
| Grout plugs | 0.80 | \$/Nr | 883.00 | 7.00 | 4,945 |
| Packers | 15.00 | \$/Nr | 1,817.00 | 12.00 | 327,060 |
| Temporary materials | 3,000.00 | \$/wk | 122.42 | 1.00 | 367,260 |
| | | | | | \$123,555,922 |
| Subcontracts | | | | | |
| Soil disposal | 25.00 | \$/m3 | 1,217,395.00 | 1.50 | 45,652,313 |
| | | | | | \$45,652,313 |

Total Estimated Cost: \$318,538,418
Total Estimated Cost per Metre: \$30,337
Total Estimated Cost per Week: \$2,601,936
Total Estimated Cost per Shift: \$123,878

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | 2TBM Maintenance | Parent Estimate ID: | 7802 |
| Tunnel Name: | HWYTunnel 2TBMs Bore | Project Phase: | Conceptual |
| Construction Activity: | TBM Maintenance | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 02, 2017 |
| Estimate Definition ID: | 7808 | Tunnel Characteristics ID: | 3394 |

Tunnel Characteristics

Finished Diameter: 11 m

Activity Details

Shift Arrangement 1 - 6 hour shifts x 1 days per week

Duration of Activity 122 Weeks

Total Number of Shifts 122

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|--------|---------------|-------------------|--------------------|
| Labor | | | | | |
| Working foreperson | 89.54 | \$/hr | 732.00 | 2.00 | 131,087 |
| Loco driver | 75.64 | \$/hr | 732.00 | 2.00 | 110,737 |
| Shaft bottom | 75.29 | \$/hr | 732.00 | 2.00 | 110,225 |
| TBM operator | 76.06 | \$/hr | 732.00 | 2.00 | 111,352 |
| Tunnel fitter | 69.92 | \$/hr | 732.00 | 2.00 | 102,363 |
| Tunnel electrician | 84.43 | \$/hr | 732.00 | 2.00 | 123,606 |
| Shaft top | 75.29 | \$/hr | 732.00 | 2.00 | 110,225 |
| Surface laborer | 75.22 | \$/hr | 732.00 | 2.00 | 110,122 |
| | | | | 16.00 | \$909,715 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 732.00 | 600.00 | 65,880 |
| | | | | | \$65,880 |
| Materials | | | | | |
| Temporary materials | 500.00 | \$/wk | 122.00 | 2.00 | 122,000 |
| | | | | | \$122,000 |
| Total Estimated Cost: | | | | | \$1,097,595 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | 2TBM Removal | Parent Estimate ID: | 7803 |
| Tunnel Name: | HWYTunnel 2TBMs Bore | Project Phase: | Conceptual |
| Construction Activity: | TBM Removal | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 02, 2017 |
| Estimate Definition ID: | 7809 | Tunnel Characteristics ID: | 3394 |

Tunnel Characteristics

Finished Diameter: 11 m

Activity Details

Shift Arrangement 3 - 8 hour shifts x 7 days per week

Duration of Activity 3 Weeks

Total Number of Shifts 63

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|--------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 504.00 | 2.00 | 90,256 |
| Tunnel miner | 76.06 | \$/hr | 504.00 | 6.00 | 230,005 |
| Shaft bottom | 75.29 | \$/hr | 504.00 | 4.00 | 151,785 |
| Tunnel fitter | 69.92 | \$/hr | 504.00 | 2.00 | 70,479 |
| Tunnel electrician | 84.43 | \$/hr | 504.00 | 2.00 | 85,105 |
| Shaft top | 75.29 | \$/hr | 504.00 | 4.00 | 151,785 |
| Crane operator | 77.86 | \$/hr | 504.00 | 4.00 | 156,966 |
| Surface laborer | 75.22 | \$/hr | 504.00 | 4.00 | 151,644 |
| Equipment laborer | 72.88 | \$/hr | 504.00 | 2.00 | 73,463 |
| | | | | 30.00 | \$1,161,488 |
| Equipment | | | | | |
| Loco | 8,000.00 | \$/wk | 3.00 | 2.00 | 48,000 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 3.00 | 12.00 | 104,400 |
| Flat cars | 480.00 | \$/wk | 3.00 | 8.00 | 11,520 |
| Person riders | 480.00 | \$/wk | 3.00 | 2.00 | 2,880 |
| Booster fans | 1,500.00 | \$/wk | 3.00 | 2.00 | 9,000 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 3.00 | 2.00 | 6,600 |
| Other plant | 2,200.00 | \$/wk | 3.00 | 2.00 | 13,200 |
| Hoists operator | 3,000.00 | \$/wk | 3.00 | 2.00 | 18,000 |
| Shaft crane | 13,900.00 | \$/wk | 3.00 | 2.00 | 83,400 |
| 50T Crane | 7,000.00 | \$/wk | 3.00 | 2.00 | 42,000 |
| TBM Crane | 20,000.00 | \$/wk | 3.00 | 2.00 | 120,000 |
| Compressors | 1,500.00 | \$/wk | 3.00 | 2.00 | 9,000 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 3.00 | 2.00 | 48,000 |
| Surface fans | 1,200.00 | \$/wk | 3.00 | 2.00 | 7,200 |
| | | | | | \$523,200 |

Estimate Definition ID: 7809

Estimated by: _____

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Page 1 of 2

Checked by: _____

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|-------|---------------|-------------------|--------------------|
| Consumables | | | | | |
| Gas oil | 1.20 | \$/L | 15.00 | 2,000.00 | 36,000 |
| Other consumables | 500.00 | \$/wk | 3.00 | 2.00 | 3,000 |
| | | | | | <u>39,000</u> |
| Materials | | | | | |
| Temporary materials | 800.00 | \$/wk | 3.00 | 2.00 | 4,800 |
| | | | | | <u>4,800</u> |
| General Supplies | | | | | |
| Small tools | 700.00 | \$/wk | 3.00 | 2.00 | 4,200 |
| | | | | | <u>4,200</u> |
| Total Estimated Cost: | | | | | \$1,732,688 |

**Appendix C1-3
Road Tunnel - Twin-Bore
Construction Costs**

Newfoundland Fixed Link Pre-feasibility - Twin TBM Bored Highway Tunnel - Cost Summary

| BORED TUNNEL CONSTRUCTION COSTS | | |
|---|-------------|------------------------|
| ITEM | UNIT | MAIN TUNNEL |
| MOBILIZATION & DEMOBILIZATION | LS | 8,000,000 |
| TUNNELLING | LS | 1,200,612,716 |
| TUNNEL FINISHES | LS | 506,965,200 |
| NORTH APPROACH STRUCTURES | LS | 11,208,810 |
| SOUTH APPROACH STRUCTURES | LS | 13,958,143 |
| NORTH VENTILATION ADIT | LS | 11,002,540 |
| SOUTH VENTILATION ADIT | LS | 24,551,169 |
| ROAD FINISHES | LS | 4,304,750 |
| NORTH VEHICLE HOLDING AREA | LS | 6,960,000 |
| SOUTH VEHICLE HOLDING AREA | LS | 4,260,000 |
| TUNNEL DRAINAGE | LS | 18,472,164 |
| UTILITY DIVERSIONS | LS | 1,200,000 |
| MONITORING | LS | 1,200,000 |
| SUBTOTAL CIVIL | | \$1,812,695,493 |
| CIVIL CONTINGENCIES | | |
| CONTINGENCY | 40% | \$725,078,197 |
| TOTAL CIVIL | | \$2,537,773,690 |
| M&E AND FINISHING WORK | | |
| VENTILATION EQUIPMENT | LS | \$12,000,000 |
| VENTILATION BUILDINGS x 2 | LS | \$4,000,000 |
| FIRE SUPPRESSION SYSTEM | LS | \$8,000,000 |
| CONTROL CENTRE | LS | \$8,000,000 |
| SIGNALLING | LS | \$0 |
| LIGHTING | LS | \$5,571,934 |
| CCTV SYSTEM | LS | \$2,230,000 |
| GAS DETECTION | LS | \$1,890,000 |
| SUBSTATION, GENERATORS, UPS | LS | \$4,202,000 |
| SUBTOTAL M&E AND FINISHING | | \$45,893,934 |
| CONTINGENCIES | 20% | \$9,178,787 |
| TOTAL M&E AND FINISHING | | \$55,072,721 |
| TOTAL CIVIL, M&E AND FINISHING | | \$2,592,846,411 |
| ALLOWANCES | | |
| CONTRACTOR OH | 15% | \$388,926,962 |
| CONTRACTOR PROFIT | 15% | \$388,926,962 |
| CONSTRUCTION TOTAL | | \$3,371,000,000 |
| PRE-CONSTRUCTION AND SUPERVISION | | |
| FEASIBILITY STUDY | LS | \$17,000,000 |
| ENVIRONMENTAL ASSESSMENT | LS | \$6,000,000 |
| DESIGN | 5% | \$168,550,000 |
| CONSTRUCTION MANAGEMENT | 10% | \$337,100,000 |
| OWNERS COSTS | 2% | \$67,420,000 |
| PRE-CONSTRUCTION TOTAL | | \$596,070,000 |
| GRAND TOTAL | | \$3,967,070,000 |

APPENDIX C2

Rail Tunnel

**Appendix C2-1
Rail Tunnel - Single-Bore, 1 TBM
Construction Costs**

Newfoundland Fixed Link Pre-feasibility - Single Bored -1TBM Railway Tunnel - Cost Summary

BORED TUNNEL CONSTRUCTION COSTS

| ITEM | UNIT | MAIN TUNNEL |
|---|-------------|------------------------|
| MOBILIZATION & DEMOBILIZATION | LS | 8,000,000 |
| TUNNELLING | LS | 565,272,196 |
| TUNNEL FINISHES | LS | 105,382,800 |
| NORTH APPROACH STRUCTURES | LS | 9,144,099 |
| SOUTH APPROACH STRUCTURES | LS | 9,583,984 |
| RAIL TRACK | LS | 13,923,100 |
| TUNNEL DRAINAGE | LS | 11,307,454 |
| UTILITY DIVERSIONS | LS | 1,000,000 |
| MONITORING | LS | 1,000,000 |
| SUBTOTAL CIVIL | | \$724,613,633 |
| CIVIL CONTINGENCIES | | |
| CONTINGENCY | 40% | \$289,845,453 |
| TOTAL CIVIL | | \$1,014,459,087 |
| M&E, ROLLING STOCK, RAIL HARDWARE AND FINISHING WORK | | |
| ROLLING STOCK, TERMINALS, OCS, ETC | LS | \$48,773,000 |
| VENTILATION EQUIPMENT | LS | \$3,000,000 |
| VENTILATION SHAFTS AND BUILDINGS x 2 | LS | \$0 |
| FIRE SUPPRESSION SYSTEM | LS | \$2,000,000 |
| CONTROL CENTRE | LS | \$1,000,000 |
| SIGNALLING | LS | \$1,000,000 |
| LIGHTING | LS | \$1,000,000 |
| CCTV SYSTEM | LS | \$0 |
| GAS DETECTION | LS | \$900,000 |
| SUBSTATION, GENERATORS, UPS | LS | \$2,000,000 |
| SUBTOTAL M&E AND FINISHING | | \$59,673,000 |
| CONTINGENCIES | 20% | \$11,934,600 |
| TOTAL M&E AND FINISHING | | \$71,607,600 |
| TOTAL CIVIL, M&E AND FINISHING | | \$1,086,066,687 |
| ALLOWANCES | | |
| CONTRACTOR OH | 15% | \$162,910,003 |
| CONTRACTOR PROFIT | 15% | \$162,910,003 |
| CONSTRUCTION TOTAL | | \$1,411,886,693 |
| PRE-CONSTRUCTION AND SUPERVISION | | |
| FEASIBILITY STUDY | LS | \$17,000,000 |
| ENVIRONMENTAL ASSESSMENT | LS | \$6,000,000 |
| DESIGN | 5% | \$70,594,335 |
| CONSTRUCTION MANAGEMENT | 10% | \$141,188,669 |
| OWNERS COSTS | 2% | \$28,237,734 |
| PRE-CONSTRUCTION TOTAL | | \$263,020,738 |
| GRAND TOTAL | | \$1,674,907,430 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Rail-Sgl.Bore Set up | Parent Estimate ID: | 7801 |
| Tunnel Name: | Rail-Single BoreT | Project Phase: | Conceptual |
| Construction Activity: | Erect TBM Only | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 06, 2017 |
| Estimate Definition ID: | 7811 | Tunnel Characteristics ID: | 3395 |

Tunnel Characteristics

Finished Diameter: 7.6 m

Activity Details

Shift Arrangement 3 - 8 hour shifts x 7 days per week

Duration of Activity 4.5 Weeks

Total Number of Shifts 94.5

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 756.00 | 1.00 | 67,692 |
| Tunnel miner | 76.06 | \$/hr | 756.00 | 2.00 | 115,003 |
| Shaft bottom | 75.29 | \$/hr | 756.00 | 3.00 | 170,758 |
| Tunnel fitter | 69.92 | \$/hr | 756.00 | 1.00 | 52,860 |
| Tunnel electrician | 84.43 | \$/hr | 756.00 | 1.00 | 63,829 |
| Shaft top | 75.29 | \$/hr | 756.00 | 1.00 | 56,919 |
| Crane operator | 77.86 | \$/hr | 756.00 | 2.00 | 117,724 |
| Surface laborer | 75.22 | \$/hr | 756.00 | 1.00 | 56,866 |
| Equipment laborer | 72.88 | \$/hr | 756.00 | 1.00 | 55,097 |
| | | | | 13.00 | \$756,748 |
| Equipment | | | | | |
| Loco | 8,000.00 | \$/wk | 4.50 | 1.00 | 36,000 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 4.50 | 6.00 | 78,300 |
| Flat cars | 480.00 | \$/wk | 4.50 | 2.00 | 4,320 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 4.50 | 1.00 | 4,950 |
| Small tools | 2,600.00 | \$/wk | 4.50 | 1.00 | 11,700 |
| Shaft crane | 13,900.00 | \$/wk | 4.50 | 1.00 | 62,550 |
| Erection crane | 10,000.00 | \$/wk | 4.50 | 1.00 | 45,000 |
| Compressors | 1,500.00 | \$/wk | 4.50 | 1.00 | 6,750 |
| Generators | 3,100.00 | \$/wk | 4.50 | 1.00 | 13,950 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 4.50 | 1.00 | 36,000 |
| Loaders | 3,550.00 | \$/wk | 4.50 | 1.00 | 15,975 |
| | | | | | \$315,495 |
| Materials | | | | | |
| Temporary materials | 3,000.00 | \$/wk | 4.50 | 1.00 | 13,500 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|-------|---------------|-------------------|-------------|
| Thrust frame | 7,000.00 | \$/wk | 4.50 | 1.00 | 31,500 |
| | | | | | \$45,000 |
| Total Estimated Cost: | | | | | \$1,117,243 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Rail-SinglBore Tnl | Parent Estimate ID: | 7800 |
| Tunnel Name: | Rail-Single BoreT | Project Phase: | Conceptual |
| Construction Activity: | TBM Tunneling | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 06, 2017 |
| Estimate Definition ID: | 7810 | Tunnel Characteristics ID: | 3395 |

Tunnel Characteristics

| | |
|-----------------------------------|----------------|
| Tunnel Length: | 30,000 m |
| Finished Diameter: | 7.6 m |
| Initial Support Type: | Not Applicable |
| Initial Support Thickness: | 0 m |
| Final Lining Thickness: | 0.35 m |
| Grout Thickness: | 0.1 m |

Theoretical Excavation Volumes

| | |
|----------------------------------|------------------------|
| Total Neat Excavation: | 1,702,349 Cubic Metres |
| Initial Lining Volume: | 0 Cubic Metres |
| Final Lining Volume: | 262,244 Cubic Metres |
| Theoretical Grout Volume: | 79,168 Cubic Metres |

Normal Excavation/Support Cycle

| | |
|---------------------------------|------------|
| Excavation Cycle Length: | 1.5 Metres |
| Excavate: | 24 Minutes |
| Erect Support: | 27 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 51 Minutes |

Difficult Excavation/Support Cycle

| | |
|--|-------------|
| Length of Difficult Excavation: | 1400 Metres |
| Excavate: | 73 Minutes |
| Erect Support: | 54 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 127 Minutes |

Reduction Factors

| | |
|--------------------------------------|---------|
| Machine availability: | 80 % |
| Back up efficiency: | 55 % |
| Planned maintenance: | 5 % |
| Learning curve efficiency: | 40 % |
| Learning curve duration time: | 8 Weeks |

| | |
|----------------------------------|------------|
| Learning Curve Rate: | 7.1 m/day |
| Experienced Advance Rate: | 17.7 m/day |
| Difficult Advance Rate: | 7.1 m/day |

TBM Skidding Through Excavation

| | |
|------------------------------|----------|
| Duration of skidding: | 0 Weeks |
| Length of skidding: | 0 Metres |

Advance Rate and Shift Details

| | |
|---|-------------------------------------|
| Shift Arrangement: | 3 - 8 hour shifts x 7 days per week |
| Avg. Drive Advance per Shift: | 5.42 Metres |
| Avg. Drive Advance per Day: | 16 Metres |
| Avg. Drive Advance per Week: | 114 Metres |
| Duration of Tunneling (Incl. Skid): | 263.72 Weeks |
| Total number of shifts (Incl. Skid): | 5,539 |

| | Metres | Days |
|------------------------------|--------|-------|
| Learning Curve Drive: | 397 | 56 |
| Experienced Drive: | 28,203 | 1,593 |
| Difficult Drive: | 1,400 | 197 |
| Skidding Portion: | 0 | 0 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|--------------|-------|---------------|-------------------|---------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 44,309.00 | 1.00 | 3,967,428 |
| Working foreperson | 89.54 | \$/hr | 44,309.00 | 2.00 | 7,934,856 |
| Tunnel miner | 76.06 | \$/hr | 44,309.00 | 3.00 | 10,110,428 |
| Tunnel laborer | 75.39 | \$/hr | 44,309.00 | 4.00 | 13,361,822 |
| Loco driver | 75.64 | \$/hr | 44,309.00 | 4.00 | 13,406,131 |
| Shaft bottom | 75.29 | \$/hr | 44,309.00 | 1.00 | 3,336,025 |
| TBM operator | 76.06 | \$/hr | 44,309.00 | 1.00 | 3,370,143 |
| Tunnel fitter | 69.92 | \$/hr | 44,309.00 | 1.00 | 3,098,085 |
| Tunnel electrician | 84.43 | \$/hr | 44,309.00 | 1.00 | 3,741,009 |
| Shaft top | 75.29 | \$/hr | 44,309.00 | 2.00 | 6,672,049 |
| Crane operator | 77.86 | \$/hr | 44,309.00 | 1.00 | 3,449,899 |
| Surface laborer | 75.22 | \$/hr | 44,309.00 | 4.00 | 13,331,692 |
| Equipment laborer | 72.88 | \$/hr | 44,309.00 | 4.00 | 12,916,960 |
| | | | | 29.00 | \$98,696,525 |
| Equipment | | | | | |
| TBM | 340,000.00 | \$/m2 | 56.74 | 1.00 | 19,291,600 |
| TBM optional equipment. | 1,500,000.00 | \$/Nr | 1.00 | 1.00 | 1,500,000 |
| Loco | 8,000.00 | \$/wk | 263.72 | 5.00 | 10,548,800 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 263.72 | 56.00 | 42,828,128 |
| Flat cars | 480.00 | \$/wk | 263.72 | 8.00 | 1,012,685 |
| Person riders | 480.00 | \$/wk | 263.72 | 2.00 | 253,171 |
| Track | 200.00 | \$/m | 30,000.00 | 1.00 | 6,000,000 |
| Air pipe | 46.00 | \$/m | 30,000.00 | 1.00 | 1,380,000 |
| Water pipe | 40.00 | \$/m | 30,000.00 | 1.00 | 1,200,000 |
| Pump main | 78.00 | \$/m | 30,000.00 | 1.00 | 2,340,000 |
| Cabling | 125.00 | \$/m | 30,000.00 | 1.00 | 3,750,000 |
| Lighting | 46.00 | \$/m | 30,000.00 | 1.00 | 1,380,000 |
| Vent ducting | 46.00 | \$/m | 30,000.00 | 1.00 | 1,380,000 |
| Grout mixers | 10,960.00 | \$/wk | 263.72 | 1.00 | 2,890,371 |
| Grout pumps | 5,250.00 | \$/wk | 263.72 | 1.00 | 1,384,530 |
| Grout hoses & pipes | 300.00 | \$/wk | 263.72 | 2.00 | 158,232 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 263.72 | 2.00 | 580,184 |
| Small tools | 4,000.00 | \$/wk | 263.72 | 1.00 | 1,054,880 |
| Shaft crane | 13,900.00 | \$/wk | 263.72 | 1.00 | 3,665,708 |
| Compressors | 1,500.00 | \$/wk | 263.72 | 1.00 | 395,580 |
| Low pressure C/A system | 5,900.00 | \$/wk | 263.72 | 1.00 | 1,555,948 |
| Pipework and controls | 1,000.00 | \$/wk | 263.72 | 2.00 | 527,440 |
| Generators | 3,100.00 | \$/wk | 263.72 | 1.00 | 817,532 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 263.72 | 1.00 | 2,109,760 |
| Surface fans | 1,200.00 | \$/wk | 263.72 | 2.00 | 632,928 |
| Loaders | 3,550.00 | \$/wk | 263.72 | 2.00 | 1,872,412 |
| Other surface plant | 4,000.00 | \$/wk | 263.72 | 1.00 | 1,054,880 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|-----------------------|-----------|--------|---------------|-------------------|---------------|
| Tunnel C/A system | 62,000.00 | \$/wk | 263.72 | 1.00 | 16,350,640 |
| | | | | | \$127,915,409 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 44,309.00 | 3,000.00 | 19,939,050 |
| Gas oil | 1.20 | \$/L | 48,000.00 | 1.00 | 57,600 |
| Lubrication materials | 140.00 | \$/wk | 263.72 | 1.00 | 36,921 |
| TBM spares, cutters | 390.00 | \$/m | 30,000.00 | 1.00 | 11,700,000 |
| Filters etc. | 465.00 | \$/wk | 263.72 | 1.00 | 122,630 |
| Hydraulic oil | 7.00 | \$/L | 32,000.00 | 1.00 | 224,000 |
| Other consumables | 200.00 | \$/wk | 263.72 | 1.00 | 52,744 |
| Tail seal grease | 200.00 | \$/m | 30,000.00 | 1.00 | 6,000,000 |
| | | | | | \$38,132,945 |
| Materials | | | | | |
| Concrete lining rings | 10,066.82 | \$/Nr | 20,000.00 | 1.00 | 201,336,364 |
| Gaskets | 170.00 | \$/m | 30,000.00 | 1.00 | 5,100,000 |
| Bolts | 20.00 | \$/Nr | 1,765.00 | 30.00 | 1,059,000 |
| Grout | 220.00 | \$/m3 | 79,168.00 | 1.00 | 17,416,960 |
| Grout plugs | 0.80 | \$/Nr | 1,765.00 | 7.00 | 9,884 |
| Packers | 15.00 | \$/Nr | 3,633.00 | 12.00 | 653,940 |
| Temporary materials | 3,000.00 | \$/wk | 263.72 | 1.00 | 791,160 |
| Other materials | 0.00 | \$/t | 0.00 | 1.00 | 0 |
| | | | | | \$226,367,308 |
| Subcontracts | | | | | |
| Soil disposal | 25.00 | \$/m3 | 1,702,349.00 | 1.50 | 63,838,088 |
| | | | | | \$63,838,088 |

Total Estimated Cost: \$554,950,274

Total Estimated Cost per Metre: \$18,498

Total Estimated Cost per Week: \$2,104,336

Total Estimated Cost per Shift: \$100,197

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Rail-SglB.-TBM Maint | Parent Estimate ID: | 7802 |
| Tunnel Name: | Rail-Single BoreT | Project Phase: | Conceptual |
| Construction Activity: | TBM Maintenance | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 06, 2017 |
| Estimate Definition ID: | 7812 | Tunnel Characteristics ID: | 3395 |

Tunnel Characteristics

Finished Diameter: 7.6 m

Activity Details

Shift Arrangement 1 - 6 hour shifts x 1 days per week

Duration of Activity 264 Weeks

Total Number of Shifts 264

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|--------|---------------|-------------------|--------------------|
| Labor | | | | | |
| Working foreperson | 89.54 | \$/hr | 1,584.00 | 1.50 | 212,747 |
| Loco driver | 75.64 | \$/hr | 1,584.00 | 1.50 | 179,721 |
| Shaft bottom | 75.29 | \$/hr | 1,584.00 | 1.50 | 178,889 |
| TBM operator | 76.06 | \$/hr | 1,584.00 | 1.50 | 180,719 |
| Tunnel fitter | 69.92 | \$/hr | 1,584.00 | 1.50 | 166,130 |
| Tunnel electrician | 84.43 | \$/hr | 1,584.00 | 1.50 | 200,606 |
| Shaft top | 75.29 | \$/hr | 1,584.00 | 1.50 | 178,889 |
| Surface laborer | 75.22 | \$/hr | 1,584.00 | 1.50 | 178,723 |
| | | | | 12.00 | \$1,476,423 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 1,584.00 | 600.00 | 142,560 |
| Other consumables | 0.00 | \$/wk | 264.00 | 1.00 | 0 |
| | | | | | \$142,560 |
| Materials | | | | | |
| Temporary materials | 500.00 | \$/wk | 264.00 | 1.00 | 132,000 |
| | | | | | \$132,000 |
| Total Estimated Cost: | | | | | \$1,750,983 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Rail-SglBoreTnICl up | Parent Estimate ID: | 7804 |
| Tunnel Name: | Rail-Single BoreT | Project Phase: | Conceptual |
| Construction Activity: | Tunnel Clean Up | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 06, 2017 |
| Estimate Definition ID: | 7814 | Tunnel Characteristics ID: | 3395 |

Tunnel Characteristics

| | |
|---------------------------------|---|
| Tunnel Length: | 30,000 m |
| Finished Diameter: | 7.6 m (Circular Tunnels) |
| Excavated Cross Section: | 0 m ² (Non-circular Tunnels) |
| Excavated Perimeter: | 0 m (Non-circular Tunnels) |

Productivity Cycle

| | |
|-----------------------------------|-------------|
| Section Length | 30 Metres |
| Vent Line Removal Time | 120 Minutes |
| Track Removal Time | 60 Minutes |
| Temp Lighting Removal Time | 60 Minutes |
| Clean Up Time | 120 Minutes |
| Total Cycle Time | 360 Minutes |

Reduction Factors

| | |
|-----------------------------------|---------|
| Learning Curve Efficiency: | 50 % |
| Back Up Efficiency: | 80 % |
| Learning Curve Duration: | 1 Weeks |

Shift Details

| | |
|--------------------------------|-------------------------------------|
| Shift Arrangement: | 3 - 8 hour shifts x 7 days per week |
| Avg. Advance per Shift: | 31.63 Metres |
| Avg. Advance per Week: | 665 Metres |
| Total number of hours: | 7,588 |

Clean Up Productivity Data

| | <u>Average Advance</u> | <u>Drive Length</u> | <u>Drive Duration</u> | | |
|-----------------------------------|------------------------|---------------------|-----------------------|----------|-------------|
| Learning Curve Portion: | 48.0 m/day | 336 Metres | 21 Shifts | 7 Days | 1.00 Weeks |
| Experienced Drive Portion: | 96.0 m/day | 29,664 Metres | 927 Shifts | 309 Days | 44.14 Weeks |
| Total: | 94.9 m/day | 30,000 Metres | 949 Shifts | 316 Days | 45.14 Weeks |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|--------------------|
| Labor | | | | | |
| Tunnel laborer | 75.39 | \$/hr | 7,588.00 | 6.00 | 3,432,356 |
| Shaft bottom | 75.29 | \$/hr | 7,588.00 | 1.00 | 571,301 |
| Shaft top | 75.29 | \$/hr | 7,588.00 | 1.00 | 571,301 |
| Crane operator | 77.86 | \$/hr | 7,588.00 | 1.00 | 590,802 |
| | | | | 9.00 | \$5,165,759 |
| Equipment | | | | | |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 45.14 | 1.00 | 49,654 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|---------------------|-----------|-------|---------------|-------------------|-------------|
| Hoists operator | 2,600.00 | \$/wk | 45.14 | 1.00 | 117,364 |
| Shaft crane | 13,900.00 | \$/wk | 45.14 | 1.00 | 627,446 |
| Compressors | 1,500.00 | \$/wk | 45.14 | 1.00 | 67,710 |
| Loaders | 3,550.00 | \$/wk | 45.14 | 1.00 | 160,247 |
| Other surface plant | 3,000.00 | \$/wk | 45.14 | 1.00 | 135,420 |
| Bobcat | 800.00 | \$/wk | 45.14 | 1.00 | 36,112 |
| | | | | | \$1,193,953 |

Consumables

| | | | | | |
|------------------|------|--------|----------|--------|-----------|
| Electrical power | 0.15 | \$/kwh | 7,588.00 | 200.00 | 227,640 |
| | | | | | \$227,640 |

Total Estimated Cost: \$6,587,352

Total Estimated Cost per Metre: \$220

Total Estimated Cost per Week: \$145,922

Total Estimated Cost per Shift: \$6,945

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Rail-Sgl.B-TBM Remv. | Parent Estimate ID: | 7803 |
| Tunnel Name: | Rail-Single BoreT | Project Phase: | Conceptual |
| Construction Activity: | TBM Removal | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 06, 2017 |
| Estimate Definition ID: | 7813 | Tunnel Characteristics ID: | 3395 |

Tunnel Characteristics

Finished Diameter: 7.6 m

Activity Details

Shift Arrangement 3 - 8 hour shifts x 7 days per week

Duration of Activity 3 Weeks

Total Number of Shifts 63

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|-----------|-------|---------------|-------------------|------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 504.00 | 1.00 | 45,128 |
| Tunnel miner | 76.06 | \$/hr | 504.00 | 3.00 | 115,003 |
| Shaft bottom | 75.29 | \$/hr | 504.00 | 2.00 | 75,892 |
| Tunnel fitter | 69.92 | \$/hr | 504.00 | 1.00 | 35,240 |
| Tunnel electrician | 84.43 | \$/hr | 504.00 | 1.00 | 42,553 |
| Shaft top | 75.29 | \$/hr | 504.00 | 2.00 | 75,892 |
| Crane operator | 77.86 | \$/hr | 504.00 | 2.00 | 78,483 |
| Surface laborer | 75.22 | \$/hr | 504.00 | 2.00 | 75,822 |
| Equipment laborer | 72.88 | \$/hr | 504.00 | 1.00 | 36,732 |
| | | | | 15.00 | \$580,744 |
| Equipment | | | | | |
| Loco | 8,000.00 | \$/wk | 3.00 | 1.00 | 24,000 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 3.00 | 6.00 | 52,200 |
| Flat cars | 480.00 | \$/wk | 3.00 | 4.00 | 5,760 |
| Person riders | 480.00 | \$/wk | 3.00 | 1.00 | 1,440 |
| Booster fans | 1,500.00 | \$/wk | 3.00 | 1.00 | 4,500 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 3.00 | 1.00 | 3,300 |
| Other plant | 2,200.00 | \$/wk | 3.00 | 1.00 | 6,600 |
| Hoists operator | 3,000.00 | \$/wk | 3.00 | 1.00 | 9,000 |
| Shaft crane | 13,900.00 | \$/wk | 3.00 | 1.00 | 41,700 |
| 50T Crane | 7,000.00 | \$/wk | 3.00 | 1.00 | 21,000 |
| TBM Crane | 20,000.00 | \$/wk | 3.00 | 1.00 | 60,000 |
| Compressors | 1,500.00 | \$/wk | 3.00 | 1.00 | 4,500 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 3.00 | 1.00 | 24,000 |
| Surface fans | 1,200.00 | \$/wk | 3.00 | 1.00 | 3,600 |
| | | | | | \$261,600 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------|-----------|-------|---------------|-------------------|------------------|
| Consumables | | | | | |
| Gas oil | 1.20 | \$/L | 15.00 | 1,000.00 | 18,000 |
| Other consumables | 500.00 | \$/wk | 3.00 | 1.00 | 1,500 |
| | | | | | <u>19,500</u> |
| Materials | | | | | |
| Temporary materials | 800.00 | \$/wk | 3.00 | 1.00 | 2,400 |
| | | | | | <u>2,400</u> |
| General Supplies | | | | | |
| Small tools | 700.00 | \$/wk | 3.00 | 1.00 | 2,100 |
| | | | | | <u>2,100</u> |
| Total Estimated Cost: | | | | | \$866,344 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|-----------------------------|-----------------------------------|-------------------|
| Project: | NFLink | Project Number: | 213789 |
| Estimate Description: | Rail-SgB-Prec.Lining | Parent Estimate ID: | 7805 |
| Tunnel Name: | Rail-Single BoreT | Project Phase: | Conceptual |
| Construction Activity: | Precast Linings | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 06, 2017 |
| Estimate Definition ID: | 7815 | Tunnel Characteristics ID: | 3395 |

Tunnel Characteristics

| | |
|--------------------------------|----------|
| Tunnel Length: | 30,000 m |
| Finished Diameter: | 7.6 m |
| Final Lining Thickness: | 0.35 m |

Assumptions

A) Duration

| | Maximum | Minimum | |
|--------------------------------------|-----------|-----------|--------|
| TBM Fabrication Time | 19 | 15 | Months |
| TBM Erection Time | 3 | 2 | Months |
| Tunneling Time | 50 | 80 | Months |
| Total | 72 | 97 | Months |
| Facility Setup Time | 6 | 5 | Months |
| Learning Curve/Shakedown Time | 1 | 1 | Months |
| Concrete Strength Gain Time | 1 | 1 | Months |
| Available Manufacturing Time | 64 | 90 | Months |

B) Production

| | |
|--|-----------------|
| Allowance for Damage | 2 % |
| Ring Length | 1.5 m |
| Number of Rings Required | 20,400 |
| Production Rate Required | 56.7 Rings/Week |
| Actual Production Rate Achieved | 60 Rings/Week |

Investment on plant, equipment and moulds \$5,000,000

| | |
|-------------------------------|-------------|
| Initial Shakedown Time | 4 Weeks |
| Production Time | 340.0 Weeks |

| | Shakedown Crew | Production / QC Crew |
|------------------------|----------------|----------------------|
| Shifts per Day | 1 | 2 |
| Hours per Shift | 12 | 12 |
| Days per Week | 5 | 5 |

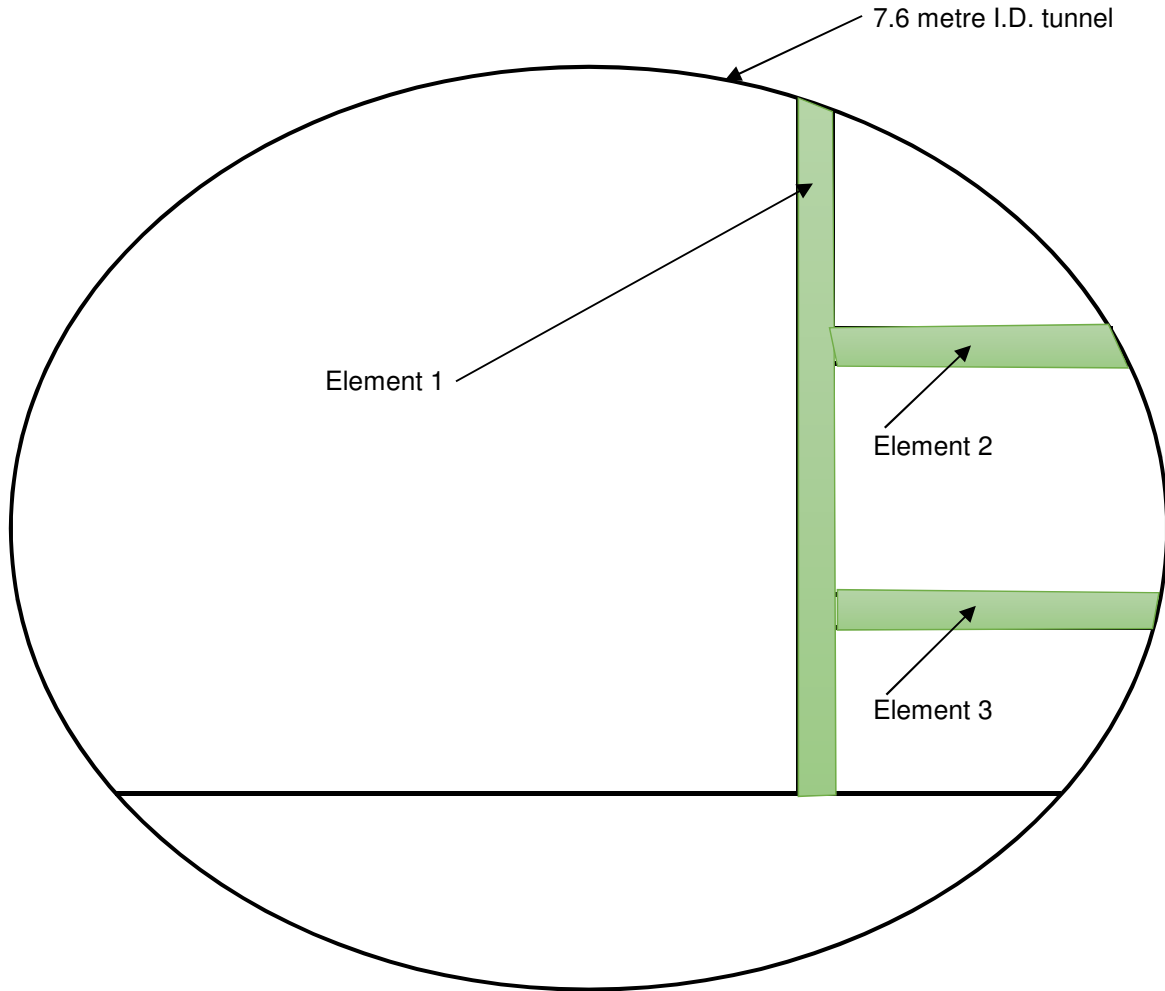
| | |
|----------------------------------|--------------------------------|
| Number of Rings per Truck | 1 |
| Concrete | 13.11 m3/ring |
| Reinforcing Steel | 120 kg/m3 |
| Dunnage Assumption | 50 % of total required storage |

| C) Overheads | \$/Month | Months | Cost | |
|---|-----------|--------|-----------|-------------|
| Project Manager Rate | 7,000 | 93 | 651,000 | |
| Plant Manager Rate | 5,000 | 91 | 455,000 | |
| Quality Manager Rate | 5,000 | 89 | 445,000 | |
| Secretary Rate | 2,000 | 93 | 186,000 | |
| Office Building Cost | | | 150,000 | |
| Office Equipment and Supplies Cost | | | 0 | |
| Finance Assume \$ | 1,000,000 | 93 | 465,000 | |
| Financing @ | 6 % | | | |
| Head Office Support @ | 1 % | | 1,788,921 | |
| Total Overhead Cost | | | | \$4,140,921 |
| Profit Margin | | | 10 % | |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|----------------------|-----------|----------|---------------|-------------------|----------------------|
| Labor | | | | | |
| Shakedown crew | 25.00 | \$/hr | 240.00 | 30.00 | 180,000 |
| Production & QC crew | 32.00 | \$/hr | 40,800.00 | 30.00 | 39,168,000 |
| | | | | 60.00 | \$39,348,000 |
| Consumables | | | | | |
| Power | 0.06 | \$/kwh | 41,040.00 | 1,020.00 | 2,511,648 |
| Heating | 0.11 | \$/m2/wk | 344.00 | 10,600.00 | 401,104 |
| Steam curing | 0.54 | \$/m2/wk | 344.00 | 2,000.00 | 371,520 |
| Fuel | 0.50 | \$/L | 344.00 | 3,000.00 | 516,000 |
| Water | 0.05 | \$/L | 267,489.00 | 100.00 | 1,337,445 |
| | | | | | \$5,137,717 |
| Materials | | | | | |
| Concrete | 180.00 | \$/m3 | 13.11 | 20,400.00 | 48,139,920 |
| Rebar | 1.50 | \$/kg | 1,573.00 | 20,400.00 | 48,133,800 |
| Grout nozzles | 3.00 | \$/Nr | 20,400.00 | 12.00 | 734,400 |
| Lifting socket | 10.00 | \$/Nr | 20,400.00 | 12.00 | 2,448,000 |
| Bolt inserts | 5.00 | \$/Nr | 20,400.00 | 12.00 | 1,224,000 |
| Gaskets | 100.00 | \$/Nr | 20,400.00 | 12.00 | 24,480,000 |
| Dunnage | 2.50 | \$/m | 18.00 | 10,200.00 | 459,000 |
| Site preparation | 10.00 | \$/m2 | 12.45 | 3,400.00 | 423,300 |
| | | | | | \$126,042,420 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|------------------------------------|------------|-------|---------------|-------------------|----------------------|
| Subcontracts | | | | | |
| Delivery | 80.00 | \$/hr | 2.00 | 20,400.00 | 3,264,000 |
| Testing | 100,000.00 | \$/Nr | 1.00 | 1.00 | 100,000 |
| | | | | | \$3,364,000 |
| Subtotal: | | | | | \$173,892,137 |
| Investment on Plant: | | | | | \$5,000,000 |
| Total Overhead Cost: | | | | | \$4,140,921 |
| Subtotal: | | | | | \$183,033,058 |
| Profit: | | | | | \$18,303,306 |
| Total Precast Lining Cost: | | | | | \$201,336,364 |
| Total Cost per Cubic Metre: | | | | | \$753 |
| Total Cost per Ring: | | | | | \$9,869 |

Tunnel length= 30000 m



Assumed tunnel cross section

**Newfoundland Fixed Link Pre-feasibility Study
 Cost Estimating
 Single Bored Railway Tunnel
 Tunnel Structural Finishes**

**Page 2 of 2
 Date: Nov. 07, 2017
 Calculation by:**

Quantity Take-off

| Concrete | | | | | Concrete | Rebar |
|----------|-----|-------|------|------|-----------------|---------------|
| Element | Nr. | L(m) | b(m) | d(m) | Qty(m3) | Qty(m3) |
| 1 | 1 | 30000 | 0.3 | 5.9 | 53100 | 6372.0 |
| 2 | 1 | 30000 | 1.2 | 0.3 | 10800 | 1296.0 |
| 3 | 1 | 30000 | 1.2 | 0.3 | 10800 | 1296.0 |
| | | | | | <u>74700</u> m3 | <u>7668</u> t |

Formwork/falsework

| Element | Nr. | L(m) | d(m) | Faces | Area(m2) |
|---------|-----|-------|------|-------|------------------|
| 1 | 1 | 30000 | 5.9 | 2 | 354000 |
| 2 | 1 | 30000 | 1.2 | 1 | 36000 |
| 3 | 1 | 30000 | 1.2 | 1 | 36000 |
| | | | | | <u>426000</u> m2 |

Rates

| | | |
|---------------|----|------|
| Concrete | m3 | 220 |
| Formwork | m2 | 180 |
| Reinforcement | t | 1600 |

Costs

| | | | | | | |
|---------------|----|-----------|----|------|-----------|--------------------|
| Concrete | m3 | 74700 m3 | at | 220 | = | 16,434,000 |
| Formwork | m2 | 426000 m2 | at | 180 | = | 76,680,000 |
| Reinforcement | t | 7668 t | at | 1600 | = | 12,268,800 |
| | | | | | \$ | <u>105,382,800</u> |

Initialisation

Project: Newfoundland Fixed Link Pre-feasibility Study

Section: North Approach

Option: Bored Rail Tunnel

Date: 7-Nov-17

Calculations by:

Surface gradient %

+ sloping same way as track/road
- sloping against track/road

Track/Road Gradient %

Ground elevation
at portal m

Bottom of slab
elevation at portal m

Total length= 748.7091 m

Total Cost=\$ 9.1 M

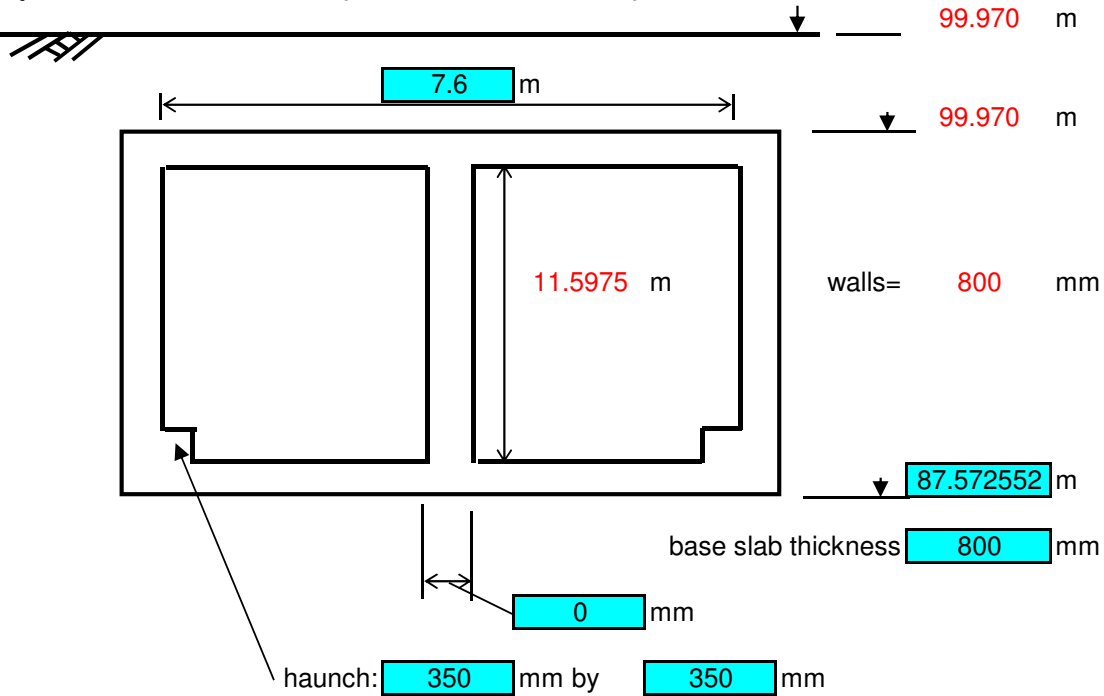
UNIT RATES

Materials

| Item | unit | Rate |
|-----------------------|--------|------|
| excavation | m3 | 60 |
| concrete | m3 | 220 |
| rebar | tonnes | 1600 |
| formwork/falsework | m2 | 241 |
| SP&L<=4.6m deep | m2 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0 |
| 20<SP&L<=25m deep | m2 | 0 |
| backfill + compact | m3 | 69 |
| surface reinstatement | m2 | 52 |

Section Cut and Cover
 Length of section: 74.87091 m Section 1

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 8539.6 | m3 | |
| concrete= | 1958.698 | m3 | |
| rebar= | 235.0 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1736.631 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 1856.4 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 688.8124 | m2 | |

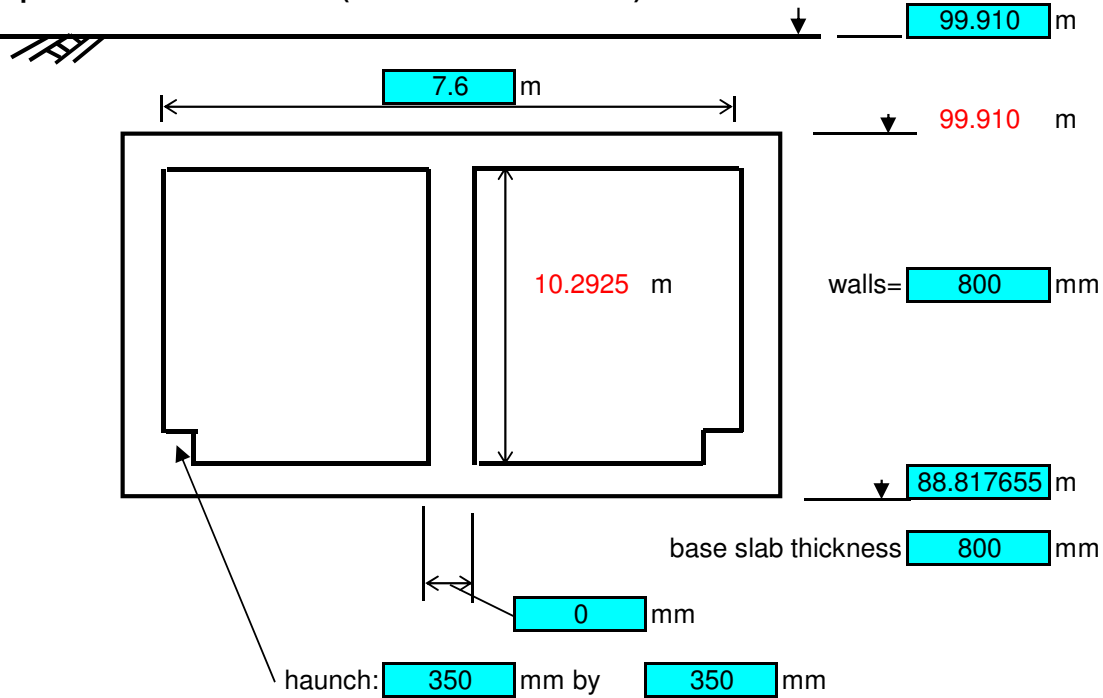
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 8539.6 | 60 | 512373.1 |
| concrete | m3 | 1958.698 | 220.0 | 430913.5 |
| rebar | tonnes | 235.0 | 1600 | 376070 |
| formwork/falsework | m2 | 1736.631 | 241 | 418528 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 1856.4 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 688.8124 | 52 | 35818.24 |

Total 1773703

Section Cut and Cover
 Length of section: 74.87091 m Section 2

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 7640.7 | m3 | |
| concrete= | 1802.367 | m3 | |
| rebar= | 216.3 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1541.218 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 1661.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 688.8124 | m2 | |

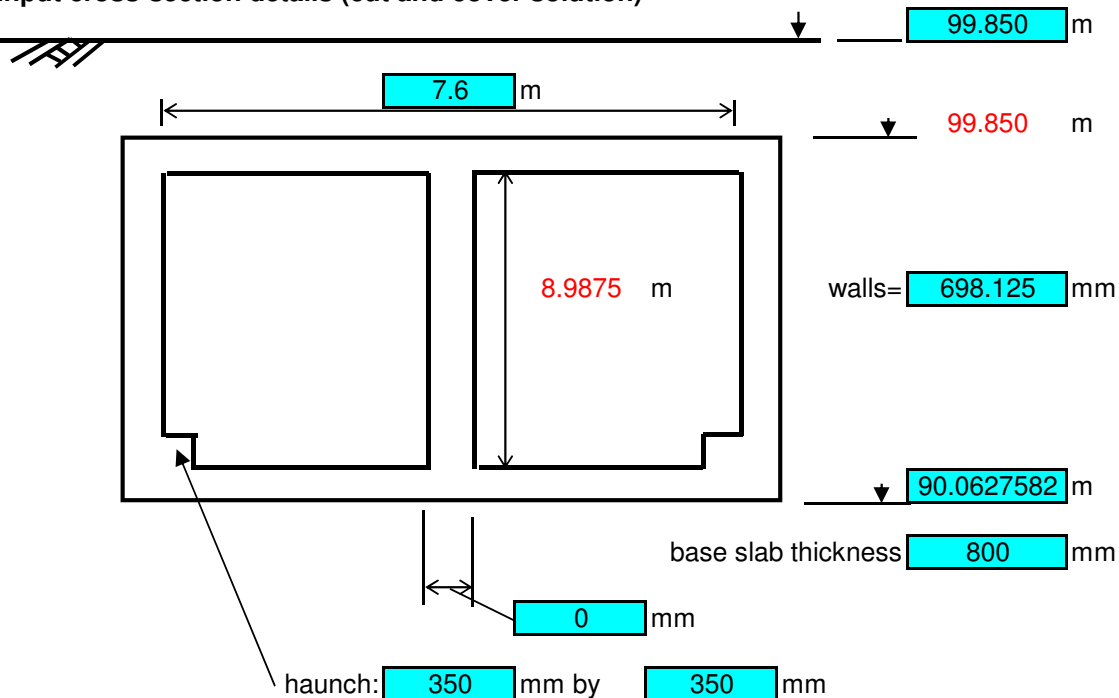
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 7640.7 | 60 | 458439.1 |
| concrete | m3 | 1802.367 | 220.0 | 396520.8 |
| rebar | tonnes | 216.3 | 1600 | 346054.6 |
| formwork/falsework | m2 | 1541.218 | 241 | 371433.5 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 1661.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 688.8124 | 52 | 35818.24 |

Total 1608266

Section Cut and Cover
 Length of section: 74.87091 m Section 3

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 6592.4 | m3 | |
| concrete= | 1496.729 | m3 | |
| rebar= | 179.6 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1345.805 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 1465.6 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 673.5574 | m2 | |

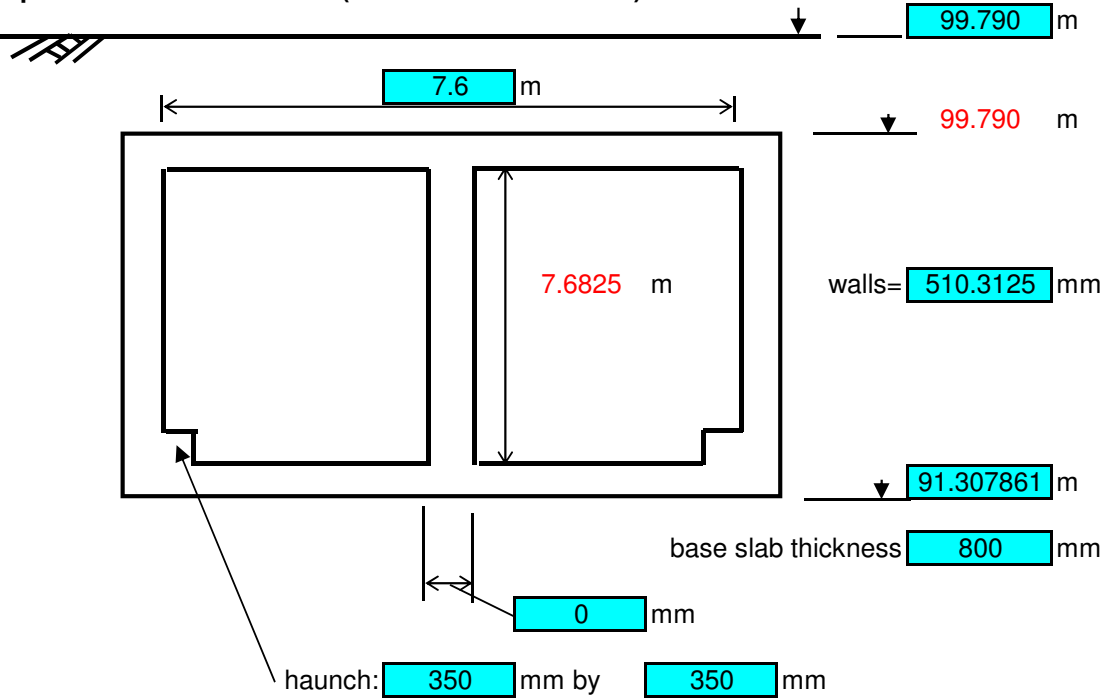
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 6592.4 | 60 | 395546.6 |
| concrete | m3 | 1496.729 | 220.0 | 329280.4 |
| rebar | tonnes | 179.6 | 1600 | 287372 |
| formwork/falsework | m2 | 1345.805 | 241 | 324338.9 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 1465.6 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 673.5574 | 52 | 35024.99 |

Total 1371563

Section Cut and Cover
 Length of section: 74.87091 m Section 4

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 5474.9 | m3 | |
| concrete= | 1121.75 | m3 | |
| rebar= | 134.6 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1150.392 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 1270.2 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 645.4341 | m2 | |

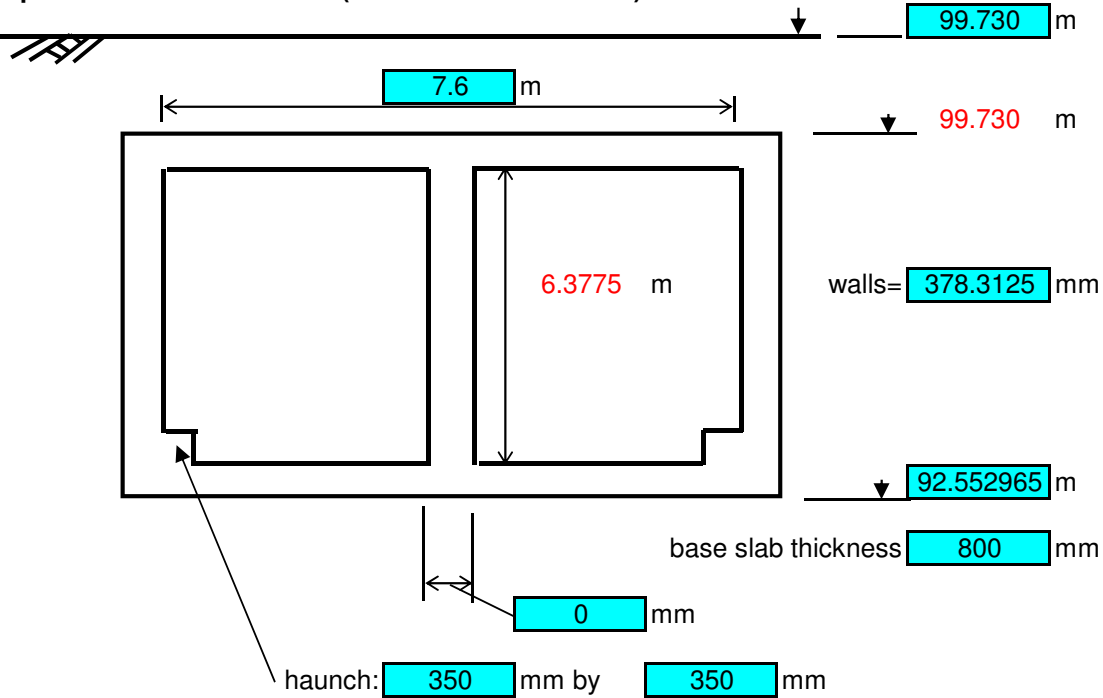
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 5474.9 | 60 | 328493.7 |
| concrete | m3 | 1121.75 | 220.0 | 246785 |
| rebar | tonnes | 134.6 | 1600 | 215376 |
| formwork/falsework | m2 | 1150.392 | 241 | 277244.4 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 1270.2 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 645.4341 | 52 | 33562.57 |

Total 1101462

Section Cut and Cover
 Length of section: 74.87091 m Section 5

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 4490.7 | m3 | |
| concrete= | 880.1582 | m3 | |
| rebar= | 105.6 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 954.9785 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 1074.8 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 625.6681 | m2 | |

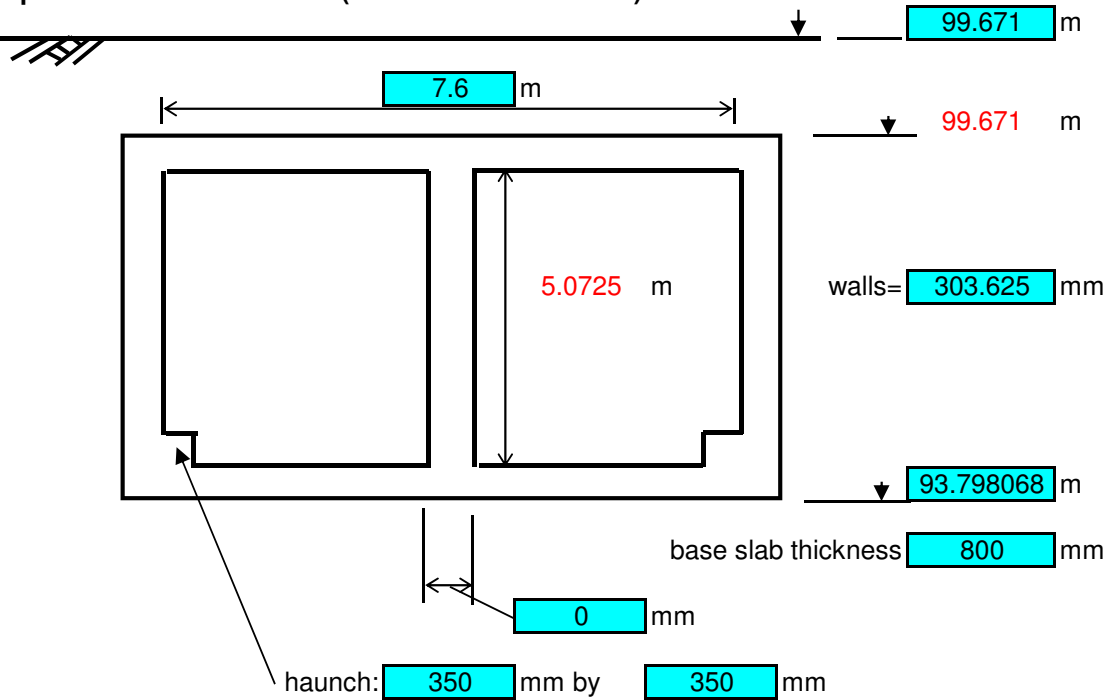
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 4490.7 | 60 | 269444 |
| concrete | m3 | 880.1582 | 220.0 | 193634.8 |
| rebar | tonnes | 105.6 | 1600 | 168990.4 |
| formwork/falsework | m2 | 954.9785 | 241 | 230149.8 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 1074.8 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 625.6681 | 52 | 32534.74 |

Total 894753.7

Section Cut and Cover
 Length of section: 74.87091 m Section 6

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 3608.6 | m3 | |
| concrete= | 740.5539 | m3 | |
| rebar= | 88.9 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 759.5654 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 879.4 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 614.4843 | m2 | |

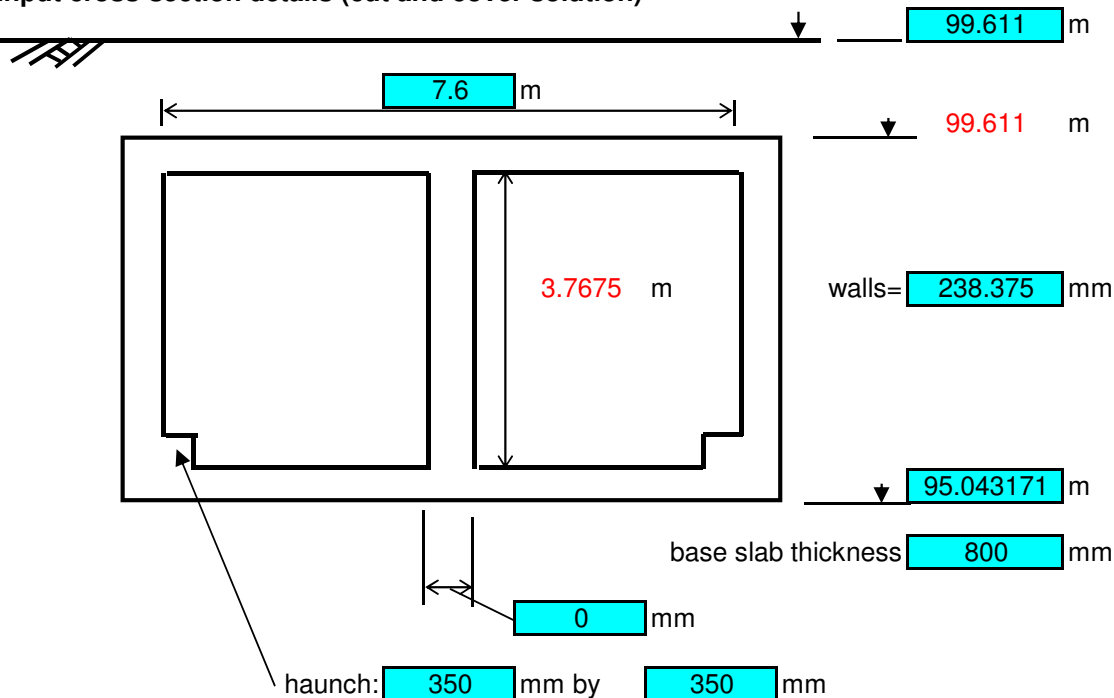
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 3608.6 | 60 | 216513.5 |
| concrete | m3 | 740.5539 | 220.0 | 162921.8 |
| rebar | tonnes | 88.9 | 1600 | 142186.3 |
| formwork/falsework | m2 | 759.5654 | 241 | 183055.3 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 879.4 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 614.4843 | 52 | 31953.18 |

Total 736630.2

Section Cut and Cover
 Length of section: 74.87091 m Section 7

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 2762.0 | m3 | |
| concrete= | 636.5941 | m3 | |
| rebar= | 76.4 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 564.1523 | m2 | |
| SP&L<=4.6m deep | 683.9 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 604.7136 | m2 | |

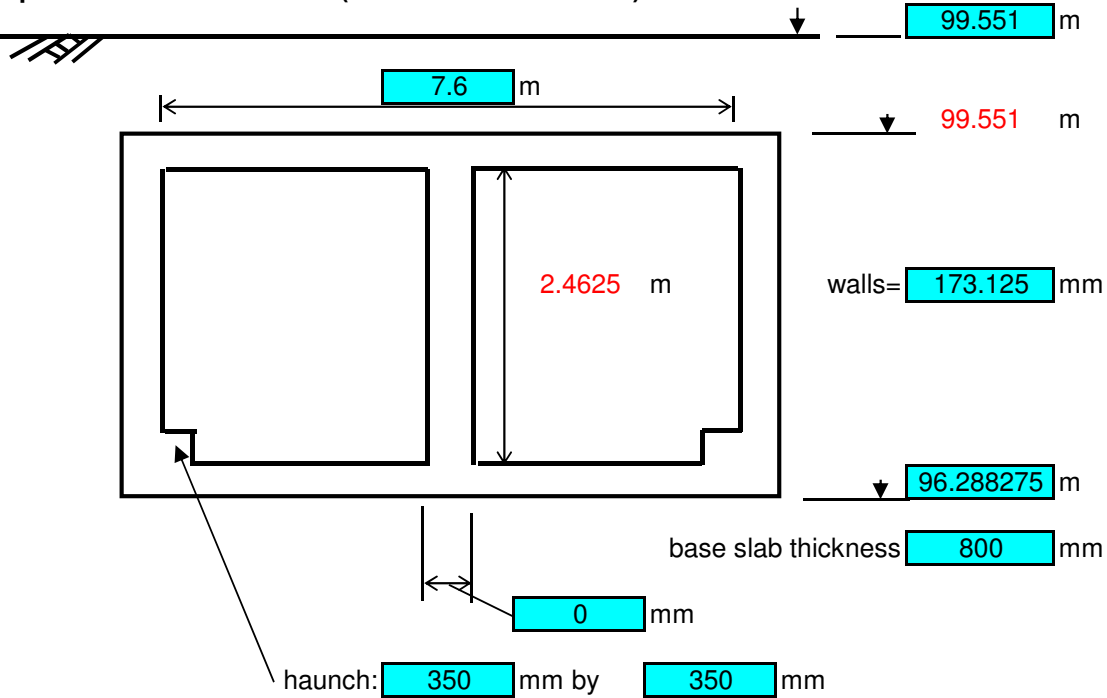
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 2762.0 | 60 | 165721.8 |
| concrete | m3 | 636.5941 | 220.0 | 140050.7 |
| rebar | tonnes | 76.4 | 1600 | 122226.1 |
| formwork/falsework | m2 | 564.1523 | 241 | 135960.7 |
| SP&L<=4.6m deep | m2 | 683.9 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 604.7136 | 52 | 31445.11 |

Total 595404.4

Section Cut and Cover
 Length of section: 74.87091 m Section 8

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 1941.0 | m3 | |
| concrete= | 558.1357 | m3 | |
| rebar= | 67.0 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 368.7392 | m2 | |
| SP&L<=4.6m deep | 488.5 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 594.943 | m2 | |

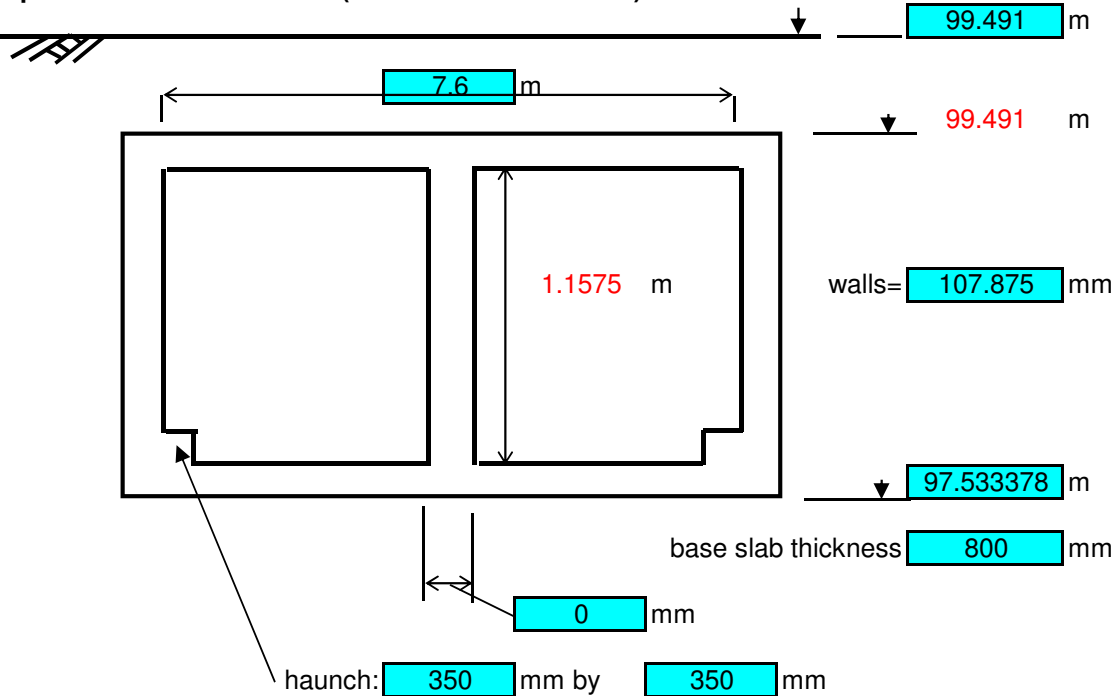
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 1941.0 | 60 | 116460.1 |
| concrete | m3 | 558.1357 | 220.0 | 122789.9 |
| rebar | tonnes | 67.0 | 1600 | 107162.1 |
| formwork/falsework | m2 | 368.7392 | 241 | 88866.16 |
| SP&L<=4.6m deep | m2 | 488.5 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 594.943 | 52 | 30937.04 |

Total 466215.2

Section Cut and Cover
 Length of section: 74.87091 m Section 9

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 1145.5 | m3 | |
| concrete= | 505.1788 | m3 | |
| rebar= | 60.6 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 173.3262 | m2 | |
| SP&L<=4.6m deep | 293.1 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 585.1723 | m2 | |

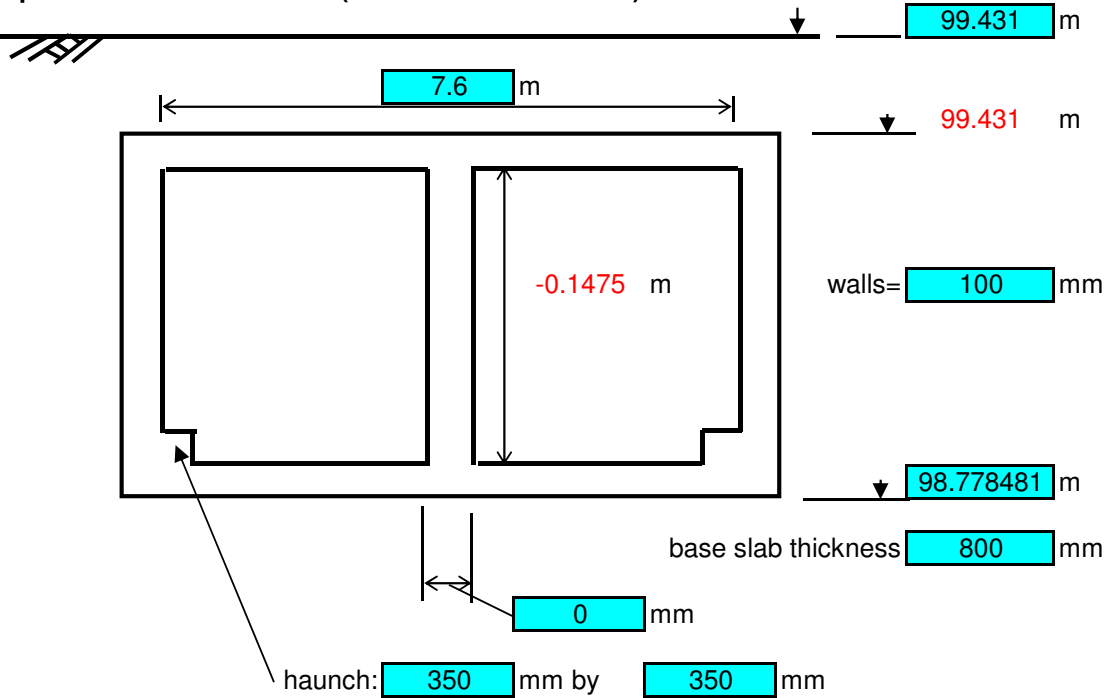
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 1145.5 | 60 | 68728.49 |
| concrete | m3 | 505.1788 | 220.0 | 111139.3 |
| rebar | tonnes | 60.6 | 1600 | 96994.33 |
| formwork/falsework | m2 | 173.3262 | 241 | 41771.6 |
| SP&L<=4.6m deep | m2 | 293.1 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 585.1723 | 52 | 30428.96 |

Total 349062.7

Section Cut and Cover
 Length of section: 74.87091 m Section 10

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 381.1 | m3 | |
| concrete= | 483.3292 | m3 | |
| rebar= | 58.0 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | -22.0869 | m2 | |
| SP&L<=4.6m deep | 97.7 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 583.9931 | m2 | |

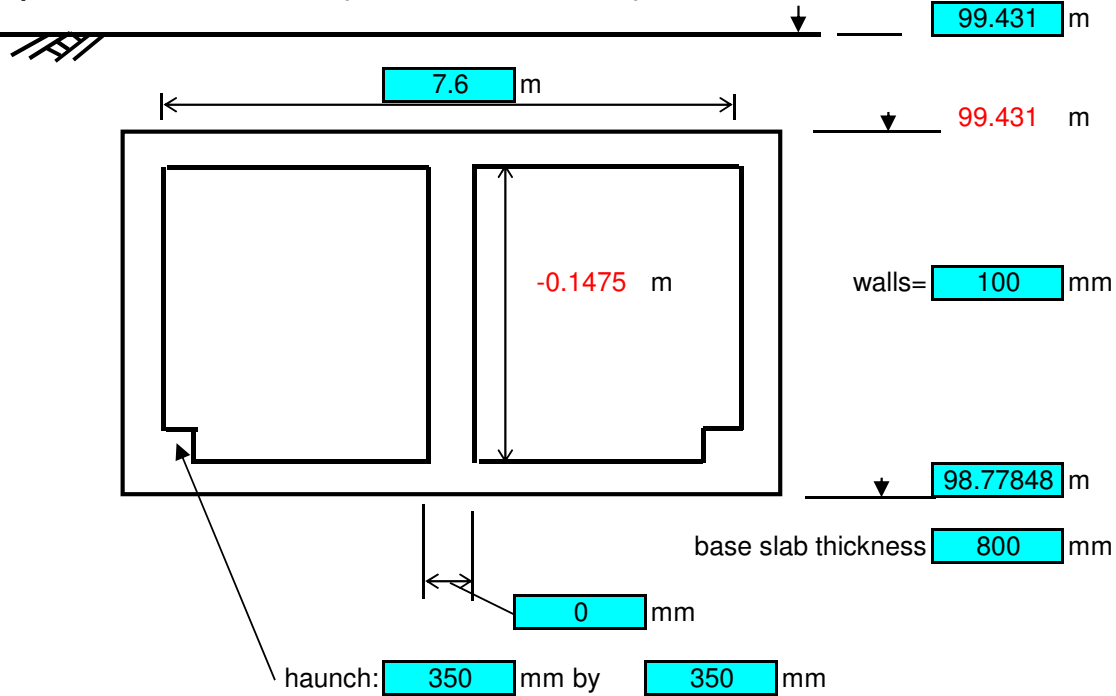
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 381.1 | 60 | 22863.33 |
| concrete | m3 | 483.3292 | 220.0 | 106332.4 |
| rebar | tonnes | 58.0 | 1600 | 92799.2 |
| formwork/falsework | m2 | -22.0869 | 241 | -5322.95 |
| SP&L<=4.6m deep | m2 | 97.7 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 583.9931 | 52 | 30367.64 |

Total 247039.6

Section Cut and Cover
 Length of section: 74.87091 m Section 10

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|----------|--------|-------------------------------|
| excavation= | 381.1 | m3 | |
| concrete= | 483.3292 | m3 | |
| rebar= | 58.0 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | -22.0869 | m2 | |
| SP&L<=4.6m deep | 97.7 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 583.9931 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|------|-----------|
| excavation | m3 | | | 2554583.7 |
| concrete | m3 | | | 2240368.7 |
| rebar | tonnes | | | 1955230.9 |
| formwork/falsework | m2 | | | 2066025.4 |
| SP&L<=4.6m deep | m2 | | | 0 |
| 4.6<SP&L<=6.7m deep | m2 | | | 0 |
| 6.7<SP&L<=10.6m deep | m2 | | | 0 |
| 10.6<SP&L<=13.7m deep | m2 | | | 0 |
| 13.7<SP&L<=16.8m deep | m2 | | | 0 |
| 16.8<SP&L<=20.0m deep | m2 | | | 0 |
| 20<SP&L<=25m deep | m2 | | | 0 |
| backfill | m3 | | | 0 |
| surface reinstatement | m2 | | | 327890.72 |

Total 9144099.4

Indirect costs, profit, and contingency

| | | |
|-----------|------------------------|------------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | <u>9144099</u> |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | <u>9144099</u> |
| 25 | % contingency = \$ | 2286024.9 |
| | Total Cost = \$ | <u>11430124</u> |

Summary of Costs

Markup for adjacent 0 %

| Section | Cost |
|------------------|---------------------|
| 1 | \$ 1,773,703 |
| 2 | \$ 1,608,266 |
| 3 | \$ 1,371,563 |
| 4 | \$ 1,101,462 |
| 5 | \$ 894,754 |
| 6 | \$ 736,630 |
| 7 | \$ 595,404 |
| 8 | \$ 466,215 |
| 9 | \$ 349,063 |
| 10 | \$ 247,040 |
| Sub-total | \$ 9,144,099 |

Initialisation

Project: Newfoundland Fixed Link Pre-feasibility Study

Section: South Approach

Option: Bored Rail Tunnel

Date: 7-Nov-17

Calculations by:

Surface gradient %

+ sloping same way as track/road
- sloping against track/road

Track/Road Gradient %

Ground elevation
at portal m

Bottom of slab
elevation at portal m

Total length= 784.7264 m

Total Cost=\$ 9.6 M

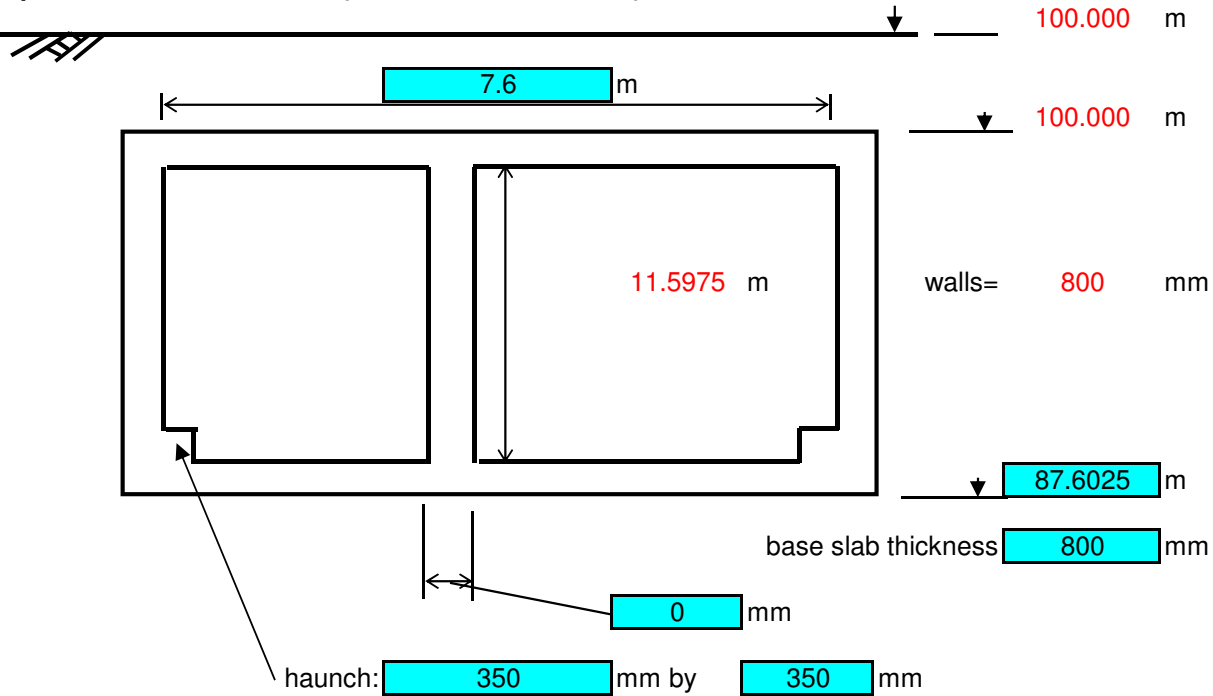
UNIT RATES

Materials

| Item | unit | Rate |
|-----------------------|-------------|-------------|
| excavation | m3 | 60 |
| concrete | m3 | 220 |
| rebar | tonnes | 1600 |
| formwork/falsework | m2 | 241 |
| SP&L<=4.6m deep | m2 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0 |
| 20<SP&L<=25m deep | m2 | 0 |
| backfill + compact | m3 | 69 |
| surface reinstatement | m2 | 52 |

Section Cut and Cover
 Length of section: 78.47264 m Section 1

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 8950.4 | m3 | |
| concrete= | 2052.92273 | m3 | |
| rebar= | 246.4 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1820.17288 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 1945.7 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 721.9482862 | m2 | |

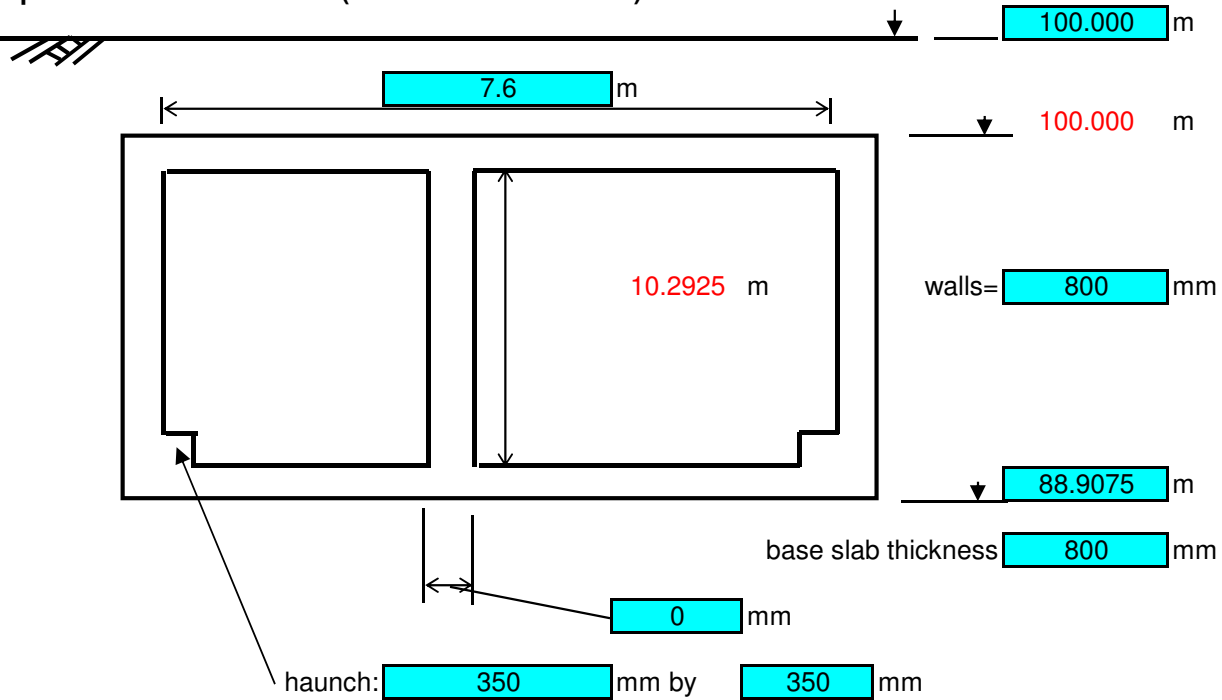
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 8950.4 | 60 | 537021.2 |
| concrete | m3 | 2052.923 | 220.0 | 451643 |
| rebar | tonnes | 246.4 | 1600 | 394161.2 |
| formwork/falsework | m2 | 1820.173 | 241 | 438661.7 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 1945.7 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 721.9483 | 52 | 37541.31 |

Total 1859028

Section Cut and Cover
 Length of section: 78.47264 m Section 2

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 8008.2 | m3 | |
| concrete= | 1889.071858 | m3 | |
| rebar= | 226.7 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1615.35929 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 1740.9 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 721.9482862 | m2 | |

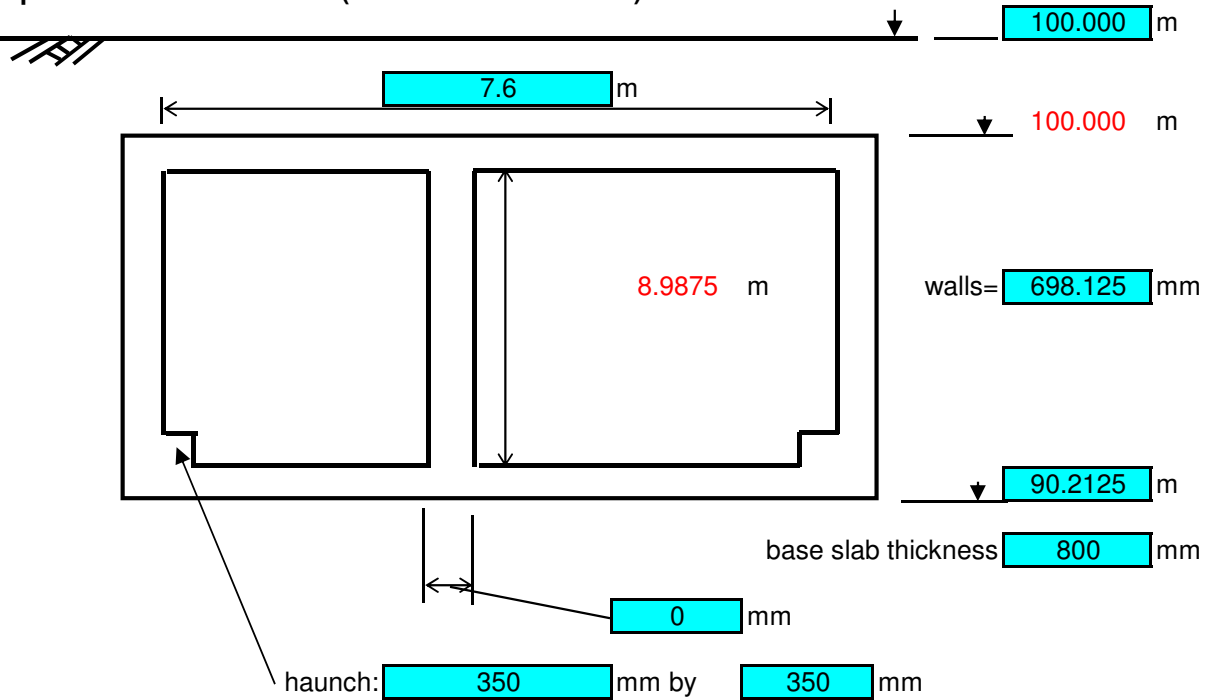
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 8008.2 | 60 | 480492.7 |
| concrete | m3 | 1889.072 | 220.0 | 415595.8 |
| rebar | tonnes | 226.7 | 1600 | 362701.8 |
| formwork/falsework | m2 | 1615.359 | 241 | 389301.6 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 1740.9 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 721.9483 | 52 | 37541.31 |

Total 1685633

Section Cut and Cover
 Length of section: 78.47264 m Section 3

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 6909.6 | m3 | |
| concrete= | 1568.730603 | m3 | |
| rebar= | 188.2 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1410.545701 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 1536.1 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 705.9594859 | m2 | |

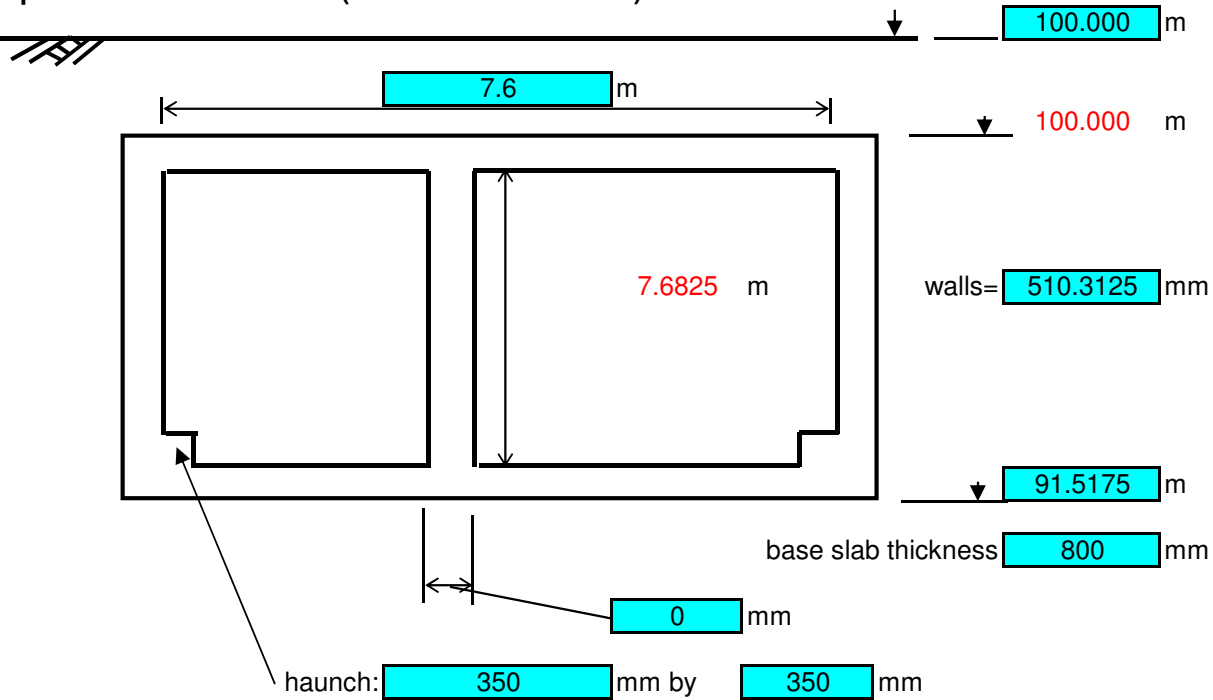
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 6909.6 | 60 | 414574.7 |
| concrete | m3 | 1568.731 | 220.0 | 345120.7 |
| rebar | tonnes | 188.2 | 1600 | 301196.3 |
| formwork/falsework | m2 | 1410.546 | 241 | 339941.5 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 1536.1 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 705.9595 | 52 | 36709.89 |

Total 1437543

Section Cut and Cover
 Length of section: 78.47264 m Section 4

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 5738.3 | m3 | |
| concrete= | 1175.712525 | m3 | |
| rebar= | 141.1 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1205.732111 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 1331.3 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 676.4832005 | m2 | |

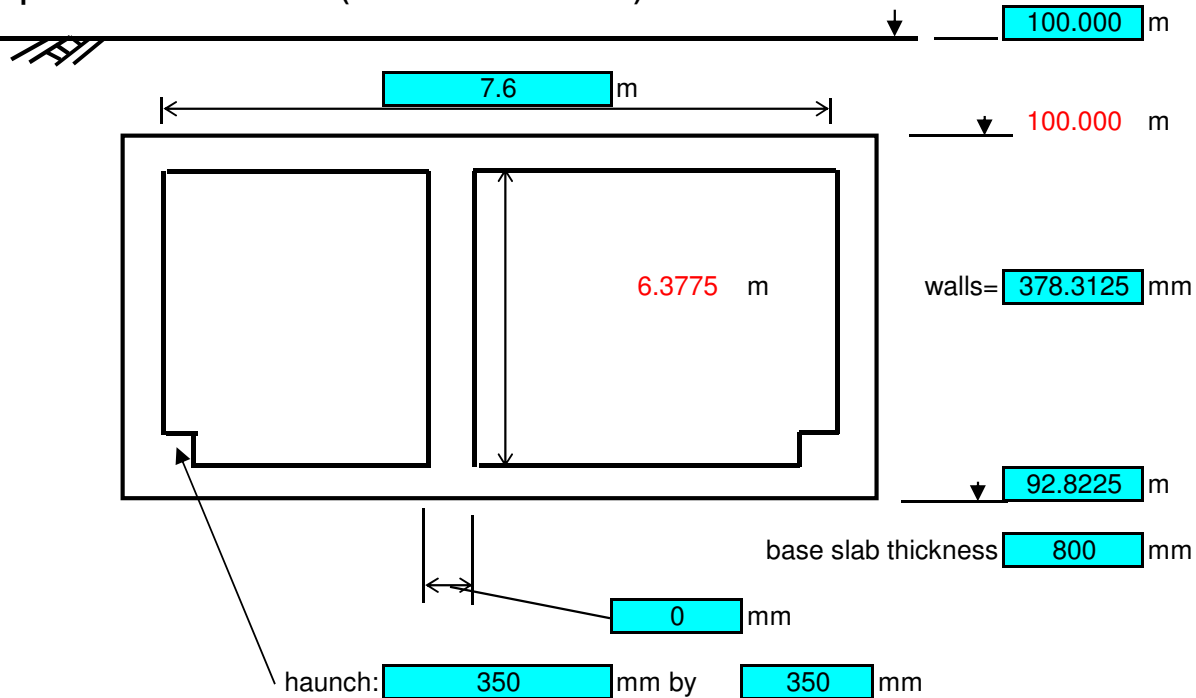
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 5738.3 | 60 | 344296.1 |
| concrete | m3 | 1175.713 | 220.0 | 258656.8 |
| rebar | tonnes | 141.1 | 1600 | 225736.8 |
| formwork/falsework | m2 | 1205.732 | 241 | 290581.4 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 1331.3 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 676.4832 | 52 | 35177.13 |

Total 1154448

Section Cut and Cover
 Length of section: 78.47264 m Section 5

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 4706.8 | m3 | |
| concrete= | 922.4989235 | m3 | |
| rebar= | 110.7 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 1000.918521 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 1126.5 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 655.7664236 | m2 | |

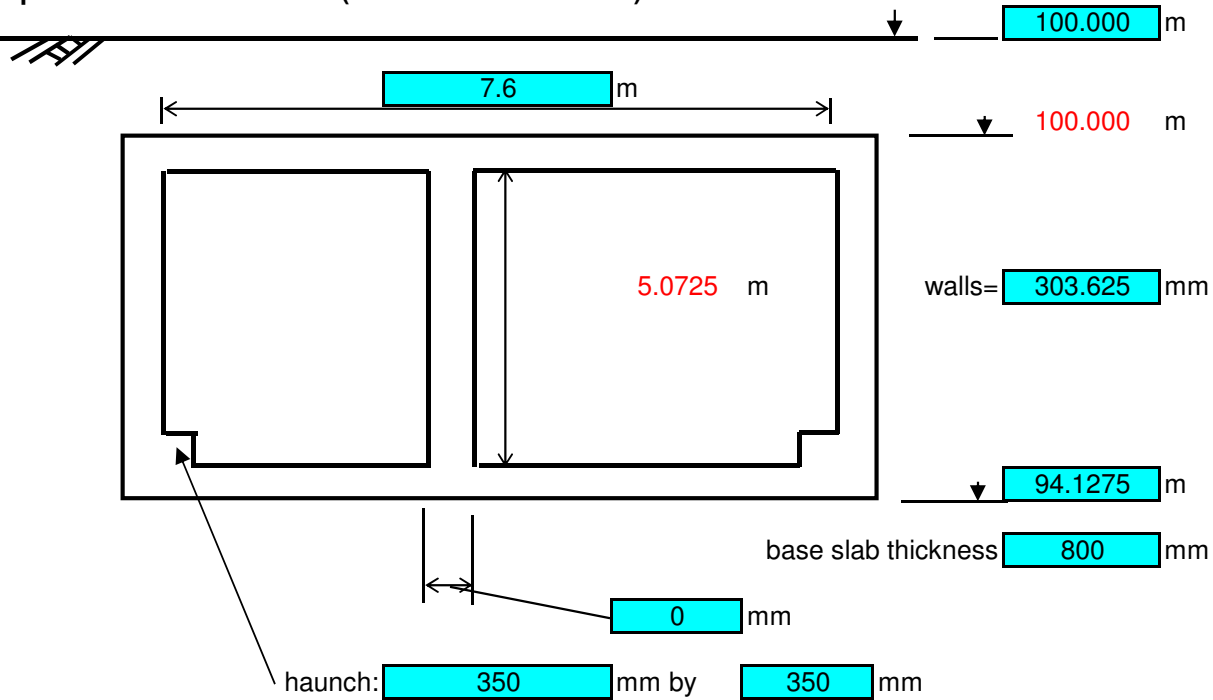
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 4706.8 | 60 | 282405.8 |
| concrete | m3 | 922.4989 | 220.0 | 202949.8 |
| rebar | tonnes | 110.7 | 1600 | 177119.8 |
| formwork/falsework | m2 | 1000.919 | 241 | 241221.4 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 1126.5 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 655.7664 | 52 | 34099.85 |

Total 937796.6

Section Cut and Cover
 Length of section: 78.47264 m Section 6

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 3782.2 | m3 | |
| concrete= | 776.1788148 | m3 | |
| rebar= | 93.1 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 796.1049308 | m2 | |
| SP&L<=4.6m deep | 0.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 921.7 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 644.0445731 | m2 | |

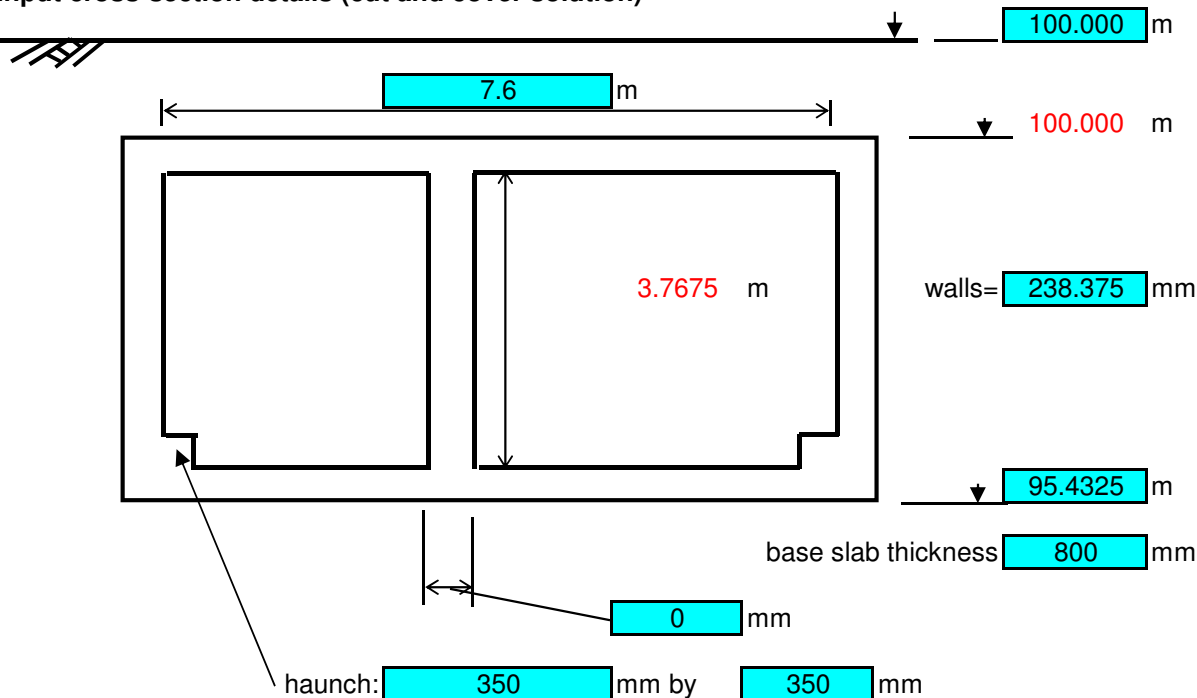
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 3782.2 | 60 | 226929.1 |
| concrete | m3 | 776.1788 | 220.0 | 170759.3 |
| rebar | tonnes | 93.1 | 1600 | 149026.3 |
| formwork/falsework | m2 | 796.1049 | 241 | 191861.3 |
| SP&L<=4.6m deep | m2 | 0.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 921.7 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 644.0446 | 52 | 33490.32 |

Total 772066.4

Section Cut and Cover
 Length of section: 78.47264 m Section 7

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 2894.9 | m3 | |
| concrete= | 667.217985 | m3 | |
| rebar= | 80.1 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 591.291341 | m2 | |
| SP&L<=4.6m deep | 716.8 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 633.8038936 | m2 | |

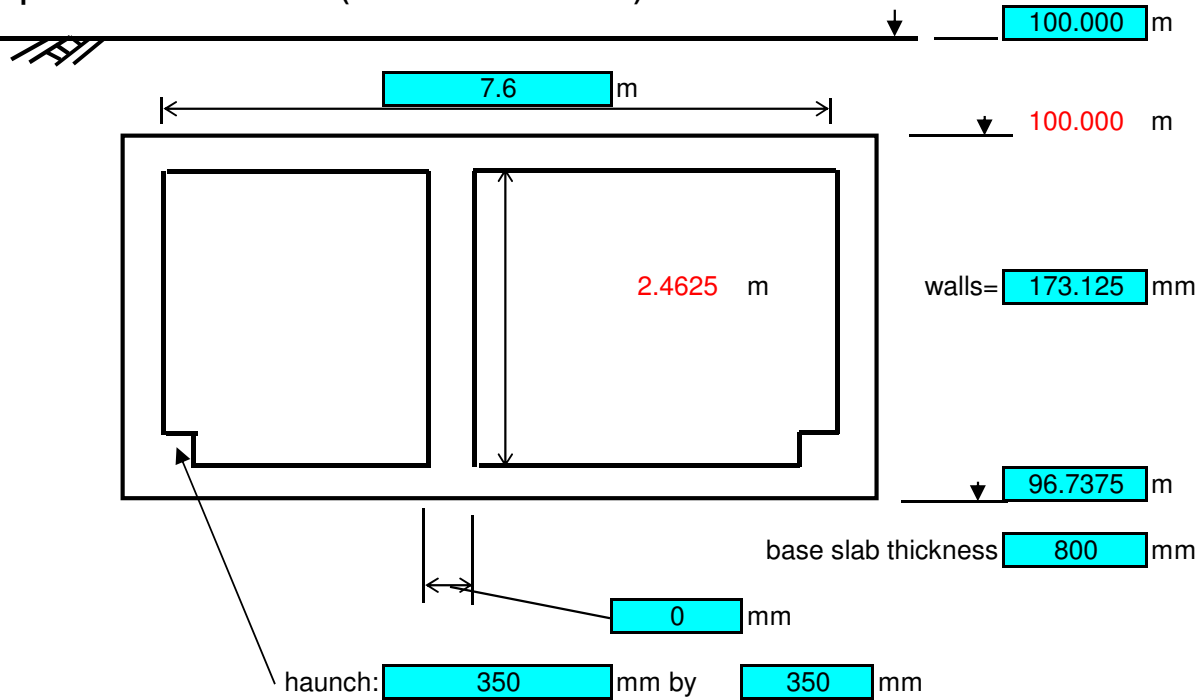
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|-------------|-----------------|-------------|-------------|
| excavation | m3 | 2894.9 | 60 | 173694 |
| concrete | m3 | 667.218 | 220.0 | 146788 |
| rebar | tonnes | 80.1 | 1600 | 128105.9 |
| formwork/falsework | m2 | 591.2913 | 241 | 142501.2 |
| SP&L<=4.6m deep | m2 | 716.8 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 633.8039 | 52 | 32957.8 |

Total 624046.8

Section Cut and Cover
 Length of section: 78.47264 m Section 8

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 2034.4 | m3 | |
| concrete= | 584.9853287 | m3 | |
| rebar= | 70.2 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 386.4777511 | m2 | |
| SP&L<=4.6m deep | 512.0 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 623.5632141 | m2 | |

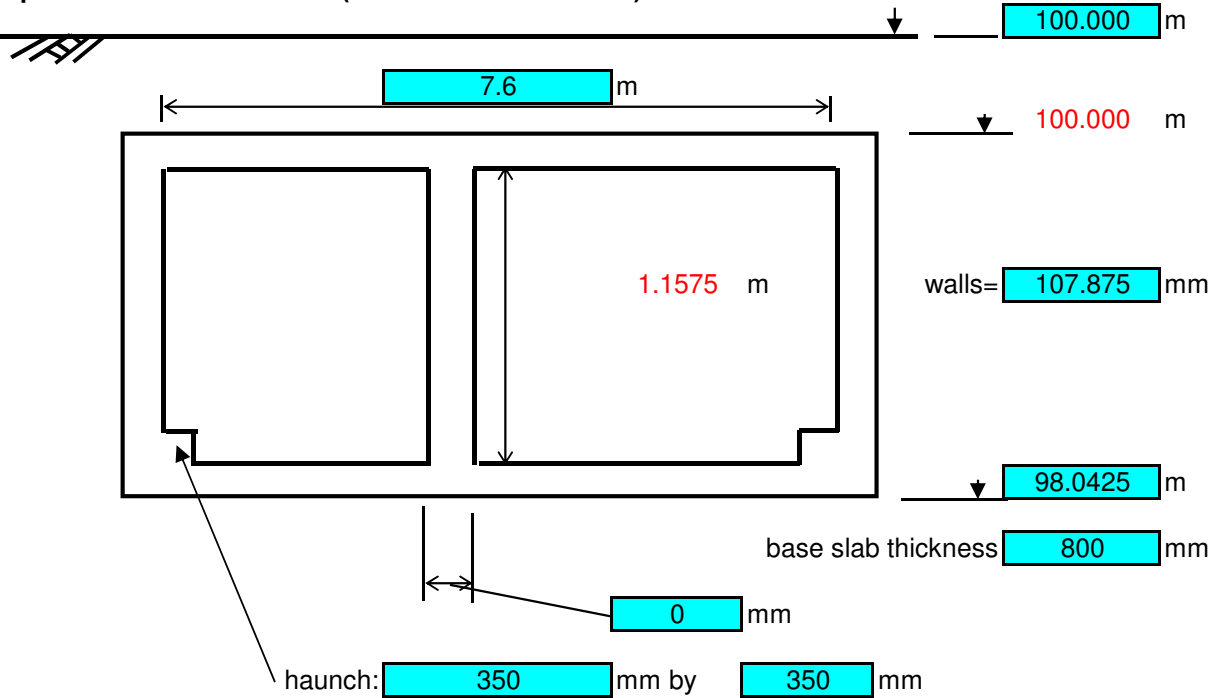
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 2034.4 | 60 | 122062.5 |
| concrete | m3 | 584.9853 | 220.0 | 128696.8 |
| rebar | tonnes | 70.2 | 1600 | 112317.2 |
| formwork/falsework | m2 | 386.4778 | 241 | 93141.14 |
| SP&L<=4.6m deep | m2 | 512.0 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 623.5632 | 52 | 32425.29 |

Total 488642.9

Section Cut and Cover
 Length of section: 78.47264 m Section 9

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|-------------|--------|-------------------------------|
| excavation= | 1200.6 | m3 | |
| concrete= | 529.4808458 | m3 | |
| rebar= | 63.5 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | 181.6641612 | m2 | |
| SP&L<=4.6m deep | 307.2 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 613.3225346 | m2 | |

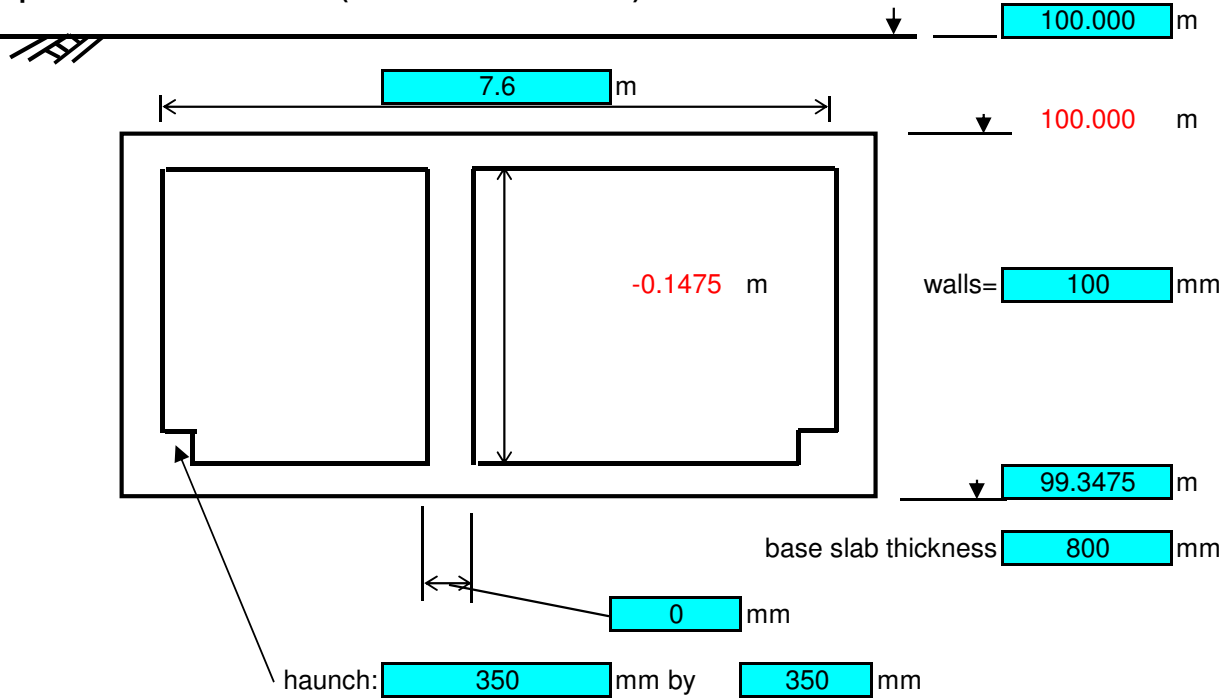
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 1200.6 | 60 | 72034.73 |
| concrete | m3 | 529.4808 | 220.0 | 116485.8 |
| rebar | tonnes | 63.5 | 1600 | 101660.3 |
| formwork/falsework | m2 | 181.6642 | 241 | 43781.06 |
| SP&L<=4.6m deep | m2 | 307.2 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 613.3225 | 52 | 31892.77 |

Total 365854.7

Section Cut and Cover
 Length of section: 78.47264 m Section 10

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|--------------|--------|-------------------------------|
| excavation= | 399.4 | m3 | |
| concrete= | 506.5801263 | m3 | |
| rebar= | 60.8 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | -23.14942874 | m2 | |
| SP&L<=4.6m deep | 102.4 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 612.0865905 | m2 | |

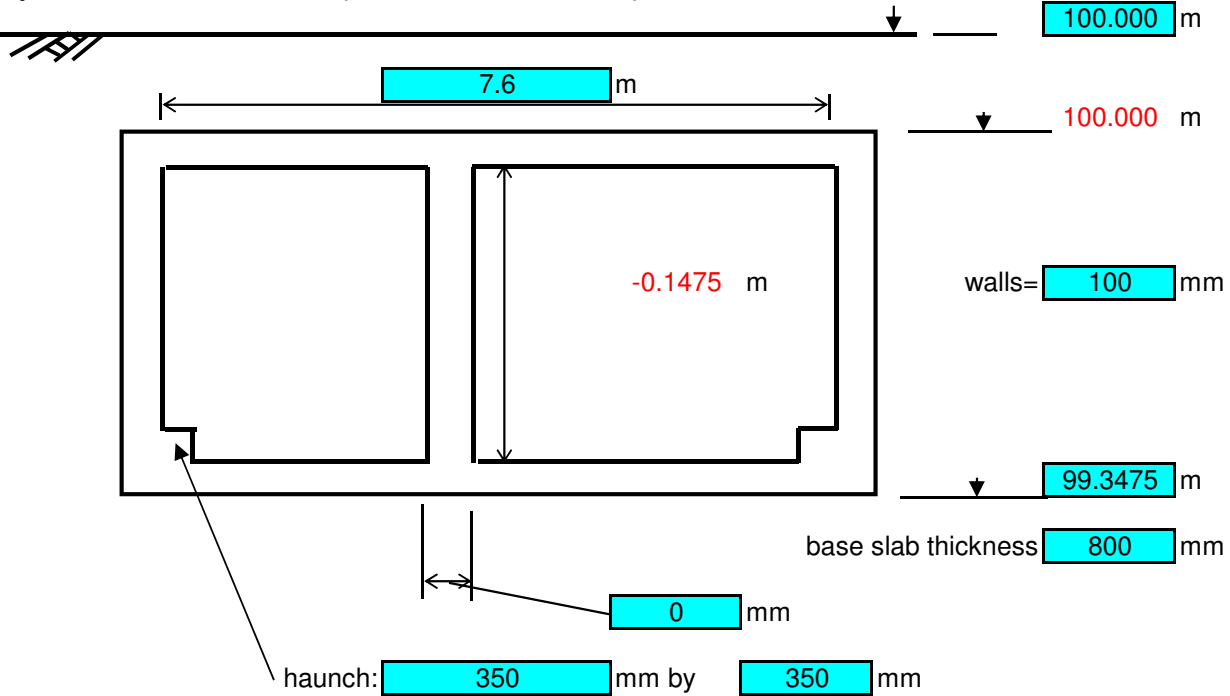
Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|----------|
| excavation | m3 | 399.4 | 60 | 23963.19 |
| concrete | m3 | 506.5801 | 220.0 | 111447.6 |
| rebar | tonnes | 60.8 | 1600 | 97263.38 |
| formwork/falsework | m2 | -23.1494 | 241 | -5579.01 |
| SP&L<=4.6m deep | m2 | 102.4 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 612.0866 | 52 | 31828.5 |

Total 258923.7

Section Cut and Cover
 Length of section: 78.47264 m
 Section 10

Input cross-section details (cut and cover solution)



Calculated Quantities

| | | | |
|------------------------|--------------|--------|-------------------------------|
| excavation= | 399.4 | m3 | |
| concrete= | 506.5801263 | m3 | |
| rebar= | 60.8 | tonnes | (assume 0.12t/m3 of concrete) |
| formwork/falsework= | -23.14942874 | m2 | |
| SP&L<=4.6m deep | 102.4 | m2 | |
| 4.6<SP&L<=6.7m deep | 0.0 | m2 | |
| 6.7<SP&L<=10.6m deep | 0.0 | m2 | |
| 10.6<SP&L<=13.7m deep | 0.0 | m2 | |
| 13.7<SP&L<=16.8m deep | 0.0 | m2 | |
| 16.8<SP&L<=20.0m deep | 0.0 | m2 | |
| 20<SP&L<=25m deep | 0.0 | m2 | |
| backfill= | 0 | m3 | |
| surface reinstatement= | 612.0865905 | m2 | |

Calculated costs

| Item | Unit | Quantity | Rate | Cost |
|-----------------------|--------|----------|-------|-------------|
| excavation | m3 | 399.4 | 60 | 2677474.041 |
| concrete | m3 | 506.5801 | 220.0 | 2348143.543 |
| rebar | tonnes | 60.8 | 1600 | 2049288.91 |
| formwork/falsework | m2 | -23.1494 | 241 | 2165413.259 |
| SP&L<=4.6m deep | m2 | 102.4 | 0 | 0 |
| 4.6<SP&L<=6.7m deep | m2 | 0.0 | 0 | 0 |
| 6.7<SP&L<=10.6m deep | m2 | 0.0 | 0 | 0 |
| 10.6<SP&L<=13.7m deep | m2 | 0.0 | 0 | 0 |
| 13.7<SP&L<=16.8m deep | m2 | 0.0 | 0 | 0 |
| 16.8<SP&L<=20.0m deep | m2 | 0.0 | 0 | 0 |
| 20<SP&L<=25m deep | m2 | 0.0 | 0 | 0 |
| backfill | m3 | 0 | 69 | 0 |
| surface reinstatement | m2 | 612.0866 | 52 | 343664.1774 |

Total 9583983.93

Indirect costs, profit, and contingency

| | | |
|-----------|------------------------|------------------------|
| 0 | % indirect costs = \$ | 0 |
| | Subtotal 2 = \$ | <u>9583984</u> |
| 0 | % profit = \$ | 0 |
| | Subtotal 3 = \$ | <u>9583984</u> |
| 25 | % contingency = \$ | 2395995.983 |
| | Total Cost = \$ | <u>11979980</u> |

Summary of Costs

Markup for adjacent 0 %

| Section | | Cost |
|------------------|-----------|------------------|
| 1 | \$ | 1,859,028 |
| 2 | \$ | 1,685,633 |
| 3 | \$ | 1,437,543 |
| 4 | \$ | 1,154,448 |
| 5 | \$ | 937,797 |
| 6 | \$ | 772,066 |
| 7 | \$ | 624,047 |
| 8 | \$ | 488,643 |
| 9 | \$ | 365,855 |
| 10 | \$ | 258,924 |
| Sub-total | \$ | 9,583,984 |

Sump Sizing

Assume inflow to tunnel of **1** litres/m²/24hours

Tunnel length= **30000** m
Tunnel diameter= **7.6** m

24 hour inflow= **1360938** litres = **1361** m³
assume same again for firefighting= **1361** m³

Required sump capacity= **2722** m³

Assumed sump diameter= **6** m
Assumed number of sump structures= **2** m

Required length of each sump= **49** m

Piping

Assume **300** mm diameter steel pipe connecting each sump to the portal areas
Number of pipes= **1**
Assume sumps located at 1/3 and 2/3 of tunnel length

Total length of piping= **20300** m

Rates

Sump construction-\$ **17795** /m of sump
Pipe-\$ **400** /m of pipe
Pipe installation labour -\$ **36** /m of pipe (assume 6 person crew installing 100m/day)
Pipe installation equipment-\$ **10** /m of pipe (assume \$1000/day for equipment)
Pumps-\$ **125000** /pump

Costs

| Item | Unit | Qty | Rate | \$-Cost |
|-------------------|------|-------|--------------|-------------------|
| Sump construction | m | 98 | 17795 | 1,743,910 |
| Pipe | m | 20300 | 400 | 8,120,000 |
| Pipe installation | m | 20300 | 36 | 740,544 |
| Equipment | m | 20300 | 10 | 203,000 |
| Pumps | Nr | 4 | 125000 | 500,000 |
| | | | Total | 11,307,454 |

Appendix C2-2
Rail Tunnel - Single-Bore, 2 TBMs
Construction Costs

Newfoundland Fixed Link Pre-feasibility - Single Bored - 2TBM Railway Tunnel - Cost Summary

| BORED TUNNEL CONSTRUCTION COSTS | | |
|---|-------------|------------------------|
| ITEM | UNIT | MAIN TUNNEL |
| MOBILIZATION & DEMOBILIZATION | LS | 8,000,000 |
| TUNNELLING | LS | 607,237,443 |
| TUNNEL FINISHES | LS | 105,382,800 |
| NORTH APPROACH STRUCTURES | LS | 9,144,099 |
| SOUTH APPROACH STRUCTURES | LS | 9,583,984 |
| RAIL TRACK | LS | 13,923,100 |
| TUNNEL DRAINAGE | LS | 11,307,454 |
| UTILITY DIVERSIONS | LS | 1,000,000 |
| MONITORING | LS | 1,000,000 |
| SUBTOTAL CIVIL | | \$766,578,880 |
| CIVIL CONTINGENCIES | | |
| CONTINGENCY | 40% | \$306,631,552 |
| TOTAL CIVIL | | \$1,073,210,432 |
| M&E, ROLLING STOCK, RAIL HARDWARE AND FINISHING WORK | | |
| ROLLING STOCK, TERMINALS, OCS, ETC | LS | \$48,773,000 |
| VENTILATION EQUIPMENT | LS | \$3,000,000 |
| VENTILATION SHAFTS AND BUILDINGS x 2 | LS | \$0 |
| FIRE SUPPRESSION SYSTEM | LS | \$2,000,000 |
| CONTROL CENTRE | LS | \$1,000,000 |
| SIGNALLING | LS | \$1,000,000 |
| LIGHTING | LS | \$1,000,000 |
| CCTV SYSTEM | LS | \$0 |
| GAS DETECTION | LS | \$900,000 |
| SUBSTATION, GENERATORS, UPS | LS | \$2,000,000 |
| SUBTOTAL M&E AND FINISHING | | \$59,673,000 |
| CONTINGENCIES | 20% | \$11,934,600 |
| TOTAL M&E AND FINISHING | | \$71,607,600 |
| TOTAL CIVIL, M&E AND FINISHING | | \$1,144,818,032 |
| ALLOWANCES | | |
| CONTRACTOR OH | 15% | \$171,722,705 |
| CONTRACTOR PROFIT | 15% | \$171,722,705 |
| CONSTRUCTION TOTAL | | \$1,488,263,442 |
| PRE-CONSTRUCTION AND SUPERVISION | | |
| FEASIBILITY STUDY | LS | \$17,000,000 |
| ENVIRONMENTAL ASSESSMENT | LS | \$6,000,000 |
| DESIGN | 5% | \$74,413,172 |
| CONSTRUCTION MANAGEMENT | 10% | \$148,826,344 |
| OWNERS COSTS | 2% | \$29,765,269 |
| PRE-CONSTRUCTION TOTAL | | \$276,004,785 |
| GRAND TOTAL | | \$1,764,268,227 |

Detailed Cost Estimate Report

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| | | | |
|--------------------------------|---|-----------------------------------|-------------------|
| Project: | Newfoundland Fixed Link Pre-feasibility Study | Project Number: | 213789 |
| Estimate Description: | Rail T.2sidesDrv | Parent Estimate ID: | 7810 |
| Tunnel Name: | Rail Tunnel-2Sides | Project Phase: | Conceptual |
| Construction Activity: | TBM Tunneling | Geology Type: | Not Applicable |
| Tunnel Technique: | EPB TBM - Precast segmental | Estimate Date: | November 08, 2017 |
| Estimate Definition ID: | 7816 | Tunnel Characteristics ID: | 3396 |

Tunnel Characteristics

| | |
|-----------------------------------|----------------|
| Tunnel Length: | 15,000 m |
| Finished Diameter: | 7.6 m |
| Initial Support Type: | Not Applicable |
| Initial Support Thickness: | 0 m |
| Final Lining Thickness: | 0.35 m |
| Grout Thickness: | 0.1 m |

Theoretical Excavation Volumes

| | |
|----------------------------------|----------------------|
| Total Neat Excavation: | 851,175 Cubic Metres |
| Initial Lining Volume: | 0 Cubic Metres |
| Final Lining Volume: | 131,122 Cubic Metres |
| Theoretical Grout Volume: | 39,584 Cubic Metres |

Normal Excavation/Support Cycle

| | |
|---------------------------------|------------|
| Excavation Cycle Length: | 1.5 Metres |
| Excavate: | 24 Minutes |
| Erect Support: | 27 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 51 Minutes |

Difficult Excavation/Support Cycle

| | |
|--|-------------|
| Length of Difficult Excavation: | 1400 Metres |
| Excavate: | 73 Minutes |
| Erect Support: | 54 Minutes |
| Extend Services: | 0 Minutes |
| Total Cycle Time: | 127 Minutes |

Reduction Factors

| | |
|--------------------------------------|---------|
| Machine availability: | 80 % |
| Back up efficiency: | 55 % |
| Planned maintenance: | 5 % |
| Learning curve efficiency: | 40 % |
| Learning curve duration time: | 8 Weeks |

| | |
|----------------------------------|------------|
| Learning Curve Rate: | 7.1 m/day |
| Experienced Advance Rate: | 17.7 m/day |
| Difficult Advance Rate: | 7.1 m/day |

TBM Skidding Through Excavation

| | |
|------------------------------|----------|
| Duration of skidding: | 0 Weeks |
| Length of skidding: | 0 Metres |

Advance Rate and Shift Details

| | Metres | Days |
|---|-------------------------------------|------|
| Shift Arrangement: | 3 - 8 hour shifts x 7 days per week | |
| Avg. Drive Advance per Shift: | 5.01 Metres | |
| Avg. Drive Advance per Day: | 15 Metres | |
| Avg. Drive Advance per Week: | 105 Metres | |
| Duration of Tunneling (Incl. Skid): | 142.68 Weeks | |
| Total number of shifts (Incl. Skid): | 2,997 | |
| Learning Curve Drive: | 397 | 56 |
| Experienced Drive: | 13,203 | 746 |
| Difficult Drive: | 1,400 | 197 |
| Skidding Portion: | 0 | 0 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|--------------------------------|--------------|-------|---------------|-------------------|---------------------|
| Labor | | | | | |
| Pit boss | 89.54 | \$/hr | 23,974.00 | 1.00 | 2,146,632 |
| Working foreperson | 89.54 | \$/hr | 23,974.00 | 2.00 | 4,293,264 |
| Tunnel miner | 76.06 | \$/hr | 23,974.00 | 3.00 | 5,470,387 |
| Tunnel laborer | 75.39 | \$/hr | 23,974.00 | 4.00 | 7,229,599 |
| Loco driver | 75.64 | \$/hr | 23,974.00 | 4.00 | 7,253,573 |
| Shaft bottom | 75.29 | \$/hr | 23,974.00 | 1.00 | 1,805,002 |
| TBM operator | 76.06 | \$/hr | 23,974.00 | 1.00 | 1,823,462 |
| Tunnel fitter | 69.92 | \$/hr | 23,974.00 | 1.00 | 1,676,262 |
| Tunnel electrician | 84.43 | \$/hr | 23,974.00 | 1.00 | 2,024,125 |
| Shaft top | 75.29 | \$/hr | 23,974.00 | 2.00 | 3,610,005 |
| Crane operator | 77.86 | \$/hr | 23,974.00 | 1.00 | 1,866,616 |
| Surface laborer | 75.22 | \$/hr | 23,974.00 | 4.00 | 7,213,297 |
| Equipment laborer | 72.88 | \$/hr | 23,974.00 | 4.00 | 6,988,900 |
| | | | | 29.00 | \$53,401,126 |
| Equipment | | | | | |
| TBM | 340,000.00 | \$/m2 | 56.74 | 1.00 | 19,291,600 |
| TBM optional equipment. | 1,500,000.00 | \$/Nr | 1.00 | 1.00 | 1,500,000 |
| Loco | 8,000.00 | \$/wk | 142.68 | 5.00 | 5,707,200 |
| Muck cars & grout cars | 2,900.00 | \$/wk | 142.68 | 56.00 | 23,171,232 |
| Flat cars | 480.00 | \$/wk | 142.68 | 8.00 | 547,891 |
| Person riders | 480.00 | \$/wk | 142.68 | 2.00 | 136,973 |
| Track | 200.00 | \$/m | 15,000.00 | 1.00 | 3,000,000 |
| Air pipe | 46.00 | \$/m | 15,000.00 | 1.00 | 690,000 |
| Water pipe | 40.00 | \$/m | 15,000.00 | 1.00 | 600,000 |
| Pump main | 78.00 | \$/m | 15,000.00 | 1.00 | 1,170,000 |
| Cabling | 125.00 | \$/m | 15,000.00 | 1.00 | 1,875,000 |
| Lighting | 46.00 | \$/m | 15,000.00 | 1.00 | 690,000 |
| Vent ducting | 46.00 | \$/m | 15,000.00 | 1.00 | 690,000 |
| Grout mixers | 10,960.00 | \$/wk | 142.68 | 1.00 | 1,563,773 |
| Grout pumps | 5,250.00 | \$/wk | 142.68 | 1.00 | 749,070 |
| Grout hoses & pipes | 300.00 | \$/wk | 142.68 | 2.00 | 85,608 |
| Transformers & switchgear - LV | 1,100.00 | \$/wk | 142.68 | 2.00 | 313,896 |
| Small tools | 4,000.00 | \$/wk | 142.68 | 1.00 | 570,720 |
| Shaft crane | 13,900.00 | \$/wk | 142.68 | 1.00 | 1,983,252 |
| Compressors | 1,500.00 | \$/wk | 142.68 | 1.00 | 214,020 |
| Low pressure C/A system | 5,900.00 | \$/wk | 142.68 | 1.00 | 841,812 |
| Pipework and controls | 1,000.00 | \$/wk | 142.68 | 2.00 | 285,360 |
| Generators | 3,100.00 | \$/wk | 142.68 | 1.00 | 442,308 |
| Transformers & switchgear - HV | 8,000.00 | \$/wk | 142.68 | 1.00 | 1,141,440 |
| Surface fans | 1,200.00 | \$/wk | 142.68 | 2.00 | 342,432 |
| Loaders | 3,550.00 | \$/wk | 142.68 | 2.00 | 1,013,028 |
| Other surface plant | 4,000.00 | \$/wk | 142.68 | 1.00 | 570,720 |

| Resource Name | Unit Rate | UOM | Unit Quantity | Resource Quantity | Total |
|-----------------------|-----------|--------|---------------|-------------------|---------------|
| Tunnel C/A system | 62,000.00 | \$/wk | 142.68 | 1.00 | 8,846,160 |
| | | | | | \$78,033,495 |
| Consumables | | | | | |
| Electrical power | 0.15 | \$/kwh | 23,974.00 | 3,000.00 | 10,788,300 |
| Gas oil | 1.20 | \$/L | 48,000.00 | 1.00 | 57,600 |
| Lubrication materials | 140.00 | \$/wk | 142.68 | 1.00 | 19,975 |
| TBM spares, cutters | 390.00 | \$/m | 15,000.00 | 1.00 | 5,850,000 |
| Filters etc. | 465.00 | \$/wk | 142.68 | 1.00 | 66,346 |
| Hydraulic oil | 7.00 | \$/L | 32,000.00 | 1.00 | 224,000 |
| Other consumables | 200.00 | \$/wk | 142.68 | 1.00 | 28,536 |
| Tail seal grease | 200.00 | \$/m | 15,000.00 | 1.00 | 3,000,000 |
| | | | | | \$20,034,757 |
| Materials | | | | | |
| Concrete lining rings | 10,066.82 | \$/Nr | 10,000.00 | 1.00 | 100,668,182 |
| Gaskets | 170.00 | \$/m | 15,000.00 | 1.00 | 2,550,000 |
| Bolts | 20.00 | \$/Nr | 1,765.00 | 30.00 | 1,059,000 |
| Grout | 220.00 | \$/m3 | 39,584.00 | 1.00 | 8,708,480 |
| Grout plugs | 0.80 | \$/Nr | 1,765.00 | 7.00 | 9,884 |
| Packers | 15.00 | \$/Nr | 3,633.00 | 12.00 | 653,940 |
| Temporary materials | 3,000.00 | \$/wk | 142.68 | 1.00 | 428,040 |
| Other materials | 0.00 | \$/t | 0.00 | 1.00 | 0 |
| | | | | | \$114,077,526 |
| Subcontracts | | | | | |
| Soil disposal | 25.00 | \$/m3 | 851,175.00 | 1.50 | 31,919,063 |
| | | | | | \$31,919,063 |

Total Estimated Cost: \$297,465,967

Total Estimated Cost per Metre: \$19,831

Total Estimated Cost per Week: \$2,084,902

Total Estimated Cost per Shift: \$99,264

APPENDIX D

Operating and Maintenance Costs

APPENDIX D1

Road Tunnel

Appendix D1-1
Road Tunnel - Single Bore
Operating and Maintenance Costs

Estimate of annual operating costs

A. Management and operation of tunnel control building

1. Manager (Assume 1 individual in full time management position)

Manager 2080 hrs/yr @ 120 \$/hr= \$ 249600 /yr

2. Office assistants (assume 2 full time office assistants)

Office staff 4160 hrs/yr @ 70 \$/hr= \$ 291200 /yr

3. Cleaners (assume 2 full time cleaners 5 days per week)

Cleaners 4160 hrs/yr @ 50 \$/hr= \$ 208000 /yr

Sub-total A - \$ 748800 /yr

B. Traffic Supervision Costs

1. Closed circuit TV system cost= \$ 1,115,000
written-off over 40 years = \$ 27875 /yr

2. Monitoring staff (assume full time monitoring by 2 persons 24 hours per day)

CCTV personnel 17472 hrs/yr @ 60 \$/hr= \$ 1048320 /yr

Sub-total B - \$ 1076195 /yr

C. Emergency Truck Costs

1. Assume 1 truck and each replaced every 15 years

Emergency truck costs= \$ 70000
written-off over 15 years = \$ 4666.7 /yr

2. Truck operators (assume 2 persons for each truck 24 hours per day)

Truck operators 34944 hrs/yr @ 68 \$/hr= \$ 2376192 /yr

3. Truck maintenance and fuel

Allow \$ 20000 /yr

Sub-total C - \$ 2,400,858.7 /yr

Estimate of annual operating costs

D. Energy Costs

2.7 kilometre long twin bore Eisenhower Highway Tunnel on I70 in Colorado has annual electricity bill of \$840,000

Assume energy costs largely governed by tunnel length

For 21 km long tunnel

Lane factor= 0.25

Energy cost - \$ 816666.67

Sub-total D - \$ 816667 /yr

Total Operating Cost - \$ \$ 5,042,520 /yr

Estimate of annual maintenance costs

A. Electrical Maintenance Costs

1. Labour (Assume equivalent of 1 electrician present at all times)

Electrician 1 8736 hrs/yr @ 84 \$/hr= \$ 733824 /yr

2. Materials/replacement of equipment (assume electrical equipment replaced once every 40 years)

Electrical equipment cost= \$ 7035000 (sub-station, generators, etc.)
written-off over 40 years = \$ 175875 /yr

Fire suppression equipment cost= \$ 4000000
written-off over 40 years = \$ 100000 /yr

Signalling equipment cost= \$ 0
written-off over 40 years = \$ 0 /yr

Gas detection equipment cost= \$ 945000
written-off over 40 years = \$ 23625 /yr

3. Platform truck for replacing lights etc.

Platform truck= \$ 60000
written-off over 15 years = \$ 4000.00 /yr

4. Platform truck maintenance and fuel

Allow \$ 5000 /yr

Sub-total A - \$ 1042324 /yr

B. Mechanical Maintenance Costs

1. Labour (Assume equivalent of 1 mechanic/maintenance technician present at all times)

Mechanic 8736 hrs/yr @ 70 \$/hr= \$ 611520 /yr

2. Materials/replacement of equipment (assume mechanical equipment replaced every 40 years)

Ventilation equipment cost= \$ 6000000
written-off over 40 years = \$ 150000 /yr

3. Van for use by the mechanical/electrical maintenance staff (assume 1 no.)

Maintenance van= \$ 25000
written-off over 8 years = \$ 3125 /yr

Estimate of annual maintenance costs

B. Mechanical Maintenance Costs contd.

4. Van maintenance and fuel

Allow \$ 5000 /yr

Sub-total B - \$ 769645 /yr

C. Structure Maintenance Costs

1. Inspection (assume 3 persons inspection crew for 5 weeks once every 4 years)

| | | | | | | |
|-----------------|----|----------|-----|--------|---------|-----|
| Senior engineer | 50 | hrs/yr @ | 150 | \$/hr= | \$ 7500 | /yr |
| Engineer 1 | 50 | hrs/yr @ | 120 | \$/hr= | \$ 6000 | /yr |
| Engineer 2 | 50 | hrs/yr @ | 120 | \$/hr= | \$ 6000 | /yr |

2. Tunnel cleaning and maintenance (assume 4 persons crew on single shift basis throughout year)

Labourers 11648 hrs/yr @ 65 \$/hr= \$ 757120 /yr

3. Street cleaner vehicle

Street cleaner vehicle= \$ 65000
written-off over 15 years = \$ 4333.3 /yr

4. Street cleaner vehicle maintenance and fuel

Allow \$ 5000 /yr

5. Washer truck

Washing truck= \$ 40000
written-off over 15 years = \$ 2666.7 /yr

6. Washer truck maintenance and fuel

Allow \$ 5000 /yr

Sub-total C - \$ 793620 /yr

Total Maintenance Cost - \$ \$ 2,605,589 /yr

Appendix D1-2
Road Tunnel - Twin Bore
Operating and Maintenance Costs

Estimate of annual operating costs

A. Management and operation of tunnel control building

1. Manager (Assume 1 individual in full time management position)

Manager 2080 hrs/yr @ 120 \$/hr= \$ 249600 /yr

2. Office assistants (assume 2 full time office assistants)

Office staff 4160 hrs/yr @ 70 \$/hr= \$ 291200 /yr

3. Cleaners (assume 2 full time cleaners 5 days per week)

Cleaners 4160 hrs/yr @ 50 \$/hr= \$ 208000 /yr

Sub-total A - \$ 748800 /yr

B. Traffic Supervision Costs

1. Closed circuit TV system cost= \$ 2230000
written-off over 40 years = \$ 55750 /yr

2. Monitoring staff (assume full time monitoring by 2 persons 24 hours per day)

CCTV personnel 34944 hrs/yr @ 60 \$/hr= \$ 2096640 /yr

Sub-total B - \$ 2152390 /yr

C. Emergency Truck Costs

1. Assume 1 truck and each replaced every 15 years

Emergency truck costs= \$ 70000
written-off over 15 years = \$ 4666.7 /yr

2. Truck operators (assume 2 persons for each truck 24 hours per day)

Truck operators 34944 hrs/yr @ 68 \$/hr= \$ 2376192 /yr

3. Truck maintenance and fuel

Allow \$ 20000 /yr

Sub-total C - \$ 2,400,858.7 /yr

Estimate of annual operating costs

D. Energy Costs

2.7 kilometre long twin bore Eisenhower Highway Tunnel on I70 in Colorado has annual electricity bill of \$840,000

Assume energy costs largely governed by tunnel length

For 42 km long tunnel Lane factor= 0.25

Energy cost - \$ 1633333.333

Sub-total D - \$ 1633333 /yr

Total Operating Cost - \$ \$ 6,935,382 /yr

Estimate of annual maintenance costs

A. Electrical Maintenance Costs

1. Labour (Assume equivalent of 1 electrician present at all times)

Electrician 1 17472 hrs/yr @ 84 \$/hr= \$ 1467648 /yr

2. Materials/replacement of equipment (assume electrical equipment replaced once every 40 years)

Electrical equipment cost= \$ 14070000 (sub-station, generators, etc.)
written-off over 40 years = \$ 351750 /yr

Fire suppression equipment cost= \$ 8000000
written-off over 40 years = \$ 200000 /yr

Signalling equipment cost= \$ 0
written-off over 40 years = \$ 0 /yr

Gas detection equipment cost= \$ 1890000
written-off over 40 years = \$ 47250 /yr

3. Platform truck for replacing lights etc.

Platform truck= \$ 60000
written-off over 15 years = \$ 4000.00 /yr

4. Platform truck maintenance and fuel

Allow \$ 5000 /yr

Sub-total A - \$ 2075648 /yr

B. Mechanical Maintenance Costs

1. Labour (Assume equivalent of 1 mechanic/maintenance technician present at all times)

Mechanic 17472 hrs/yr @ 70 \$/hr= \$ 1223040 /yr

2. Materials/replacement of equipment (assume mechanical equipment replaced every 40 years)

Ventilation equipment cost= \$ 12000000
written-off over 40 years = \$ 300000 /yr

3. Van for use by the mechanical/electrical maintenance staff (assume 1 no.)

Maintenance van= \$ 25000
written-off over 8 years = \$ 3125 /yr

Estimate of annual maintenance costs

B. Mechanical Maintenance Costs contd.

4. *Van maintenance and fuel*

Allow \$ 5000 /yr

Sub-total B - \$ 1531165 /yr

C. Structure Maintenance Costs

1. *Inspection (assume 3 persons inspection crew for 5 weeks once every 4 years)*

| | | | | | | |
|-----------------|-----|----------|-----|--------|----------|-----|
| Senior engineer | 100 | hrs/yr @ | 150 | \$/hr= | \$ 15000 | /yr |
| Engineer 1 | 100 | hrs/yr @ | 120 | \$/hr= | \$ 12000 | /yr |
| Engineer 2 | 100 | hrs/yr @ | 120 | \$/hr= | \$ 12000 | /yr |

2. *Tunnel cleaning and maintenance (assume 4 persons crew on single shift basis throughout year)*

Labourers 23296 hrs/yr @ 65 \$/hr= \$ 1514240 /yr

3. *Street cleaner vehicle*

Street cleaner vehicle= \$ 65000
written-off over 15 years = \$ 4333.3 /yr

4. *Street cleaner vehicle maintenance and fuel*

Allow \$ 5000 /yr

5. *Washer truck*

Washing truck= \$ 40000
written-off over 15 years = \$ 2666.7 /yr

6. *Washer truck maintenance and fuel*

Allow \$ 5000 /yr

Sub-total C - \$ 1570240 /yr

Total Maintenance Cost - \$ \$ 5,177,053 /yr

APPENDIX D2

Rail Tunnel – Single Bore Operating and Maintenance Costs

Estimation of annual operating and maintenance costs

A. Energy Costs

- 1. Tunnel lighting 3000000 kWh/yr @ 0.1 \$/kWh= \$ 300000 /yr
 - 2. Ventilation fans 400000 kWh/yr @ 0.1 \$/kWh= \$ 40000 /yr
 - 3. Sump pumps 20000 kWh/yr @ 0.1 \$/kWh= \$ 2000 /yr
- Sub-total A - \$ **342,000** /yr

B. Electrical Maintenance Costs

- 1. Labour (Assume 1 electricians for 5 days per week throughout year)

Electrician 1 2080 hrs/yr @ 84 \$/hr= \$ 174720 /yr

- 2. Materials/replacement of equipment (assume electrical equipment replaced once every 40 years)

Electrical equipment cost= \$ 1000000
written-off over 40 years = \$ 25000 /yr

Sub-total B - \$ **199,720** /yr

C. Mechanical Maintenance Costs

- 1. Labour (Assume 1 mechanic/maintenance technician for 5 days per week throughout year)

Mechanic 2080 hrs/yr @ 70 \$/hr= \$ 145600 /yr

- 2. Materials/replacement of equipment (assume mechanical equipment replaced every 40 years)

Ventilation system cost= \$ 1000000
written-off over 40 years = \$ 25000 /yr

Emergency generators= \$ 500000
written-off over 40 years = \$ 12500 /yr

Drainage pumps cost= \$ 200000
written-off over 40 years = \$ 5000 /yr

Sub-total C - \$ **188,100** /yr

contd.

Estimation of annual operating costs

D. Structure Maintenance Costs

1. Inspection (assume 3 person inspection crew for 10 weeks once every 4 years)

| | | | | | | | |
|-----------------|-----|----------|-----|--------|----|-------|-----|
| Senior engineer | 100 | hrs/yr @ | 150 | \$/hr= | \$ | 15000 | /yr |
| Engineer 1 | 100 | hrs/yr @ | 120 | \$/hr= | \$ | 12000 | /yr |
| Engineer 2 | 100 | hrs/yr @ | 120 | \$/hr= | \$ | 12000 | /yr |

2. Tunnel Cleaning (Assume 4 person crew for 13 weeks once a year)

| | | | | | | | |
|-----------|------|----------|----|--------|----|--------|-----|
| Labourers | 2080 | hrs/yr @ | 65 | \$/hr= | \$ | 135200 | /yr |
|-----------|------|----------|----|--------|----|--------|-----|

3. Sump clean-up (Assume 2 person crew for 2 days twice a year)

| | | | | | | | |
|-----------|----|----------|----|--------|----|------|-----|
| Labourers | 64 | hrs/yr @ | 65 | \$/hr= | \$ | 4160 | /yr |
|-----------|----|----------|----|--------|----|------|-----|

4. Structure maintenance (Assume 4 person crew for 13 weeks once a year)

| | | | | | | | |
|-----------|------|----------|----|--------|----|--------|-----|
| Labourers | 2080 | hrs/yr @ | 65 | \$/hr= | \$ | 135200 | /yr |
|-----------|------|----------|----|--------|----|--------|-----|

5. Permanent Way Inspection (Assume 2 persons inspect track 3 times per week)

| | | | | | | | |
|-----------|-----|----------|----|--------|----|-------|-----|
| Labourers | 312 | hrs/yr @ | 65 | \$/hr= | \$ | 20280 | /yr |
|-----------|-----|----------|----|--------|----|-------|-----|

6. Permanent way maintenance (Assume 4 person crew for 12 weeks once a year)

| | | | | | | | |
|-----------|------|----------|----|--------|----|--------|-----|
| Labourers | 1920 | hrs/yr @ | 65 | \$/hr= | \$ | 124800 | /yr |
|-----------|------|----------|----|--------|----|--------|-----|

Sub-total D - \$ 280280 /yr

Total estimated maintenance cost=\$ 1,010,100 /yr

**Newfoundland Fixed Link
Rail System - Estimation of Annual Operating Cost**

Item **Annual Cost**

Assumptions:

Facility operates 12 hours/day. 7 days/week 50% of this time the facility is fully staffed with toll collectors the remainder of the time only 1 toll booth is operating on each side.

| A | Salaried Staff | Mult. | Unit | Rate | |
|---|---|--------|------|------------|------------------|
| | Tunnel Manager | 1.4 | Year | 100,000 | 140,000 |
| | Tunnel Operator | 1.4 | Year | 100,000 | 140,000 |
| | Apprentice Operator | 1.4 | Year | 60,000 | 84,000 |
| | Site Maintenance Mgr | 1.4 | Year | 50,000 | 70,000 |
| B | Hourly Staff | Quant | Unit | Rate | |
| | Toll collector-(2FT.4PT, 1PT Spare) | 7,000 | h | 50 | 350,000 |
| | Site Labour | 4,000 | h | 75 | 300,000 |
| | Electricians | 6,000 | h | 84 | 504,000 |
| | Mechanics | 6,000 | h | 70 | 420,000 |
| | Train Drivers | 8,000 | h | 90 | 720,000 |
| | Loading supervisor | 4,000 | h | 75 | 300,000 |
| | Labourers | 20,000 | h | 65 | 1,300,000 |
| C | Subcontracted Work | Quant | Unit | Rate | |
| | Locomotive maintenance | 1 | LS | 120,000 | 120,000 |
| | Rolling Stock maintenance | 1 | LS | 25,000 | 25,000 |
| | Facility cleaning | 52 | w | 250 | 13,000 |
| | Landscaping | 52 | w | 500 | 26,000 |
| | Building maintenance | 1 | LS | 10,000 | 10,000 |
| D | Power | Quant | Unit | Rate | |
| | | 8922 | MWh | 60 | 535,320 |
| E | Equipment Depreciation (strait-line) | Life | Unit | Cap.Cost | |
| | Locomotive (20years) | 20 | Year | 13,750,000 | 687,500 |
| | Rolling Stock (20years) | 20 | Year | 10,128,421 | 506,421 |
| | Buildings | 30 | Year | 2,912,000 | 97,067 |
| | Roads | 30 | Year | 1,250,000 | 41,667 |
| | Trackwork | 25 | Year | 3,130,000 | 125,200 |
| | OCS system | 15 | Year | 9,298,814 | 619,921 |
| | Signal system | 15 | Year | 6,592,000 | 439,467 |
| | Train loading system | 12 | Year | 500,000 | 41,667 |
| | Heavy vehicles | 12 | Year | 560,000 | 46,667 |
| | Light trucks | 7 | Year | 220,000 | 31,429 |
| Total Annual Operating Cost of Shuttle Train | | | | | 7,694,324 |

APPENDIX E

Schedules

APPENDIX E1

Road Tunnel

Appendix E1-1

Single Bore 1 TBM - Schedule

Newfoundland Fixed Link Pre-feasibility Study

Single Bored - 1TBM Highway Tunnel - Project Timeline

| ID | Task Name | Duration | Start | Finish | Predecessors | Y1 | Y2 | Y3 | Y4 | Y5 | Y6 | Y7 | Y8 | Y9 | Y10 | Y11 | Y12 | Y13 | Y14 | Y15 | | |
|----|--------------------------------|------------------|----------|----------|--------------|---------------|----|---------------|----|----|------------|----|----|----|-----|-----|-----|-----|-----|-----|--|--|
| 1 | Planning | 780 days | 2/1/18 | 1/27/21 | | [Summary Bar] | | | | | | | | | | | | | | | | |
| 2 | Planning | 780 days | 2/1/18 | 1/27/21 | | [Task Bar] | | | | | | | | | | | | | | | | |
| 3 | Design | 520 days | 1/28/21 | 1/25/23 | | | | [Summary Bar] | | | | | | | | | | | | | | |
| 4 | Design | 520 days | 1/28/21 | 1/25/23 | 2 | | | [Task Bar] | | | | | | | | | | | | | | |
| 5 | Preliminary Activities | 1813 days | 1/26/23 | 1/7/30 | | | | [Summary Bar] | | | | | | | | | | | | | | |
| 6 | Procure TBM | 325 days | 1/26/23 | 4/24/24 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 7 | Tunnel liner plant set-up | 213 days | 1/26/23 | 11/20/23 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 8 | Tunnel liner manufacture | 1600 days | 11/21/23 | 1/7/30 | 7 | | | | | | [Task Bar] | | | | | | | | | | | |
| 9 | Construction | 2343 days | 1/26/23 | 1/19/32 | | | | [Summary Bar] | | | | | | | | | | | | | | |
| 10 | Set-up TBM | 32 days | 4/25/24 | 6/7/24 | 6 | | | | | | [Task Bar] | | | | | | | | | | | |
| 11 | Tunnel drive/maintenance | 1680 days | 6/10/24 | 11/15/30 | 10 | | | | | | [Task Bar] | | | | | | | | | | | |
| 12 | Remove TBM | 21 days | 11/18/30 | 12/16/30 | 11 | | | | | | | | | | | | | | | | | |
| 13 | Tunnel clean up | 222 days | 12/17/30 | 10/22/31 | 12 | | | | | | | | | | | | | | | | | |
| 14 | Tunnel finishes | 285 days | 12/17/30 | 1/19/32 | 12 | | | | | | | | | | | | | | | | | |
| 15 | Tunnel mechanical Electrical | 285 days | 12/17/30 | 1/19/32 | 12 | | | | | | | | | | | | | | | | | |
| 16 | North approach | 186 days | 1/26/23 | 10/12/23 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 17 | South Approach | 186 days | 1/26/23 | 10/12/23 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 18 | North Approach Finish | 93 days | 12/17/30 | 4/24/31 | 12 | | | | | | | | | | | | | | | | | |
| 19 | South Approach Finish | 93 days | 12/17/30 | 4/24/31 | 12 | | | | | | | | | | | | | | | | | |
| 20 | Tunnel Control Building | 186 days | 12/26/23 | 9/10/24 | | | | | | | [Task Bar] | | | | | | | | | | | |
| 21 | North Holding Area & Buildings | 186 days | 12/26/23 | 9/10/24 | | | | | | | [Task Bar] | | | | | | | | | | | |
| 22 | South Holding Area & Buildings | 186 days | 12/26/23 | 9/10/24 | | | | | | | [Task Bar] | | | | | | | | | | | |

Project: HWY Sg.Bore Tunnel Schedu
Date: 11/15/17

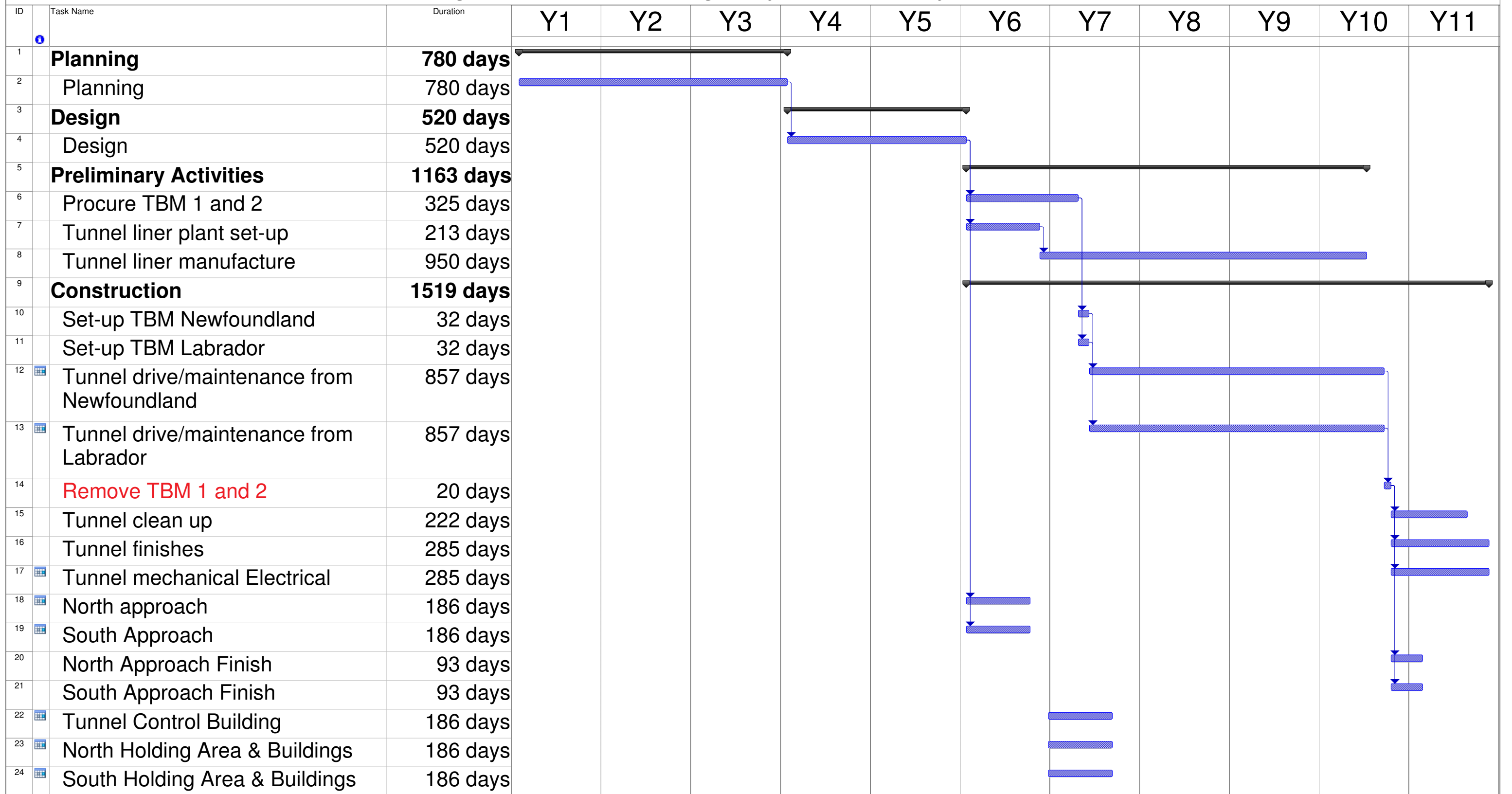
| | | | | | | | | | | | | | |
|-----------|--|-----------------|--|--------------------|--|-----------------------|--|----------------|--|----------------|--|-------------|--|
| Task | | Summary | | External Milestone | | Inactive Summary | | Manual Task | | Manual Summary | | Finish-only | |
| Split | | Project Summary | | Inactive Task | | Manual Summary Rollup | | Manual Summary | | Start-only | | Progress | |
| Milestone | | External Tasks | | Inactive Milestone | | Duration-only | | Start-only | | Start-only | | Deadline | |

Appendix E1-2

Single Bore 2 TBM - Schedule

Newfoundland Fixed Link Pre-feasibility Study

Single Bored - 2TBM Highway Tunnel - Project Timeline



Project: HWY Sg.Bore Tunnel Schedu
Date: 11/15/17

| | | | | | | | | | | | |
|-----------|--|-----------------|--|--------------------|--|------------------|--|-----------------------|--|-------------|--|
| Task | | Summary | | External Milestone | | Inactive Summary | | Manual Summary Rollup | | Finish-only | |
| Split | | Project Summary | | Inactive Task | | Manual Task | | Manual Summary | | Progress | |
| Milestone | | External Tasks | | Inactive Milestone | | Duration-only | | Start-only | | Deadline | |

Appendix E1-3

Twin Bore 2 TBM - Schedule

Newfoundland Fixed Link Pre-feasibility Study Twin Bored - 2TBM Highway Tunnel - Project Timeline

| ID | Task Name | Duration | Start | Finish | Predecessors | Y1 | Y2 | Y3 | Y4 | Y5 | Y6 | Y7 | Y8 | Y9 | Y10 | Y11 | Y12 | Y13 | Y14 | Y15 | | |
|----|--------------------------------|------------------|----------|----------|--------------|---------------|----|---------------|---------------|----|------------|------------|----|----|-----|-----|-----|-----|-----|-----|--|--|
| 1 | Planning | 780 days | 2/1/18 | 1/27/21 | | [Summary Bar] | | | | | | | | | | | | | | | | |
| 2 | Planning | 780 days | 2/1/18 | 1/27/21 | | [Task Bar] | | | | | | | | | | | | | | | | |
| 3 | Design | 520 days | 1/28/21 | 1/25/23 | | | | [Summary Bar] | | | | | | | | | | | | | | |
| 4 | Design | 520 days | 1/28/21 | 1/25/23 | 2 | | | [Task Bar] | | | | | | | | | | | | | | |
| 5 | Preliminary Activities | 1913 days | 1/26/23 | 5/27/30 | | | | | [Summary Bar] | | | | | | | | | | | | | |
| 6 | Procure TBM | 325 days | 1/26/23 | 4/24/24 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 7 | Tunnel liner plant set-up | 213 days | 1/26/23 | 11/20/23 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 8 | Tunnel liner manufacture | 1700 days | 11/21/23 | 5/27/30 | 7 | | | | | | [Task Bar] | | | | | | | | | | | |
| 9 | Construction | 2350 days | 1/26/23 | 1/28/32 | | | | | [Summary Bar] | | | | | | | | | | | | | |
| 10 | Set-up TBM 1 | 32 days | 4/25/24 | 6/7/24 | 6 | | | | | | | [Task Bar] | | | | | | | | | | |
| 11 | Set-up TBM 2 | 32 days | 5/13/24 | 6/25/24 | | | | | | | | [Task Bar] | | | | | | | | | | |
| 12 | Tunnel drive 1/maintenance | 1680 days | 6/10/24 | 11/15/30 | 10 | | | | | | [Task Bar] | | | | | | | | | | | |
| 13 | Tunnel drive 2/maintenance | 1680 days | 7/18/24 | 12/25/30 | 11 | | | | | | [Task Bar] | | | | | | | | | | | |
| 14 | Remove TBM 1 | 21 days | 11/18/30 | 12/16/30 | 12 | | | | | | | | | | | | | | | | | |
| 15 | Remove TBM 2 | 21 days | 12/26/30 | 1/23/31 | 13 | | | | | | | | | | | | | | | | | |
| 16 | Tunnel clean up 1 | 222 days | 11/18/30 | 9/23/31 | 12 | | | | | | | | | | | | | | | | | |
| 17 | Tunnel clean up 2 | 222 days | 12/26/30 | 10/31/31 | 13 | | | | | | | | | | | | | | | | | |
| 18 | Tunnel finishes 1 | 285 days | 12/17/30 | 1/19/32 | 14 | | | | | | | | | | | | | | | | | |
| 19 | Tunnel finishes 2 | 285 days | 12/17/30 | 1/19/32 | 14 | | | | | | | | | | | | | | | | | |
| 20 | Tunnel mechanical Electrical 1 | 285 days | 12/26/30 | 1/28/32 | 13 | | | | | | | | | | | | | | | | | |
| 21 | Tunnel mechanical Electrical 2 | 285 days | 12/26/30 | 1/28/32 | 13 | | | | | | | | | | | | | | | | | |
| 22 | North approach | 186 days | 1/26/23 | 10/12/23 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 23 | South Approach | 186 days | 1/26/23 | 10/12/23 | 4 | | | | | | [Task Bar] | | | | | | | | | | | |
| 24 | North Approach Finish | 93 days | 1/24/31 | 6/3/31 | 15,14 | | | | | | | | | | | | | | | | | |
| 25 | South Approach Finish | 93 days | 1/24/31 | 6/3/31 | 15,14 | | | | | | | | | | | | | | | | | |
| 26 | Tunnel Control Building | 186 days | 12/26/23 | 9/10/24 | | | | | | | [Task Bar] | | | | | | | | | | | |
| 27 | North Holding Area & Buildings | 186 days | 12/26/23 | 9/10/24 | | | | | | | [Task Bar] | | | | | | | | | | | |
| 28 | South Holding Area & Buildings | 186 days | 12/26/23 | 9/10/24 | | | | | | | [Task Bar] | | | | | | | | | | | |

| | | | | | | | | |
|--|-----------------------------------|-------------------------------|-----------------------------------|------------------------------|-------------------------------------|------------------------|------------------------|------------------------|
| Project: HWY Sg.Bore Tunnel Schedu Date: 11/15/17 | Task [Blue Bar] | Summary [Grey Bar] | External Milestone [Black Arrow] | Inactive Summary [White Bar] | Manual Summary Rollup [White Arrow] | Finish-only [Blue Bar] | Progress [Black Arrow] | Deadline [Green Arrow] |
| Split [Dotted Blue Bar] | Project Summary [Dotted Grey Bar] | External Tasks [Grey Bar] | Inactive Task [Black Arrow] | Manual Task [White Bar] | Manual Summary [Green Bar] | Progress [Black Arrow] | Deadline [Green Arrow] | |
| Milestone [Black Diamond] | External Tasks [Grey Bar] | Inactive Milestone [Grey Bar] | Inactive Milestone [Grey Diamond] | Duration-only [White Bar] | Start-only [Blue Bar] | Progress [Black Arrow] | Deadline [Green Arrow] | |

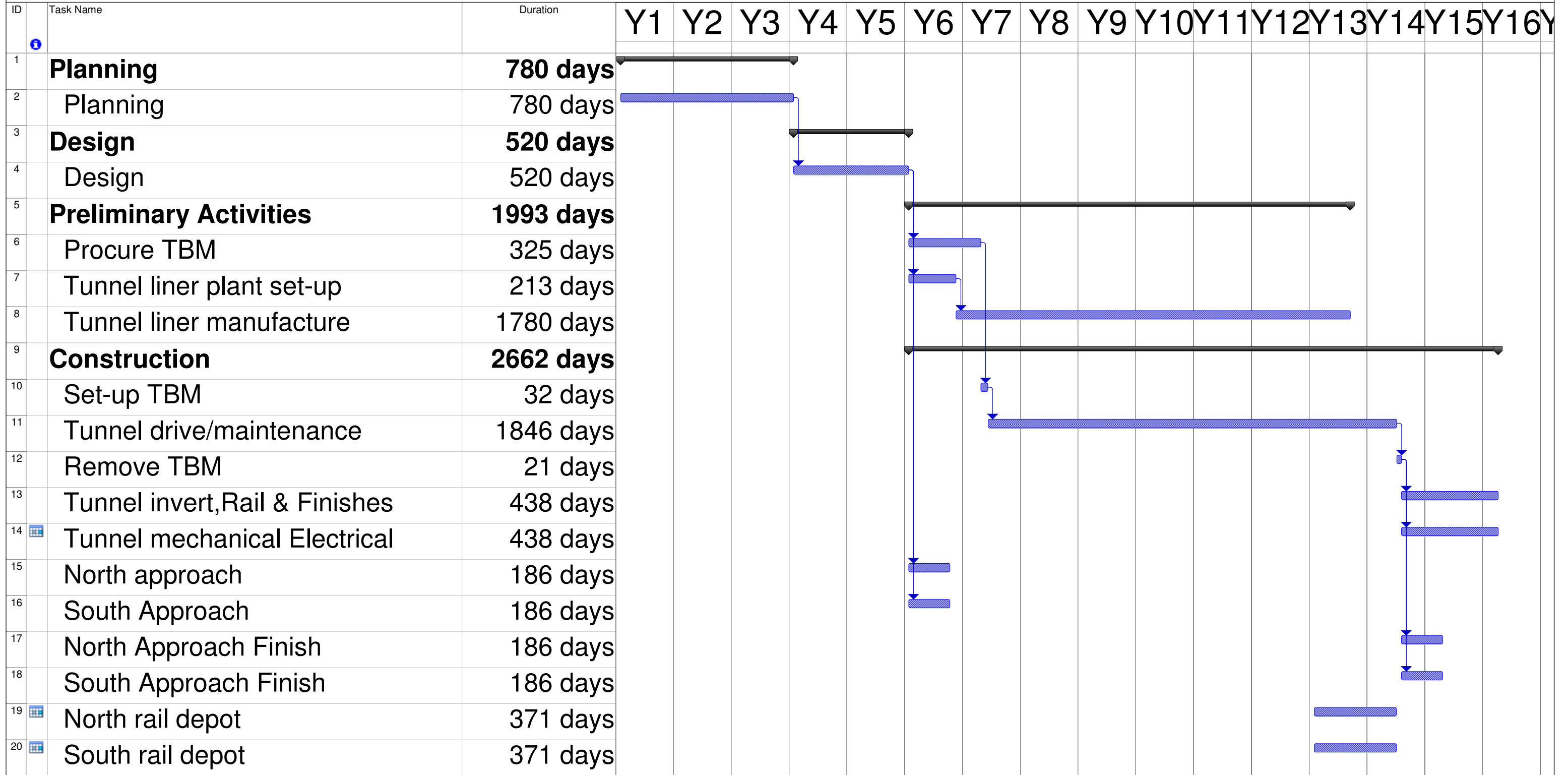
APPENDIX E2

Rail Tunnel

Appendix E2-1

Single Bore 1 TBM - Schedule

Newfoundland Fixed Link Pre-feasibility Study Single Bored - 1TBM Rail Tunnel - Project Timeline



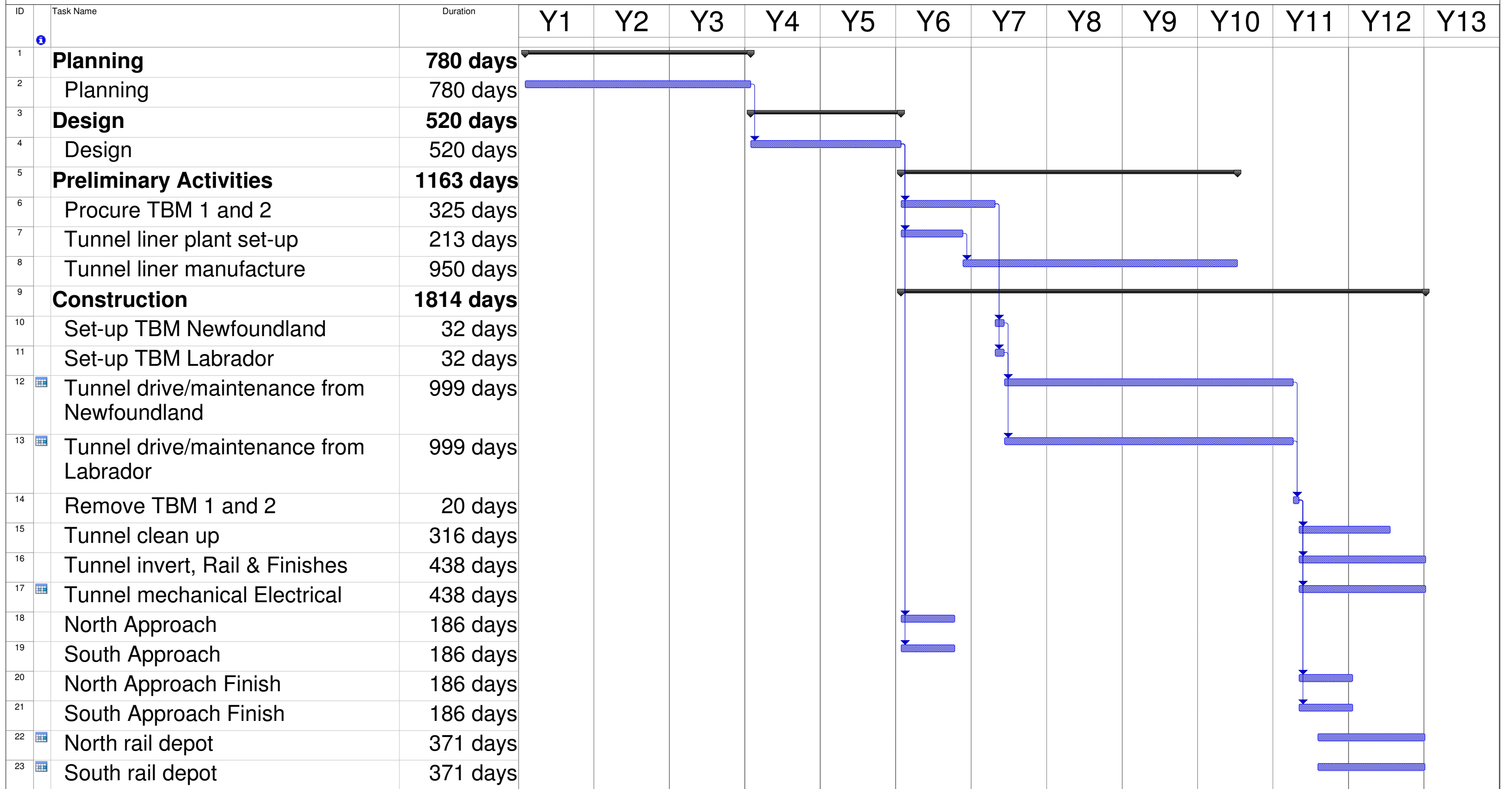
| | | | | | | | | | | | | |
|---------------------------------------|-----------|--|--------------------|--|--------------------|--|-----------------------|--|----------|--|----------|--|
| Project: E2.1 Single Bore - 1TBM - Sc | Task | | Project Summary | | Inactive Milestone | | Manual Summary Rollup | | Progress | | Deadline | |
| | Split | | External Tasks | | Inactive Summary | | Manual Summary | | Deadline | | | |
| | Milestone | | External Milestone | | Manual Task | | Start-only | | | | | |
| | Summary | | Inactive Task | | Duration-only | | Finish-only | | | | | |

Appendix E2-2

Single Bore 2 TBM - Schedule

Newfoundland Fixed Link Pre-feasibility Study

Single Bored - 2TBM Rail Tunnel - Project Timeline



Project: E2.2 Single Bore - 2TBM - Sc
 Date: 11/16/17

| | | | | | | | | | | | |
|-----------|-------------------|-----------------|------------------|--------------------|-------------|------------------|------------------|-----------------------|-----------------|-------------|----------|
| Task | [Blue Bar] | Summary | [Grey Bar] | External Milestone | [Diamond] | Inactive Summary | [Light Blue Bar] | Manual Summary Rollup | [Dark Blue Bar] | Finish-only | [Square] |
| Split | [Dotted Blue Bar] | Project Summary | [Light Grey Bar] | Inactive Task | [White Bar] | Manual Task | [Green Bar] | Manual Summary | [Dark Grey Bar] | Progress | [Arrow] |
| Milestone | [Diamond] | External Tasks | [Dark Grey Bar] | Inactive Milestone | [Diamond] | Duration-only | [Light Blue Bar] | Start-only | [Square] | Deadline | [Arrow] |

APPENDIX F

Economic and Financial Analysis – Rail Tunnel

APPENDIX F1

Rail Tunnel, 1 TBM

Economic and Financial Analysis

Appendix F1-1

Rail Tunnel, 1 TBM, 100% Tolls

Variables - Option 1 TBM 100% Tolls - Rail Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|--------------------|--------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,595 | \$1,370 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 8,704,424 | \$ 8,704,424 | |
| | | | |
| Tolls | | | |
| Per passenger | \$11.30 | \$11.30 | |
| Per passenger vehicle | \$47.66 | \$47.66 | |
| Per commercial vehicle | \$120.75 | \$120.75 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|--------|----------|-----------------|
| | Rates | Fuel | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |

Appendix F1-2

Rail Tunnel, 1 TBM, 150% Tolls

Variables - Option 1 TBM 150% Tolls - Rail Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|---------------------------|---------------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,279 | \$1,010 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 8,704,424 | \$ 8,704,424 | |
| | | | |
| Tolls | | | |
| Per passenger | \$16.95 | \$16.95 | |
| Per passenger vehicle | \$71.48 | \$71.48 | |
| Per commercial vehicle | \$181.13 | \$181.13 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|----------------|----------|----------------|
| | Rates | Fuel Surcharge | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |

| | | | | |
|---------------------------|----------|--------|----------|-----------------|
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |
|---------------------------|----------|--------|----------|-----------------|

Appendix F1-3

Rail Tunnel, 1 TBM, 200% Tolls

Variables - Option 1 TBM 200% Tolls - Rail Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|--------------------|--------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,015 | \$725 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 8,704,424 | \$ 8,704,424 | |
| | | | |
| Tolls | | | |
| Per passenger | \$22.60 | \$22.60 | |
| Per passenger vehicle | \$95.31 | \$95.31 | |
| Per commercial vehicle | \$241.50 | \$241.50 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|----------------|----------|----------------|
| | Rates | Fuel Surcharge | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |

| | | | | |
|---------------------------|----------|--------|----------|-----------------|
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |
|---------------------------|----------|--------|----------|-----------------|

APPENDIX F2

Rail Tunnel, 2 TBMs

Economic and Financial Analysis

Appendix F2-1
Rail Tunnel, 2 TBMs, 100% Tolls

Economic Analysis - Option 2 TBM 100% Tolls - Rail Tunnel

Table with columns for Year (2014-2045), Traffic (Existing Bthc Isle Ferry, One Time Surp 30%, Existing Golf Ferries, Diversion from Golf Ferries (40%), Diversion from Golf Ferries (60%), Total Traffic for Fixed Link (40%), Total Traffic for Fixed Link (60%)), and Revenue (Total Revenue for Fixed Link (40%), Total Revenue for Fixed Link (60%), Total Operating Costs for Fixed Link, Total Net Revenue for Fixed Link, Total Net Revenue including subsidy saving, Total Capital Cost). Rows include Revenue, Economic Analysis, and Economic Analysis with sub-categories like Net Present Value of Fixed Link Cash Flows and Benefit/Cost Ratio and IRR.

Financial Analysis - Option 2 TBM 100% Tolls - Rail Tunnel

Table with columns for Year (2014-2045), Financing During Construction (40% Diversion, 60% Diversion), Financing Base Case (40% Diversion, 60% Diversion), and Subsidized Financing (40% Diversion, 60% Diversion). Rows include Project Cost, Annual Debt Amortization (CSP), Net Cash Flow, IRR excluding IDC, and Internal Rate of Return (IRR).

Variables - Option 2 TBM 100% Tolls - Rail Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|-----------------------|-----------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,655 | \$1,430 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 8,704,424 | \$ 8,704,424 | |
| | | | |
| Tolls | | | |
| Per passenger | \$11.30 | \$11.30 | |
| Per passenger vehicle | \$47.66 | \$47.66 | |
| Per commercial vehicle | \$120.75 | \$120.75 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|----------------|----------|----------------|
| | Rates | Fuel Surcharge | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |

| | | | | |
|---------------------------|----------|--------|----------|-----------------|
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |
|---------------------------|----------|--------|----------|-----------------|

Appendix F2-2
Rail Tunnel, 2 TBMs, 150% Tolls

Variables - Option 2 TBM 150% Tolls - Rail Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|-----------------------|-----------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,340 | \$1,040 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 8,704,424 | \$ 8,704,424 | |
| | | | |
| Tolls | | | |
| Per passenger | \$16.95 | \$16.95 | |
| Per passenger vehicle | \$71.48 | \$71.48 | |
| Per commercial vehicle | \$181.13 | \$181.13 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|----------------|----------|----------------|
| | Rates | Fuel Surcharge | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |

| | | | | |
|---------------------------|----------|--------|----------|-----------------|
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |
|---------------------------|----------|--------|----------|-----------------|

Appendix F2-3
Rail Tunnel, 2 TBMs, 200% Tolls

Variables - Option 2 TBM 200% Tolls -Rail Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|-----------------------|-----------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,045 | \$695 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 8,704,424 | \$ 8,704,424 | |
| | | | |
| Tolls | | | |
| Per passenger | \$22.60 | \$22.60 | |
| Per passenger vehicle | \$95.31 | \$95.31 | |
| Per commercial vehicle | \$241.50 | \$241.50 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|----------------|----------|----------------|
| | Rates | Fuel Surcharge | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |

| | | | | |
|---------------------------|----------|--------|----------|-----------------|
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |
|---------------------------|----------|--------|----------|-----------------|

APPENDIX G

Economic and Financial Analysis – Road Tunnel

APPENDIX G1

Road Tunnel, 1 TBM

Economic and Financial Analysis

Appendix G1-1

Road Tunnel, 1 TBM, 100% Tolls

Economic Analysis - Option 1 TBM 100% Tolls - Road Tunnel

Table with columns: Year, Phase 1, Phase 2, Phase 3, Phase 4. Rows include Traffic (Existing, One Time Surge, Diversion), Revenue (Fixed Link, Operating Costs, Net Revenue), and Economic Analysis (Net Present Value, Benefit/Cost Ratio).

Financial Analysis - Option 1 TBM 100% Tolls - Road Tunnel

Table with columns: Year, Phase 1, Phase 2, Phase 3, Phase 4. Rows include Financing During Construction (40% and 60% Diversion Case), Financing Base Case (40% and 60% Diversion Case), and Subsidized Financing (40% and 60% Diversion Case).

Variables - Option 1 TBM 100% Tolls - Road Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|---------------------------|---------------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$2,041 | \$1,820 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 7,648,109 | \$ 7,648,109 | |
| | | | |
| Tolls | | | |
| Per passenger | \$11.30 | \$11.30 | |
| Per passenger vehicle | \$47.66 | \$47.66 | |
| Per commercial vehicle | \$120.75 | \$120.75 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|--------|----------|-----------------|
| | Rates | Fuel | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |

Appendix G1-2

Road Tunnel, 1 TBM, 150% Tolls

Economic Analysis - Option 1 TBM 150% Tolls - Road Tunnel

| Year | 2014 | 2015 | Phase 1 - Planning Approval | | | | | | | | | | Phase 2 - Design | | | | | | | | | | Phase 3 - Construction | | | | | | | | | | Phase 4 - Operation | | | | | | | | | | | | | | | | | | | |
|---|------------------------------------|------|-----------------------------|---|---|---|---|---|---|---|---|----|------------------|----|----|----|----|----|----|----|----|----|------------------------|----|----|----|----|----|----|----|----|----|---------------------|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|
| | | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 |
| Traffic | Total | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Existing Belle Isle Ferry | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | One Time Surge 30% | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Existing Golf Ferries | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Diversion from Golf Ferries (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Diversion from Golf Ferries (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Total Traffic for Fixed Link (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Total Traffic for Fixed Link (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Revenue | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Total Revenue for Fixed Link (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Revenue for Fixed Link (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Operating Costs for Fixed Link | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Net Revenue for Fixed Link | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Net Revenue including subsidy saving | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Capital Cost | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Economic Analysis | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Benefit/Cost Ratio and IRR | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Economic Analysis | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Benefit/Cost Ratio and IRR (including subsidy saving) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Financial Analysis - Option 1 TBM 150% Tolls - Road Tunnel

| Year | 2014 | 2015 | Phase 1 - Planning Approval | | | | | | | | | | Phase 2 - Design | | | | | | | | | | Phase 3 - Construction | | | | | | | | | | Phase 4 - Operation | | | | | | | | | | | | | | | | | | | |
|-------------------------------|---------------------|------|-----------------------------|---|---|---|---|---|---|---|---|----|------------------|----|----|----|----|----|----|----|----|----|------------------------|----|----|----|----|----|----|----|----|----|---------------------|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|
| | | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 |
| Financing During Construction | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Financing Base Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Subsided Financing | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Financing Base Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Variables - Option 1 TBM 150% Tolls - Road Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|--------------------|--------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,725 | \$1,448 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 7,648,109 | \$ 7,648,109 | |
| | | | |
| Tolls | | | |
| Per passenger | \$16.95 | \$16.95 | |
| Per passenger vehicle | \$71.48 | \$71.48 | |
| Per commercial vehicle | \$181.13 | \$181.13 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|--------|----------|-----------------|
| | Rates | Fuel | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |

Appendix G1-3

Road Tunnel, 1 TBM, 200% Tolls

Economic Analysis - Option 1 TBM 200% Tolls - Road Tunnel

| Year | 2014 | 2015 | Phase 1 - Planning Approval | | | | | | | | | | Phase 2 - Design | | | | | | | | | | Phase 3 - Construction | | | | | | | | | | Phase 4 - Operation | | | | | | | | | | | | | | | | | | | |
|---|------------------------------------|------|-----------------------------|---|---|---|---|---|---|---|---|----|------------------|----|----|----|----|----|----|----|----|----|------------------------|----|----|----|----|----|----|----|----|----|---------------------|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|
| | | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 |
| Traffic | Total | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Existing Belle Isle Ferry | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | One Time Surge 30% | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Existing Golf Ferries | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Division from Golf Ferries (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Division from Golf Ferries (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Total Traffic for Fixed Link (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Total Traffic for Fixed Link (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Revenue | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | Total Revenue for Fixed Link (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Revenue for Fixed Link (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Operating Costs for Fixed Link | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Net Revenue for Fixed Link | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Net Revenue (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Net Revenue (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Net Present Value (40%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Net Present Value (60%) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Every Subsidy Saving | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Net Revenue including subsidy saving | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Total Capital Cost | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Net Present Value (\$2017) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Economic Analysis | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Net Present Value of Fixed Link Cash Flows | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Benefit/Cost Ratio and IRR | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Economic Analysis | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Net Present Value of Fixed Link Cash Flows (including subsidy saving) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Benefit/Cost Ratio and IRR (including subsidy saving) | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Financial Analysis - Option 1 TBM 200% Tolls - Road Tunnel

| Year | 2014 | 2015 | Phase 1 - Planning Approval | | | | | | | | | | Phase 2 - Design | | | | | | | | | | Phase 3 - Construction | | | | | | | | | | Phase 4 - Operation | | | | | | | | | | | | | | | | | | | |
|-------------------------------|--------------------|------|-----------------------------|---|---|---|---|---|---|---|---|----|------------------|----|----|----|----|----|----|----|----|----|------------------------|----|----|----|----|----|----|----|----|----|---------------------|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|
| | | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 |
| Financing During Construction | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Subsidized Financing | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 40% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | 60% Diversion Case | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

Variables - Option 1 TBM 200% Tolls - Road Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|--------------------|--------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,454 | \$1,140 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 7,648,109 | \$ 7,648,109 | |
| | | | |
| Tolls | | | |
| Per passenger | \$22.60 | \$22.60 | |
| Per passenger vehicle | \$95.31 | \$95.31 | |
| Per commercial vehicle | \$241.50 | \$241.50 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|--------|----------|-----------------|
| | Rates | Fuel | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |

APPENDIX G2

Road Tunnel, 2 TBMs

Economic and Financial Analysis

Appendix G2-1

Road Tunnel, 2 TBMs, 100% Tolls

Variables - Option 2 TBM 100% Tolls - Road Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|--------------------|--------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$2,104 | \$1,880 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 7,648,109 | \$ 7,648,109 | |
| | | | |
| Tolls | | | |
| Per passenger | \$11.30 | \$11.30 | |
| Per passenger vehicle | \$47.66 | \$47.66 | |
| Per commercial vehicle | \$120.75 | \$120.75 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|--------|----------|-----------------|
| | Rates | Fuel | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |

Appendix G2-2

Road Tunnel, 2 TBMs, 150% Tolls

Economic Analysis - Option 2 TBM 150% Tolls - Road Tunnel

Main economic analysis table with columns for Year (2018-2050) and rows for Traffic (Existing Belle Isle Ferry, One Time Surge 30%, Existing Golf Ferries, Diversion from Golf Ferries (40%), Diversion from Golf Ferries (60%), Total Traffic for Fixed Link (40%), Total Traffic for Fixed Link (60%), Revenue (Total Revenue for Fixed Link (40%), Total Revenue for Fixed Link (60%), Total Operating Costs for Fixed Link, Total Net Revenue for Fixed Link, Total Net Revenue for Fixed Link (including subsidy saving), Total Capital Cost), Economic Analysis (Net Present Value of Fixed Link Cash Flows, Benefit/Cost Ratio and IRR), and Economic Analysis (Net Present Value of Fixed Link Cash Flows including subsidy saving, Benefit/Cost Ratio and IRR including subsidy saving).

Financial Analysis - Option 2 TBM 150% Tolls - Road Tunnel

Main financial analysis table with columns for Year (2018-2050) and rows for Financing During Construction (40% Diversion Case, 60% Diversion Case), Financing Base Case (40% Diversion Case, 60% Diversion Case), and Subsidized Financing (40% Diversion Case, 60% Diversion Case).

Variables - Option 2 TBM 150% Tolls - Road Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|--------------------|--------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,785 | \$1,495 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 7,648,109 | \$ 7,648,109 | |
| | | | |
| Tolls | | | |
| Per passenger | \$16.95 | \$16.95 | |
| Per passenger vehicle | \$71.48 | \$71.48 | |
| Per commercial vehicle | \$181.13 | \$181.13 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|--------|----------|-----------------|
| | Rates | Fuel | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |

Appendix G2-3

Road Tunnel, 2 TBMs, 200% Tolls

Variables - Option 2 TBM 200% Tolls - Road Tunnel

| | 40% Diversion Case | 60% Diversion Case | |
|--|-----------------------|-----------------------|--|
| Debt Financing (Real) | 8.0% | 8.0% | |
| Long Term Inflation | 2.5% | 2.5% | |
| Equity Real | 11.0% | 11.0% | |
| Equity Tax Rate | 40.0% | 40.0% | |
| Equity Pre Tax - Real | 18.3% | 18.3% | |
| Equity PreTax - Nominal | 22.5% | 22.5% | |
| | | | |
| % debt financed | 0.0% | 0.0% | |
| % equity financed | 0.0% | 0.0% | |
| Nominal Grant \$ Millions | \$1,505 | \$1,145 | |
| Blended Cost of Capital - Real | 0.0% | 0.0% | |
| Blended Cost of Capital - Nominal | 0.0% | 0.0% | |
| | | | |
| Financing during Construction | 5.0% | 5.0% | |
| | | | |
| Social Discount Rate - Nominal | 10.0% | 10.0% | |
| Social Discount Rate - Real | 7.5% | 7.5% | |
| Grant Discount Rate | 0.0% | 0.0% | |
| | | | |
| Operating Cost | \$ 7,648,109 | \$ 7,648,109 | |
| | | | |
| Tolls | | | |
| Per passenger | \$22.60 | \$22.60 | |
| Per passenger vehicle | \$95.31 | \$95.31 | |
| Per commercial vehicle | \$241.50 | \$241.50 | |
| | | | |
| | | | |
| Passenger Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Commercial Vehicle Labrador - Traditional Growth | 2.5% | 2.5% | |
| Passenger Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Commercial Vehicle Labrador - Surge | 30.0% | 30.0% | |
| Passenger Gulf - Traditional Growth | -1.0% | -1.0% | |
| Passenger Vehicle Gulf - Traditional Growth | -1.0% | -1.0% | |
| Commercial Vehicle Gulf - Traditional Growth | 1.6% | 1.6% | |
| Passenger - Diversion from Gulf | 40.0% | 60.0% | |
| Passenger Vehicle - Diversion from Gulf | 40.0% | 60.0% | |
| Commercial Vehicle- Diversion from Gulf | 40.0% | 60.0% | |
| | | | |
| Ferry Subsidy | \$ 8,000,000 | \$ 8,000,000 | |

St.Barbe - Blanc Sablon, NL

Rates in effect for travel up to May31, 2018.

Passengers Rates (one way)

| | Rates | | Total | Blended Rate |
|-----------------------------------|---------|--|---------|----------------|
| Adults 13 - 64 years | \$11.75 | | \$11.75 | \$9.40 |
| Children 5 - 12 years | \$9.50 | | \$9.50 | \$0.95 |
| Children under 5 | FREE | | FREE | |
| Senior Citizens 65 years + | \$9.50 | | \$9.50 | \$0.95 |
| | | | | \$11.30 |

Vehicle Rates (one way)

| Autos with trailers, campers, motor homes, vans, minibuses | | | | |
|--|----------|--------|----------|-----------------|
| | Rates | Fuel | Total | Blended Rate |
| Autos, pickups up to 20' | \$35.25 | \$0.00 | \$35.25 | \$27.50 |
| Over 20' to 30' | \$51.75 | \$0.00 | \$51.75 | |
| Over 30' to 40' | \$69.00 | \$0.00 | \$69.00 | \$3.45 |
| Over 40' to 50' | \$86.25 | \$0.00 | \$86.25 | |
| Over 50' to 60' | \$103.25 | \$0.00 | \$103.25 | \$15.49 |
| Over 60' | \$120.75 | \$0.00 | \$120.75 | |
| Minibus up to 30' | \$0.00 | \$0.00 | \$0.00 | |
| Buses | \$86.25 | \$0.00 | \$86.25 | \$0.86 |
| Motorcycles / ATVs | \$18.00 | \$0.00 | \$18.00 | \$0.36 |
| Motorcycles / ATVs with sidecar or trailer | \$0.00 | \$0.00 | \$0.00 | |
| Bicycles | \$0.00 | \$0.00 | \$0.00 | |
| | | | | \$47.66 |
| Commercial Vehicle | \$120.75 | \$0.00 | \$120.75 | \$120.75 |

APPENDIX H

Biographies – Project Team

Mark F. Cumby, P. Eng. – Project Sponsor
Regional Director, Energy, Eastern North America, Hatch

Mark Cumby offers over 24 years of progressive and comprehensive experience in a variety of energy projects across the globe. Accomplished in project management, engineering management, and general business management, through work with Tier 1 operators and EPC firms, he has demonstrated a "customer first" focus, with a keen ability to get deliverables driven to closure, and effective problem-solving skills recognized by colleagues and clients.

Prior to joining Hatch in 2016, he was involved in project management and delivery of the drilling modules for the Hebron Project. Presently in the role of Regional Director, Energy for Eastern North America, Mr. Cumby also serves as project manager or sponsor for a number of energy and infrastructure projects in the region.

Mr. Cumby is a registered Professional Engineer in the province of Newfoundland and Labrador.

Jean Habimana, P. Eng., PhD – Project Manager
Director of Tunnel Practice Lead, Eastern North American, Hatch

Jean Habimana is experienced in a variety of aspects of geotechnical and tunnel engineering for both soft ground and hard rock tunneling. He has over 20 years of experience and has worked on design and construction of major projects including tunnels, caverns, shafts, open-cut, cut-and-cover excavations, highways, and bridge foundation. His experience ranges from conceptual design to preparation of engineering final design documents including Geotechnical Baseline Reports, drawings and specifications and providing engineering services during construction.

Mr. Habimana has extensive experience in static and seismic soil-structure interaction analysis and seismic design of tunnels and underground facilities. He notably managed tunnel design and construction projects, and he is skilled in the coordination of different disciplines, the management of client inspections, and completing projects on time and on budget. His experience includes projects in Europe, China, Australia, United States and Canada.

Brian Garrod, P. Eng. – Tunnelling Expert
Director Tunnels, Australasia, Hatch

Brian Garrod is a Director - Tunnelling with Hatch and a world-recognized expert on tunnels and tunnel technology. With multiple publications, he is a sought-after speaker at technical conferences. He has 46 years of general civil engineering experience, the last 36 years devoted solely to tunnelling assignments. Mr. Garrod's technical expertise covers a broad range of tunnel techniques for high and low-strength rocks and soft ground using excavation methods such as earth pressure balance tunnel boring machines (EPB TBM) and the New Austrian Tunnel Method (NATM). Moreover, he specializes in the design of pressurized-face TBM-driven tunnels, one-pass segmental tunnel linings, risk analysis, productivity analyses and cost

estimates for underground projects. Mr. Garrod has developed Hatch's in-house tunnels estimating system that enables his team to produce highly accurate project cost estimates.

With outstanding tunnelling qualifications, Mr. Garrod has been engaged on significant projects around the world such as the Channel Tunnel. In Canada, his expert knowledge has been applied on major projects to expand transit facilities, e.g. Toronto's new Eglinton Crosstown LRT and Vancouver's Skytrain. He also has significant expertise with water conveyance tunnels through various roles on such projects as the new 10 km diversion tunnel at Niagara Falls to serve the Sir Adam Beck hydro power station. Mr. Garrod has significant construction management experience, providing oversight and engineering support during construction for major tunnel projects.

Mr. Garrod was involved in the preparation of the 2004 pre-feasibility study.

***Gabriele Mellies, P. Eng. – Rock and Tunnel Engineering
Senior Project Engineer, Geotechnique and Tunnels, Hatch***

Gabriele Mellies is a Senior Engineer with over 18 years of experience in consulting engineering in the field of geotechnical, rock and tunnel engineering. Ms. Mellies has extensive technical and project management experience from various civil engineering and mining engineering projects for private and federal clients in Canada and Switzerland with a main focus on tunnel and rock slope stabilization projects. Her areas of expertise include geotechnical analysis and design, geotechnical investigations and data analysis, rock mass characterization, ground support design and installation, rockfall hazard assessments, rock slope remediation and geotechnical monitoring.

Ms. Mellies' experience extends from preliminary to final design and preparation of final design documents, including drawings, technical specifications and geotechnical baseline, data and design reports. Ms. Mellies also has extensive experience in providing engineering services during construction for geotechnical and tunnel projects. She is recognized for her client focus and for delivering complex projects in time and on budget.

Ms. Mellies is a registered Professional Engineer in the provinces of Ontario, Québec and Newfoundland and Labrador.

***Djoko Corovic, P. Eng. – Electrical Engineering
Electrical Engineering Design Lead, Hatch***

Djoko Corovic is a licensed professional registered in Ontario, Alberta and Quebec with over 25 years of experience in the field. He has significant design and operational improvements experience for projects across North America and globally. His team leadership and project management skills have been proven for numerous transit infrastructures, tunnels and industrial projects, both in private and public sectors. His skills include performance audits, conceptual design, tendering (including P3) and project estimates.

Mr. Corovic's experience includes detail design, construction and commissioning supervision for electrical and communications infrastructures. He has led electrical and systems teams for

various projects including for rail and road tunnels, transit yards, transit control centers, underground stations and surface stops. His experience includes HAZOP studies and provisions of safety egress components for rail and road tunnels, elevated structures and underground stations. Mr. Corovic has performed project solutions safety and security analysis and design details including surveillance coverage, intruder protection and access control. He is currently engaged with several DMBF delivery exercises across Canada.

***Amir Golpaygan, P. Eng. – Mechanical Engineering
Principal Engineer/ Fire Life Safety & Security, Hatch***

Dr. Amir Golpaygan has 20 years of experience in mechanical engineering design and analysis, fire & life safety engineering, tunnel ventilation and engineering management including performance-based fire engineering based on North American fire standards including National Fire Protection Association Standard (NFPA) 130 and 502. Dr. Golpaygan has extensive knowledge and experience in the application of simulation techniques in engineering design, in specific use of Subway Environmental System (SES) and Computational Fluid Dynamics (CFD) as an analysis tool for the design verification, and optimization of tunnel ventilation systems, subway station smoke management, odor control system, ventilation system design and optimization, smoke dispersion analysis, and gas handling systems. He has been responsible for the design of numerous ventilation and smoke control systems where the complexity of the design has required the use of CFD. His work has resulted in many innovate techniques used to assess performance and options available to the design team.

Dr. Golpaygan's experience has involved in the application of the NFPA130 and NFPA502 and the Ontario Building Code (OBC) in the design of normal and emergency ventilation systems for subway systems, light rail transit systems, tunnel fire, analysis of aerodynamics of trains in tunnels, and the prediction of smoke movement and fires in tunnels and underground infrastructures.

***John Hemingway, P. Eng., PTOE – Civil Engineering
Senior Project Manager, Hatch***

John Hemingway is a Senior Project Manager with more than 40 years of experience in the planning, designing and delivering transportation infrastructure and services for clients across Canada. Mr. Hemingway's areas of specialization include conceptual planning, feasibility studies, travel demand forecasting, trend analysis, economic assessment and business case analysis, traffic operations and safety studies, traffic impact assessment and traffic management plan development. He is a licensed Professional Engineer and is certified as a Professional Traffic Operations Engineer by the Transportation Professional Certification Board of the Institute of Transportation Engineers. Mr. Hemingway has provided expert witness testimony before tribunals, including the Ontario Municipal Board (OMB) and provides peer review services on request.

Michael Lindsay

Global Director, Infrastructure Planning & Advisory, Hatch

Michael Lindsay is the Global Director of Infrastructure Planning & Advisory for Hatch. He leads a global team supporting a set of public and private clients pursuing major infrastructure investment programs and projects. His team integrates technical expertise (“what do we build”) with planning expertise (“where do we build”) and financial expertise (“what funding/delivery model do we use”) to help create viable and valuable infrastructure projects.

Before joining Hatch, Mr. Lindsay was Senior Vice President of Commercial Projects at Infrastructure Ontario. In that role, he was responsible for commercial transactions related to the financing, development, and divestiture of public assets in Ontario (e.g. land, buildings, services, etc.). His team investigated public-private partnerships in connection to critical new infrastructure (e.g. energy, transit, social) and contracted private entities to deliver government services through alternative service delivery (ASD) arrangements. Prior to this, Mr. Lindsay was an Associate Principal with McKinsey & Company, where he was a core leader of the Canadian Public Sector Practice.



Harris Centre Regional Analytics Lab

***Preliminary Regional Economic Impact of a Fixed Link
Between Labrador and the Island of Newfoundland: An
Examination of Transportations Costs and Subsequent
Impacts on Regional Economies***

**Submitted by:
Alvin Simms PhD
Jamie Ward MSc**

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INTRODUCTION

This report presents the outcomes from the analysis of the impact of a proposed Fixed Link between Labrador and the Island of Newfoundland. The report examines the overall impact of shifting a portion of passenger traffic (tourist and non-tourist) as well as diverting a percentage of the commercial truck traffic from the Channel-Port aux Basques ferry to the Fixed Link proposed for the Labrador Straits. In general, these impact metrics are presented in terms of:

1. Estimated changes in travel times and costs related to interprovincial commodity flows and their potential impact on the economy;
2. Estimates of potential changes to commodity prices and production costs by domestic industries due to changes in transportation costs;
3. The impact on the costs of imports and exports associated with a change in the mode of transport, and the impact on the general economy; and
4. An estimate of the economic impact of diverting a portion of the passenger and commercial truck traffic from the Marine Atlantic ferry service to the Fixed Link. Note that this scenario assumes that the Marine Atlantic Gulf ferry remains in service.

The first 3 items in the preceding list can be addressed with an adaptive transportation cost model that is generally used to generate the optimal or least-cost outcomes by various modes of transport. For this report, the comparison is the Marine Atlantic ferry versus the Fixed Link. These models require inputs such as the per-unit cost for truck shipments (TEUs¹) required from suppliers to meet a demand or the number of TEUs required by each destination. The estimated economic impact of the diversion is calculated from a geo-spatial simplex supply chain model that is used to construct sub-regional Leontief input-output matrices and multipliers that can be used to measure the impact of diverting traffic away from Channel-Port aux Basques to the Fixed Link in the Labrador Straits. Local multipliers (e.g., \$1 spent in one industry generates another \$1.55 throughout the local economy) are required because the diversion does not result in an economic loss provincially but a re-distribution of a percentage of the economy from the Southwestern part of the province to other regions in Labrador and the Northern Peninsula. For example, because there is “no loss” of economic benefit from the provincial economy, the provincial level input-output table multipliers related to the industries affected will not change but instead the multipliers will change at the local or sub-regional level. Thus, calculating the economic impacts associated with diverting traffic from Channel-Port aux Basques requires local rather than provincial multipliers [see endnotes 2, 3 and 4].

¹ TEU or twenty-foot equivalency unit is used to identify the carrying capacity for a specific mode of transport. For example, how many trucks can be loaded on a train, ship or in terms of traffic, how many TEUs can travel through a highway tunnel in one hour?

The report focuses on 3 outcomes from the analysis:

1. Cost differences related to transport cost model analysis.
2. Intra-provincial re-distribution of value associated with relevant sectors of the economy due to diversion of a percentage of the traffic from Marine Atlantic to the Fixed Link.
3. Identification of specific commodities and industries that will be impacted in terms of net gains and losses.

ASSUMPTIONS AND DATA

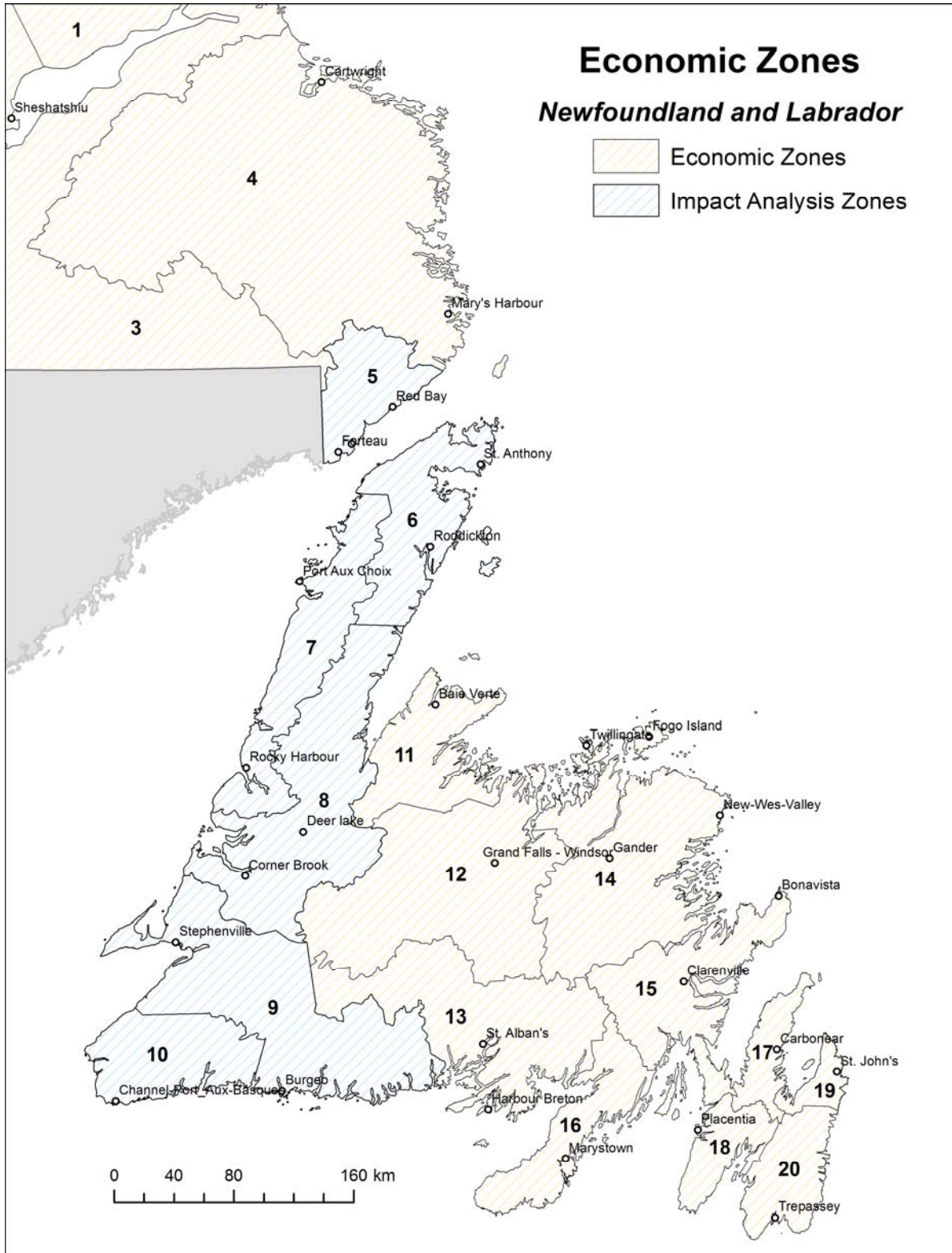
The assumptions for the regional economic impact analysis related to diverting traffic from the Marine Atlantic Gulf ferry service to the proposed Fixed Link in the Strait of Belle Isle are based on terms of reference for the project and on model constraints for estimating commercial transportation impacts (e.g., dollars, travel time), as well as on the redistribution of passenger vehicle traffic. Note that the geography used for the regional impact analysis is the former Economic Zones because the NL Department of Tourism exit surveys that provide information on areas of the province visited by non-resident travelers who stayed at least one night is reported by Economic Zone. These results are required to calculate the portion of the economy dependent on travelers that enter the province via the Marine Atlantic ferry service to Channel-Port aux Basques (Economic Zone 10). Preliminary analysis indicated that redistribution of these travelers and subsequent impacts were more likely to occur in Economic Zones 5 to 10 (see highlighted rows in Table 1 and highlighted regions in Figure 1).

Table 2 List of Newfoundland and Labrador Economic Zones

| Economic Zones |
|---|
| Zone 1: Rigolet to Nain |
| Zone 2: Labrador West/Churchill Falls |
| Zone 3: Happy Valley/Goose Bay/Northwest River |
| Zone 4: Mary's Harbour to Cartwright |
| Zone 5: Labrador Straits (L'Anse Au Clair to Red Bay) |
| Zone 6: Viking Trail, St. Anthony South West to Plum Point, East to Roddington/Englee |
| Zone 7: Gros Morne Area, Viking Trail North to and including Plum Point |
| Zone 8: Deer Lake/Humber Area Corner Brook/Massey Drive |
| Zone 9: Stephenville/Port-au-Port/Burgeo |
| Zone 10: Port-aux-Basques/Doyles/Rose Blanche |
| Zone 11: Baie Verte/La Scie/Green Bay |
| Zone 12: Grand Falls Windsor Area |
| Zone 13: Bay D'Espoir Area |
| Zone 14: Gander/Twillingate East to Terra Nova |
| Zone 15: Clarenville/Bonavista Peninsula Area |
| Zone 16: Burin Peninsula |
| Zone 17: North West Avalon |
| Zone 18: Argentia/Placentia Area |
| Zone 19: North East Avalon and St. John's CMA (including Mount Pearl) |
| Zone 20: Southern Shore Area |

Source: 2011 Exit Survey – Program Highlights, Government of Newfoundland and Labrador

Figure 1 Baseline Geography for Impact Analysis: Economic Zones



Source: Newfoundland & Labrador Statistics Agency

For the regional analysis, the assumptions of diverting traffic from Marine Atlantic gulf ferry service to the Fixed Link are as follows:

1. There is no change to the Marine Atlantic Gulf ferry service.
2. This analysis is preliminary and outcomes should be viewed as potential rather than actual. A full impact analysis would require more time and effort, plus additional data on specific construction costs as well as a survey of local businesses in all areas of impact.
3. Economic Zone 10 (Channel-Port aux Basques) is the only region that will be negatively impacted by diversion of travelers.
4. The level of intra-provincial trade (trucking of commodities) is dependent on the size of the population.
5. The impact of intermediate goods (trucking) is constrained to transportation margins (i.e., the % of total expenditures spent on truck transportation).
6. Any changes in travel costs are passed on to the consumer as final demand.
7. Tourism expenditures (on restaurants, accommodations etc.) are simply transferred north (e.g., to Economic Zones 5, 6 and 7) with no changes to local multipliers.
8. Before and after scenarios are relevant for 1 year. (Future impacts are complicated by induced traffic through the tunnel).
9. Before and after scenarios for tourism impacts are based on Hatch's estimated 2.5% annual growth rate in traffic over 42 years.
10. A time series analysis of traffic trends indicates that on average, 60% of the annual traffic could be potentially diverted from the Marine Atlantic ferry service to the Fixed Link. (This relates to all traffic and not just to peak traffic).
11. Diverted commercial truck traffic breakdown is 40% tractor-trailer (4 TEUs) and 60% smaller commercial trucks (2 TEUs). TEU equivalency conversions are based on Hatch's assumptions.
12. The Quebec North Shore Highway 138 link is completed to an acceptable standard and travel times are based on Hatch's 2004 estimates.
13. The travel cost by mode of transport is based on two scenarios: (1) the cost of traveling through the Fixed Link tunnel is the same as traveling on the Marine Atlantic ferry; and (2) the cost of the tunnel passage is equivalent to PEI's Confederation Bridge fee structure.
14. Non-resident expenditure industry distribution from the input-output table is equal to the total non-resident expenditure industry distribution from the NL Tourism survey.

15. All of the commercial trucking and tourism traffic from the Maritime Provinces continues to use the Marine Atlantic Gulf Ferry service (i.e., there is 0% diversion to the Fixed Link).
16. For this analysis, the indirect effects (i.e., industries that buy from or sell to other local businesses) are spin-offs that are restricted to the Channel-Port aux Basques region (Economic Zone 10).

For the trucking cost model, it is important to differentiate the cost of operating a transport truck while stationary on a ferry crossing versus the continuous driving associated with travel to the Fixed Link tunnel. Although the average travel time associated with the Fixed Link (23.5 hrs) is less than that for the ferry route (26.2 hrs) there is a greater portion of the Marine Atlantic ferry route where the truck is stationary and therefore less expensive to operate. Thus, for a portion of the Fixed Link route, the truck is slightly more expensive to operate and this will be accounted for in the transport cost model.

Also, due to provincial regulations in Canada, truck drivers are limited in the number of hours they are permitted to drive. For example, in Ontario and Quebec, drivers can be on duty for 14 hours or drive continuously for 13 hours. This regulation has the effect of lengthening, for long haul trips, the driving time to both the ferry and the Fixed Link through either driver rest stops or switchovers. However, the travel time and costs are somewhat consistent between the two routes and these factors are included in the various transport cost models below.

For this analysis, data were provided directly by Provincial Government agencies or downloaded from Provincial or Federal Government data portals. For example, the Government of Newfoundland and Labrador's Department of Tourism, Culture, Industry and Innovation, as well as the Department of Transportation and Works provided data on tourism exit surveys and marine traffic, respectively. Population data (2016 census) for the Economic Zones was downloaded from NL Statistics Agency's Community Accounts. Input-output (IO) industry transaction data related to tourism "expenditure product breakdown" from provincial IO tables were downloaded from Statistics Canada IO Table(s) web site. Commercial trucking rates, crossing and waiting times for Marine Atlantic and commercial rates for Confederation Bridge are sourced from their respective web pages. Data on operational trucking costs that impact the cost of shipping (e.g., wages, fuel efficiency and required break time for drivers) were acquired through the OECD, and through Provincial and Federal Government data portals. Table 2 provides a list of data sources and where relevant a web portal link.

Table 3 Data Sources and Web Portal Linkages

| DataSource | Analysis | Link |
|---|--|---|
| Input-Output Table (CANSIM 381-0033) | Tourism (expenditure product breakdown) | http://www5.statcan.gc.ca/cansim/a26?lang=eng&retrLang=eng&id=3810033&tabMode=dataTable&p1=-1&p2=9&srchLan=-1 |
| NL Tourism Exit Surveys Department of Tourism | Tourism (baseline tourism expenditure/traffic numbers by province) | Government of NL Department of Tourism |
| Trucking Commodity Origin Destination Survey - Statistics Canada | Trucking (checking result quality) | http://www23.statcan.gc.ca/imdb/p2SV.pl?Function=getSurvey&DDS=2741 |
| Marine Atlantic | Trucking (commercial shipments over time) | Government of NL Department of Transportation |
| Marine Atlantic website | Commercial trucking rates, crossing & waiting times | https://www.marineatlantic.ca/ |
| Confederation bridge website | Commercial trucking rates | http://www.confederationbridge.com/tolls-fees/tolls-fees.html |
| Ontario ministry of transportation | Required trucking break times | http://www.mto.gov.on.ca/english/trucks/handbook/section1-7-4.shtml |
| Societe de l'assurance automobile Quebec | Required trucking break times | https://saag.gouv.qc.ca/en/transportation-goods/driver/driving-and-off-duty-time/ |
| Transport Canada Economic Analysis Directorate - Estimation of Costs of Heavy Vehicle Use Per Vehicle Kilometre In Canada | Trucking cost model | http://www.bv.transports.gouv.qc.ca/mono/0965385.pdf |
| OECD | Trucking cost model updates (wage burden) | http://www.oecd.org/ctp/tax-policy/taxing-wages-20725124.htm |
| Statistics Canada (CANSIM 281-0030) | Trucking cost model updates (average wage) | https://www.statcan.gc.ca/tables-tableaux/sum-som/l01/cst01/labr74a-eng.htm |
| NRCan | Trucking cost model updates (fuel efficiency) | http://www.nrcan.gc.ca/energy/efficiency/transportation/commercial-vehicles/reports/7607 |
| NL Statistics Agency Community Accounts (Population by Economic Zone) | Trucking (distribute cost savings throughout island) | http://nl.communityaccounts.ca/mvrc.asp? |

METHODS

RANLab's geo-spatial simplex supply chain and transportation cost linkage model generates a value chain industry-to-industry linkage database by Census Subdivision for the entire provincial economy. It requires the provincial input-output table to initialize the model and establish industry linkages. The input-output tables are best described as an industry transaction table that accounts for the level of sales between industries whereby industries sell commodities and intermediate goods to produce final products that are sold to consumers. The input-output tables can be divided into 4 components:

1. **Industry production relationships (transactions)** are used to identify the linkages between raw materials and intermediate goods and how they are combined and linked to other industries for the production of goods for other industries and consumers
2. **Final Demand and Market Behaviour** are used to identify the consumer, private and government purchasing patterns for the final industry outputs. The export section of the final demand table identifies the external demand for outputs.
3. **Incomes or Value-Added** are used to identify the incomes of various sectors in the economy that includes salaries, industry earnings, and taxes paid. This number is represented as a value-added row in the table. This part of the table also includes payments to external industries (e.g. other regions within the province as well as other provinces or countries) for materials and intermediate goods (e.g., final payments)
4. **Non-Market Transfers** are used to identify government subsidies, taxes and inter-government transfers as well as final demand purchases by external industries.

The four sections of the input-output table are used to identify the income linkages of an economy where the final demand table identifies who consumes the finished goods. The income and value-added section identifies who receives payments for their contribution in the production process while the industry production relationships or transaction table identifies the inter-industry linkages or the market sector of the economy. The non-market transfers identify the government and individual monetary transactions in terms of taxes, labour costs, surpluses, deficits etc.

By itself the input-output table describes rather than analyzes the economy at a particular point in time. However, integrating additional transportation information such as transportation cost matrices (distance/time/dollar costs) and commodity values as well as volume of shipments permits the calculation of the impact of these inputs on the economy as well as on different industries. In addition, including the transportation model also permits an assessment of the likelihood that a particular industry located somewhere in the province can supply inputs to another industry in another part of the province. Thus, for a particular sector one can not only estimate the impact of that enterprise on an economy but the likelihood it can get its required inputs within a region.

The economic spillover effects incurred when adding or removing a portion of industrial activity from the economy are derived by generating local input-output tables from the geo-spatial supply chain linkage model and these tables are used to estimate the regional economic impact of diverting traffic from the Channel-Port aux Basques region. These local input-output tables are used to calculate Leontief multipliers that are used in the impact analysis [4, 5].

Note that the integration of Statistics Canada Business Registry data within the geo-spatial simplex supply chain model is used in the calculation of localized input-output tables as well as local multipliers. The business registry geography is at the census sub-division or municipal level and for this study the outcomes are aggregated to the Economic Zone geography. These supply chain outcomes are used to profile and assess economic impacts of changes in the “local/sub-regional” economies related to the diversion of a portion of the traffic from the Marine Atlantic Gulf ferry service to the Fixed Link.

The re-distribution of traffic (e.g., tourist and non-tourist) and economic spin-offs from the Channel-Port aux Basques area to the Fixed Link location at the Labrador Straits and parts of the Northern Peninsula is estimated with an economic based gravity model [2, 3, 4] where the economic redistribution varies with the magnitude of economic activity (e.g., population, GDP etc.) at the origin and destination and inversely related to the distance between places i and j where:

$$RDE_{ij} = \emptyset \frac{Y_i^{B_1} Z_j^{B_2}}{Dist_{ij}^{B_3}}$$

The original gravity model can be converted to an exponential regression equation where:

$$RDE_{ij} = EXP [B_0 + B_1 \ln(Y_i) + B_2 \ln(Z_j) - B_3 \ln(Dist_{ij})]$$

RDE_{ij} = Redistribution of diverted economic activity from Channel-Port aux Basques to Economic Zones 5, 6, 7, 8, 9. In this instance the origin is the Fixed Link location while the destinations are in Economic Zones 5 to 9

\emptyset = gravity constant = B_0 intercept for exponential regression equation

B_1, B_2, B_3 slope values for regression equation

$Y_i^{B_1}$ = indicator for level of economic activity to be diverted (origin)

$Z_j^{B_2}$ = indicator for level of economic activity at destination

$Dist_{ij}$ = distance between origin and destination

\ln = natural log

The steps required to complete the analysis are:

1. Data preparation:

- a) Define transport dollar costs associated with ferry and Fixed Link tunnel routes.
- b) Define transport time cost associated with ferry and Fixed Link tunnel routes.

- c) Recalibrate geo-spatial simplex supply chain model to use origin and destination time factors.
 - d) Aggregate tourism exit survey data by Economic Zone and reason for visit.
 - e) Estimate commodity mode of shipping breakdown by Oceanex versus Marine Atlantic.
- 2. Run the geo-spatial simplex supply chain model:**
- a) Generate trade point scenario to calculate local input-output tables and multipliers for the existing ferry system.
 - b) Generate trade point scenario with 60% of traffic diverted from Marine Atlantic to the Fixed Link.
 - c) Run two scenarios, both modes of transport, with tourist-only and total traffic.
- 3. Evaluate the tourism impact from final demand allocations in step 2:**
- a) Remove intervening opportunity tourists (plus their expenditure) from the Channel-Port aux Basques region.
 - b) Add “intervening opportunity tourists”² to the Labrador Straits and the Northern Peninsula.
 - c) Convert the diverted tourism expenditure to an estimated number of jobs.

The final step in the analysis is to organize outputs for interpretation and presentation.

TRUCKING COST MODEL BACKGROUND

Transportation costs have a significant impact on regional economies in that they influence the cost of intermediate goods used in production (imports), and in many cases, transport cost margins determine whether or not a particular industry is competitive in a marketplace. For example, evidence suggests that a 10% increase in transport costs can reduce trade by 20% [15].

The first step required to estimate the impacts of transportation cost changes related to diverting tractor trailer traffic from Marine Atlantic ferry service to the Fixed Link is to estimate the proportion of existing Marine Atlantic traffic originating from each province using the Hatch TEU data and the Trucking Commodity Origin Destination (TCOD) summary file. Statistics Canada’s 2007 to 2011 Truck Commodity Origin Destination (TCOD) surveys are utilized to examine trucking and commodity shipment patterns and volumes from other provinces as well as from the USA/Mexico to Newfoundland and Labrador. The TCOD survey purpose is:

“to measure the commodity movement and outputs of Canadian trucking. The estimates produced include total tonnage transported by commodity type, and revenues by origin and destination of the shipments [13].”

² “Intervening opportunity tourists” are those tourists that spend at least one night in Channel-Port aux Basques region after leaving the ferry.

The TCOD data indicates that for Newfoundland and Labrador, approximately 40% of truck transport revenues for this province are associated with traffic using the Marine Atlantic Gulf ferry service. Temporal analysis of the 2007 to 2011 TCOD data indicates that approximately 74% of the commodities shipped by truck through Marine Atlantic originated predominately from Quebec and Ontario and the western provinces including USA and Mexico, whereas 26% of the shipments originate in the Maritime Provinces. An examination of the TCOD data suggests that 81% of the shipments originating outside of Maritime Provinces or an estimated 60% of the total would potentially be diverted to the Fixed Link tunnel route (see Table 3 for detailed listing by province). This is hypothetically associated with existing truck companies that use locations in New Brunswick or Nova Scotia as transshipment points where commodities are redistributed via truck throughout the Maritimes and via Marine Atlantic to Newfoundland and Labrador.

Table 4 Percent of Total Shipments by Truck to Newfoundland & Labrador

| Province | % Total Shipment by Province | % Diverted to Fixed Link |
|----------|------------------------------|--------------------------|
| PE | 0.22% | 0.00% |
| NS | 12.03% | 0.00% |
| NB | 14.08% | 0.00% |
| QC | 19.99% | 16.20% |
| ON | 36.62% | 29.66% |
| MB | 0.33% | 0.27% |
| SK | 0.07% | 0.06% |
| AB | 1.05% | 0.85% |
| BC | 0.43% | 0.35% |
| YT | 0.00% | 0.00% |
| NT | 0.01% | 0.01% |
| USMEX | 15.16% | 12.28% |

After the proportional values (Table 3) are assigned by province these values are used in the trucking cost model to estimate the change in costs associated with diverting the TEUs from Channel-Port aux Basques to St. Barbe area (e.g., the tunnel exit point) or the cost of trucking commodities to the Island portion of the province. To estimate cost, the model requires the definition of a truck operating profile. Table 4(A) presents an operating profile of annual “heavy use truck” cost factors [14] while Table 4(B) provides updated cost values used as constants in the model.

Table 4(A) includes the breakdown of the truck operating costs. Importantly, it also provides insight to driving versus non-driving hours. These inputs are used to construct per tonne/kilometer as well as hourly operational costs that can be used to differentiate between continuous driving and completing the required rest times (non-driving) over a long-haul trip, versus those costs associated with driving to the ferry in North Sydney and accounting for when the truck is stationary (non-driving) during the ferry crossing to Channel-Port aux Basques. The values in Table 4(A) are based on a 2006 study [14] while the information in Table 4(B) includes constants in the cost model along with updates for wages, NRCan fuel

consumption in L/100 km, wage burden update and recent fuel prices (e.g., Montreal average price for the week of Nov. 1, 2017), etc.

Table 5 Truck Operating Cost Profile (A) and Model Constants (B)

| (A) Truck Operating Cost Profile | Value | (B) Truck Cost Model Constants | Value |
|---|--------------|---------------------------------------|----------------|
| Annual Distance Travelled (km) | 300,000 | Avg Trip Dist (km) | 1,000 |
| Avg Trip Driving Time (Hours) | 11.11 | # Trips per Day | 1.00 |
| Avg Daily Driving Time (Hours) | 11.11 | # Days per Year Working | 300 |
| Avg Annual Driving Time (Hours) | 3,333.3 | Avg Speed (km/hr.) | 90 |
| Avg Non Driving_Day (Hours) | 2.89 | Avg Non Driving/Trip (Hours) | 2.89 |
| Avg Non Driving_Annual (Hours) | 866.7 | Vehicle RGVM (kg) | 54,000 |
| Total Annual Hours | 4,200 | Vehicle Tare Weight | 14,340 |
| Per Vehicle Annual Tonne-km | 3,135,600 | Payload (kg) | 16,000 |
| Per Vehicle Annual Tonne-Hours | 34,840 | % Loaded (TCOD) | 65.33 |
| Annual Fuel Consumption | \$118,500 | NRCan Fuel Consumption (L/100 km) | 39.50 |
| Non-Driving Paid Hourly | \$22,031 | % Driven_Pavement | 100 |
| Driving Pay | \$84,733 | % Driven_Gravel | 0.00 |
| Wage Burden | \$33,524 | Hourly Wage (CANSIM 281-0300) | \$25.42 |
| Interest Financing Equipment | \$13,271 | Per km \$Pay | \$0.28 |
| Insurance Costs | \$3,636 | Pay Binary (Hourly = 0, km = 1) | 0 |
| Administrative Costs | \$18,180 | % Wage Burden (OECD taxing Wages) | 31.40 |
| Operator Margin | \$9,090 | Fuel Price (\$/Litre) | \$1.10 |
| | | Repair & Tire Costs (\$/km) | \$0.27 |

Table 5 presents the updated annual operating costs using the data in Tables 4(A) and 4(B). For example, note that in the annual operating cost part of the table, the total fuel cost is updated to \$129,758 from \$118,500 (Table 4(A)) presented in the 2006 study [14]. These values are subsequently converted to tonne per kilometer and per-hour cost (Table 5). These variants on transport costs can account for loads as well as distances in a cost model. Finally, the cost per hour and tonne/kilometre can be converted to TEU equivalencies, using Hatch’s TEU conversion formula. In this case, the final truck operating costs are presented in Table 6 where the cost per hour of operation is estimated to be \$127.58³ and cost per hour of waiting or resting time is \$64.35. The TEU equivalents are \$45.56 per hour of operation and \$22.98 per hour of waiting or when the truck is stationary for a rest period on a ferry/tunnel crossing.

³ For example, hourly truck operating costs are based on “the annual cost of operating of operating a truck (sum of annual operating cost in Table 4)/annual average driving time in Table 3”.

Table 6 Annual Operating Costs Conversion to Tonne per Kilometre and Hour

| Annual Operating Cost | Value |
|---|-------------------------|
| Total Driver | \$140,288 |
| Total Fuel | \$129,758 |
| Total Repair & Tire | \$81,007 |
| Annual Licence Fee | \$2,586 |
| Annual Vehicle Ownership (Depreciation) | \$27,436 |
| Annual Overhead | \$44,177 |
| Tonne per Kilometre Costs | |
| | Tonne*km Value |
| Total Driver | \$0.045 |
| Total Fuel | \$0.041 |
| Total Repair & Tire | \$0.026 |
| Annual Licence Fee | \$0.001 |
| Annual Vehicle Ownership (Depreciation) | \$0.009 |
| Annual Overhead | \$0.014 |
| Total | \$0.136 |
| Tonne per Hour Costs | |
| | Tonne*Hour Value |
| Total Driver | \$4.03 |
| Total Fuel | \$3.72 |
| Total Repair & Tire | \$2.33 |
| Annual Licence Fee | \$0.07 |
| Annual Vehicle Ownership (Depreciation) | \$0.79 |
| Annual Overhead | \$1.27 |

Table 7 Estimating Final Operating and Waiting Cost per Hour with TEU Conversions

| Final Truck Operating Costs per Hour | |
|---|----------|
| Cost per Truck*Hour Operation | \$127.58 |
| Cost per Truck*Hour Waiting | \$64.35 |
| Truck/TEU Conversion | |
| | 2.8 |
| Cost per TEU*Hour Operation | |
| | \$45.56 |
| Cost per TEU*Hour Waiting | |
| | \$22.98 |

To test the assumptions and the truck cost parameterizing process, a sensitivity analysis was performed to determine if the transportation cost model and its estimates were comparable to known transportation revenues generated by trucking commodities via the Marine Atlantic ferry to Newfoundland and Labrador. In this case Statistics Canada TCOD⁴ estimates for revenues generated by trucking of commodities from 2007 to 2011 are used to evaluate the outputs from the transportation cost model.

⁴ Statistics Canada Trucking Commodity Origin Destination (TCOD) records revenues generated by trucking. This is the cost of transporting commodities between provinces and at this time is the best estimate available.

The TCOD costs per tonne per kilometre over the 5-year period ranged from \$0.123 to \$0.131 while the calculated value for the transport model was \$0.136 or a 3.82% difference. Analysis of the TCOD 2011 survey indicated that \$124.27 million in revenues was generated by shipping commodities via truck and the Marine Atlantic gulf service to Newfoundland and Labrador. The comprehensive trucking cost model estimated trucking revenues of \$126.69 million, or a 1.94% difference. Thus, any transport cost estimates when comparing the cost of trucking via the ferry versus the Fixed Link are $\pm 1.94\%$. This variation is only valid within the context of the TCOD values, the transportation cost model and the assumptions used in this study. However, the TCOD values validate the accuracy of the RANLab model.

Marine Atlantic versus Fixed Link Route Travel Time Outcomes

On average, the Marine Atlantic gulf ferry crossing is 7 hours duration and accounts for 50% of the goods being shipped to and from the Island portion of the province⁵. The assessment of diverting 60% of the truck traffic from the Marine Atlantic gulf service to the Fixed Link tunnel is presented as: (1) time savings relating to driving along a completed HWY 138 to Blanc-Sablon and the fixed tunnel exit in the St. Barbe Area and (2) cost savings in truck cost revenues.

The time savings calculation is based on the distance from the largest centre in each province to the Fixed Link tunnel exit on the Island side, and for Marine Atlantic it is the distance to North Sydney (Nova Scotia) and the loading plus travel time associated with the ferry crossing to Channel-Port aux Basques. For the USA/Mexico originating trucks, the border crossing south of Montreal was selected as the start location. The average for the driving calculations is 90 km/hr and the driving time also includes rest time required for all long-haul trips. For example, federal and provincial regulations⁶ in Canada have limits on the number of hours a driver can operate a truck and the time shall not exceed 13 hours and this may be reduced in extreme driving conditions and in some cases the drivers are required to not drive for more than 10 hours per day or have at least 8 hours of rest. Note the 13-hour period includes loading time if required, waiting time in heavy traffic, refueling etc. Thus, on average the actual driving time may not exceed 10 or 11 hours. This is supported by the 2006 study [14] where the average daily driving time is 11.1 hours (see Table 4(A) "Avg. Daily Driving Time"). A general solution to these regulations is to switch drivers at pre-determined locations along the long-haul route or to have a second driver on the trip but this option increases operation costs. The switch driver option also results in some non-driving time for refueling, checking the truck, etc.

Table 7 presents the calculated total travel times for both the ferry and Fixed Link routes. The calculated time does not include travel on the Island portion of the province. For example, the distance from Channel-Port aux Basques to Deer Lake is 265 km while the distance from the tunnel exit in St. Barbe area

⁵ Source: <https://www.marineatlantic.ca/en/commercial/Commercial-Traffic/>

⁶ Source: <http://laws-lois.justice.gc.ca/eng/regulations/SOR-2005-313/>

to Deer Lake is 298 km; the 33 km difference in travel distance is not a significant factor in the overall transport costs. Using the existing road network and Marine Atlantic ferry route, the average travel time from the Maritime Provinces to Channel-Port aux Basques is 14.4 hours. However, if the Quebec HWY 138 and Fixed Link route is used, it increases the average travel time to 30.5 hours or more than twice the Marine Atlantic route including the ferry crossing time (Table 7). There is no comparative advantage for truck shipments originating in the Maritimes to use the alternative Fixed Link route.

Table 8 Calculated Travel Times for the Marine Atlantic Gulf Ferry and Fixed Link Tunnel

| Province | Total Travel Time Ferry | Total Travel Time Tunnel | Tunnel Time Addition Factor |
|----------|-------------------------|--------------------------|-----------------------------|
| PE | 15.22 | 31.11 | 15.89 |
| NS | 13.72 | 32.11 | 18.39 |
| NB | 14.27 | 27.22 | 12.96 |
| QC | 26.42 | 21.51 | -4.92 |
| ON | 31.94 | 27.02 | -4.92 |
| MB | 56.17 | 51.25 | -4.92 |
| SK | 67.06 | 62.14 | -4.92 |
| AB | 72.06 | 67.14 | -4.92 |
| BC | 84.94 | 80.03 | -4.92 |
| YT | 99.83 | 94.92 | -4.92 |
| NT | 88.94 | 86.92 | -4.92 |
| USMEX | 27.42 | 22.51 | -4.92 |

*Note the NT time savings is 2.0 hours (there’s a mathematical trip break line at 66 .66 hours which is in between 65 and 68.25 hours) (trip length = 11.11 hours above)

The average travel time from Ontario and Quebec is 29.18 hours and for the Western provinces the average is 70.06 hours. However, when factors associated with “long-haul trips”, described above, are accounted for the travel time for the Fixed Link route is on average 4.9 hours less than the road travel and ferry crossing combined.

Marine Atlantic Route versus Fixed Link Trucking Revenue Outcomes

The transportation cost model is used to estimate the cost differences between diverting 60% of the commercial truck traffic from the Marine Atlantic route to the Fixed Link tunnel route in the Labrador Straits. The cost model is run with the following assumptions:

1. Two Fixed Link toll scenarios where (a) the toll for the tunnel passage is equivalent to the Marine Atlantic commercial truck fee structure (e.g., for a 75’ total length truck, the fee would be \$582.94) and (2) the toll for the tunnel passage is equivalent to the PEI Confederation Bridge fee structure (e.g., \$42.50 for the first 2 axles plus \$8.00 for each additional axle). In essence, the

Confederation Bridge fee structure is lower than Marine Atlantic's, which is not surprising given the latter's much longer travel distance.

2. There are two trucking driver models for long haul trips: (1) where the driver takes the required rest period after 13 hours of driving as per regulations, and (2) where the driver is switched or replaced at predetermined locations after driving the requisite time.

Note that in Table 6, the estimated per-hour cost of operating a truck is \$127.58 and in waiting mode, when the driver is stopped or resting, it is \$64.35. This cost breakdown affects both the Marine ferry crossing where the driver is resting/waiting for 7 to 9 hours during the crossing as well as on the long-haul trip rest times that would be required on the Highway 138 Fixed Link route. For example, the distance from Montreal to North Sydney is 1,429 km while the distance from Montreal to St. Barbe tunnel exit is 1,692 km⁷. Under the driving time regulations both trips are considered long-haul except that on the ferry route there would be more "rest/waiting time" because of the ferry crossing, thus the waiting rate would be applied in this instance.

Table 8 presents the outcomes from the transportation cost model that estimates the cost savings associated with diverting 60% of the commercial truck traffic from the Marine Atlantic gulf ferry service to the Fixed Link tunnel route. Results are presented within the context of two different fee structures as well as trucker driver rest and switch models for long haul trips.

The estimated truck revenue generated by 60% of the traffic using the Marine Atlantic service is approximately \$145.1 million and if this volume of traffic was diverted to the Fixed Link route, using the ferry fee cost structure for the tunnel passage, the savings would be approximately \$3.25 million, or a 2.24% drop in transport costs. For this study, it is assumed that the cost of transportation associated with the revenues is redirected to the final demand part of the input-output table (i.e., the cost is passed on to the consumer). In this case, the savings per capita is \$6.59. When comparing the savings for the whole economy where the \$3.25 million reduction in transport cost is applied to the Statistics Canada 2011 TCOD Newfoundland and Labrador total trucking revenues of \$391.3 million, the overall impact is a cost decline of 0.83%.

When the Confederation Bridge fee structure is used in the model, the diverted transport cost reduction is \$13.1 million or a 9.24% saving. Thus, the final demand or per capita consumer cost saving is increased to \$27.22 but in terms of overall consumer purchasing savings. Therefore, differences in the fee structure account for a 7% decrease in transportation costs. At the provincial level the overall diverted savings increase from 0.83% to 3.43% when using the Confederation Bridge fee structure (Table 8).

⁷ Calculated from Google Earth and Hatch's 2004 distance calculations for the Fixed Link tunnel

Table 9 Trucking Commodity Shipment Revenue Estimates and Fixed Link Diversion Savings for Long Haul Rest Stop Model

| Description | Ferry Fee Cost Option (Long Haul Model With Rest Stops) | Confederation Bridge Fee Cost Option (Long Haul Model With Rest Stops) |
|---|--|---|
| Initial estimated truck revenues for diverted truck traffic using existing ferry route | \$145,055,252 | \$145,055,252 |
| Diverted savings (\$) | \$3,246,510 | \$13,055,252 |
| Diverted savings (%) | 2.24% | 9.24% |
| Initial Truck transport revenues/costs added to final demand and expressed as per capita or consumer cost | \$295 | \$295 |
| Diverted savings per capita | \$6.59 | \$27.22 |
| Statistics Canada TCOD 2011 total trucking revenue for shipping commodities to NL | \$391,329,087 | \$391,329,087 |
| Overall diverted savings (%) | 0.83% | 3.43% |

The results presented in Table 8 are based on both the Marine Atlantic ferry and Confederation Bridge fee structure with a single driver using the mandatory rest period after 13 hours of driving. To determine if there is a significant difference between switching drivers at predetermined locations after reaching maximum daily driving time versus a single driver with an extended rest period (Table 8 outcomes), the model was run with the “switching driver option”. Table 9 presents the results of this analysis and there is a negligible cost difference between the two models. For example, for the ferry fee structure the diverted savings was 2.36% versus the 2.24% with the rest stop model (Table 8). Likewise, the results were similar to the Confederation Bridge fee result with a 9.72% saving for the switch model versus 9.24% for the rest stop model. When comparing the per capita and total provincial TCOD revenue there are no differences (Tables 8 and 9).

Table 10 Trucking Commodity Shipment Revenues Estimates and Fixed Link Diversion Savings for Long Haul Switch Driver Model

| Description | Ferry Fee Cost Option (Long Haul Model With Switch Driver) | Confederaton Bridge Fee Cost Option (Long Haul Model With Switch Driver) |
|---|---|---|
| Initial estimated truck revenues for diverted truck traffic using existing ferry route | \$137,847,328 | \$137,847,328 |
| Diverted savings (\$) | \$3,246,986 | \$13,404,381 |
| Diverted savings (%) | 2.36% | 9.72% |
| Initial Truck transport revenues/costs added to final demand and expressed as per captia or consumer cost | \$280 | \$280 |
| Diverted savings per captia | \$6.59 | \$27.22 |
| Statistics Canada TCOD 2011 total trucking revenue for shipping commodities to NL | \$391,329,087 | \$391,329,087 |
| Overall diverted savings (%) | 0.83% | 3.43% |

Summary

The amount of estimated savings in transportation cost is very sensitive to the tunnel transit cost. Overall, the estimated savings in transportation costs by diverting traffic from the Marine Atlantic ferry service to the Highway 138 Fixed Link route range between \$3.25 million and \$13.5 million per year, or between \$6.60 and \$27.20 per capita. When referenced in the context of the overall economy, this represents approximately 0.8% to 3.4% of the total trucking revenues on 2011 shipments to the province.

The analysis of the “Fixed Link tunnel option” estimates that it would take approximately double the time to travel to Newfoundland for trips originating in the Maritimes. In the future, this factor could potentially disrupt traditional supply routes whereby locations in the Maritimes that are currently used as transshipment points for the Island portion of the province could be replaced by direct shipments via the Fixed Link route.

REGIONAL ECONOMIC IMPACT OF DIVERTING TOURIST AND OTHER PASSENGER TRAFFIC

The regional economic impact of diverting tourist and non-tourist passenger traffic from Marine Atlantic ferry service to the Fixed Link route is calculated from the Government of Newfoundland and Labrador’s tourist exit surveys from 2011 and 2015, from the Department of Transportation and Works’ ferry traffic database, from Statistics Canada’s 2016 Business Registry database, and from the Leontief local input-output tables and multipliers generated by RAnLab’s geo-spatial simplex survey chain model. The geographical re-distribution of the revenues generated by the diverted passengers is estimated using a GDP economic based gravity model [2, 3 and 4] where the economic redistribution varies with the

magnitude of economic activity at the origin and destination and inversely related to the distance between places i and j (see **Methods** above).

The outcomes from the regional impacts analysis are presented as: (1) overall impact on the Channel-Port aux Basques area (Economic Zone 10), (2) Specific industry impacts (dollars and jobs) in the Channel-Port aux Basques region, and (3) dollar value and jobs diverted northwards to the relevant Economic Zones.

To determine the impacts, the supply-chain model requires two runs. The first run is to profile the Channel-Port aux Basques regional economy before passenger traffic is diverted and the second run of the model removes the portion of traffic that will be diverted from the Marine Atlantic ferry service to the Fixed Link route. The impact of diverting passenger traffic is essentially a comparative analysis of the before and after scenarios.

Before the analysis can be initiated, an estimate of the number of passengers that may be potentially diverted from the Marine Atlantic ferry service as well as their expenditures is required. NL Tourism exit surveys are used to determine what portion of the passengers may be diverted by trip origin. The information in Table 10 is used to determine the portion of passengers and vacationers that will be potentially diverted to the Fixed Link route. Note that 60% of the passengers by car are from Quebec west. This percentage is potentially what is diverted to the Fixed Link route.

Table 11 Marine Atlantic Gulf Ferry Service Passenger Information by Origin and Number of Passengers and Vacationers by Car

| Passenger Origin | Total Number of Passengers by Car Origin | Number of Vacationers by Car | % Passengers by Car |
|-------------------------|---|-------------------------------------|----------------------------|
| Maritimes | 38171 | 20613 | 39.60% |
| Quebec | 4771 | 2577 | 4.95% |
| Ontario | 30537 | 16490 | 31.68% |
| Other Canada | 7634 | 4123 | 7.92% |
| United States | 13360 | 7214 | 13.86% |
| Overseas | 1909 | 1031 | 1.98% |

Source: NL Tourism (Table 2.0)

To estimate an economic impact, the analysis requires an assessment of what portion of the 96,383 passengers arriving by car actually stay in the Channel-Port aux Basques region and this is subsequently used to estimate what spinoffs this cohort has on the regional economy. For this study, the exit survey data that identified passengers that stayed at least one night in Channel-Port aux Basques region was used has an indicator of the potential value that arriving passengers contributed to the regional economy. Table 11 presents a summary of these outcomes as well as the portion of passengers that stayed at least one night in the region. From the exit survey, on an annual basis, 31,035 or 32% of all the passengers stay at least one night (the average is 2.3 nights) with an average nightly expenditure of \$56.74 compared to the \$67.60 spent by the subset of vacationers.

Table 12 Marine Atlantic Ferry Passengers Traveling by Car and Staying at Least One Night in the Channel-Port aux Basques Region

| Passenger Origin | % Passengers Staying at Least 1 Night | Total All Passengers | Number of Vacationers | Avg. Nightly Expenditures All Passengers | Avg. Nightly Expenditures by Vacationers |
|-------------------------|--|-----------------------------|------------------------------|---|---|
| Maritimes | 12.8% | 12291 | 6637 | \$697,337 | \$448,672 |
| Quebec | 1.6% | 1536 | 830 | \$87,167 | \$56,084 |
| Ontario | 10.2% | 9833 | 5310 | \$557,870 | \$358,938 |
| Other Canada | 2.6% | 2458 | 1327 | \$139,467 | \$89,734 |
| United States | 4.5% | 4302 | 2323 | \$244,068 | \$157,035 |
| Overseas | 0.6% | 615 | 332 | \$34,867 | \$22,434 |
| Total | 32.2 | 31035 | 16759 | \$1,760,776 | \$1,132,898 |

The portion of “staying passengers” and expenditures from Table 11 are used to calibrate the impact analysis model in order to estimate the regional impacts of passengers that stay at least one night in the Channel-Port aux Basques region.

Table 12 contains the estimates from the before and after impact scenarios that the “stay at least one night” passengers have on the regional economy. The outcomes are for direct plus indirect effects. Direct effects are related to the number of jobs or value created by a business to meet a particular demand, while the indirect effects are related to businesses purchasing/selling supplies from other businesses based on spinoffs from direct effects. The results presented in Table 12 are based on vacationers only staying⁸ (17.3%) versus all passengers staying (32%), which are used conceptually as low and high estimates.

Table 13 Total Value of Direct and Indirect Effects of Passengers Staying at Least One Night in Channel-Port aux Basques Region and Impact of Diversion to Fixed Link Route

| Diversion Scenario | Vacationers Only | All Passengers |
|--------------------------------|-------------------------|-----------------------|
| Economy Before Diversion | \$3,100,654 | \$4,819,108 |
| Economic Value After Diversion | \$1,227,982 | \$1,908,558 |
| Amount Diverted | \$1,872,673 | \$2,910,550 |

The estimated spinoff generated by passengers that stay at least one night ranges from a low of \$3.101 million to a high \$4.819 million. However, if 60% of these passengers were diverted, the region would lose an estimated \$1.873 to \$2.911 million in spinoffs. Note that the final demand/consumer spending in the region is estimated at \$163.3 million and the impact would be a decline of 0.75% to 1.17%.

⁸ “Vacationers Only” is defined as the portion of the total number of passengers who identified themselves as vacationing as reason for visiting and staying “at least one night” in the region while the “all passengers” includes both vacationing passengers and others.

Outcomes presented in Table 13 represent the conversion, using local multipliers, of the direct and indirect spinoff effects in dollars to employment. The estimated number of jobs affected by the diversion is presented as low, medium and high. These intervals are used because of the non-availability of detailed local employment information by industry. When “Vacationers Only” are considered, 26 to 54 jobs could be diverted to other regions while the “All Passengers” outcomes indicate that 40 to 83 jobs could be redistributed to other regions.

Table 14 Estimated Employment Impact Scenarios of Diverting Passenger Cars to Fixed Link Route

| Employment Scenarios | Low Estimate | Medium Estimate | High Estimate |
|---|---------------------|------------------------|----------------------|
| Vacationers Only (Before Diverting) | 43 | 66 | 89 |
| Vacationers Only (After Diversion) | 17 | 26 | 35 |
| Employment Diverted to Other Regions | 26 | 40 | 54 |
| <hr/> | | | |
| All Passengers (Before Diverting) | 66 | 102 | 138 |
| All Passengers (After Diversion) | 26 | 38 | 55 |
| Employment Diverted to Other Regions | 40 | 62 | 83 |

Channel-Port aux Basques Regional Industrial Impacts

The preceding outcomes provide an overview of potential impacts associated with direct and indirect spinoff in dollars and employment redistribution to other regions in the province if 60% of the passenger traffic was diverted from Channel-Port aux Basques to the Fixed Link route. The following section identifies what specific industries are likely to be impacted with the redirection of passenger traffic out of the region to the Fixed Link route. Table 14 presents the direct and indirect expenditures for all passengers that stay at least one night in the Channel-Port aux Basques region by industry. The “Before Redistribution” is the regional economy before redistribution while the “Redistribution to Other Regions” are the expenditures that are diverted northwards. Finally, the “% Total Before \$ Diverted” is a percentage of an industry’s share of the total “Before Redistribution” regional economy that is diverted to other regions or the industry’s percentage removed from the regional economy through redistribution.

The only previous references for the Fixed Link diversion impacts are previous studies where business is diverted to other locations because of a bypass road. These studies indicated that retail sales, accommodations, food and beverage services, as well as gas stations were the businesses most likely to experience a decline in revenues because of diversion [9]. For this impact analysis, all of these sectors in the regional economy are ranked in the top 10 businesses that would be negatively impacted by the diversion of passenger car traffic from Channel-Port aux Basques to the Labrador Straits and the Northern Peninsula (Table 14). In this case, food and beverage services experience the highest impact with \$0.711 million of \$1.177 million, or 14.75% of the expenditures by all passengers staying at least one night, are removed from the regional economy. This results in an estimated loss of 11 to 23 jobs in this sector (Tables 14 and 15).

Of note, accommodations are the second highest industry impacted but when combined with RV parks, camps and B&Bs, it would be the highest with \$0.775 million of \$1.284 million being diverted or a 16.10% reduction in overall passenger expenditures. The potential employment decline in these sectors combined is between 11 and 28 jobs (Tables 14 and 15). Gas stations are third on the list with \$0.244 million of \$0.404 being redistributed or 5.06% of the expenditures. This equates to a decline in employment of 3 to 6 jobs.

For industries with more than 1.0% in diverted passenger expenditures, \$2.622 million of \$4.341 million is redistributed to regions in Newfoundland and Labrador along the Fixed Link route and this represents 54.4% of the total expenditures by passengers who stay in the region (Tables 14 and 15). It is evident that for this type of analysis examining impacts associated with specific industries provides an insight on those sectors dependent on ferry traffic within the Channel-Port aux Basques region and their sensitivity to changes in passenger traffic flows, especially flows related to stays. From a total regional economy perspective, the “All Passengers” expenditures impact on the estimated total regional GDP of \$300 million is less than -1.0%. However, at the industry level breakdown, the passenger expenditures on industries such as retail that are traditionally impacted by a “type of bypass” or diversion, the reduction can be -13.0% or more in the region; this represents a significant decrease for small and medium size enterprises.

Table 15 “At Least One Night Stay All Passenger” Estimated Redistributed Expenditures by Industry

| Industry | All Passenger Expenditure \$ | | |
|--|------------------------------|--------------------------------|-----------------------------|
| | Before Redistribtuion | Redistributed to Other Regions | % of Total Before Diversion |
| Food services and drinking places | \$1,177,175 | \$710,967 | 14.75% |
| Traveller accommodation | \$1,040,991 | \$628,717 | 13.05% |
| Gasoline stations | \$403,496 | \$243,696 | 5.06% |
| Clothing and clothing accessories stores | \$386,163 | \$233,227 | 4.84% |
| Lessors of real estate | \$267,614 | \$161,628 | 3.35% |
| Food and beverage stores | \$256,178 | \$154,721 | 3.21% |
| RV (recreational vehicle) parks, recreational camps, and rooming and boarding houses | \$243,174 | \$146,867 | 3.05% |
| Water transportation | \$170,541 | \$103,000 | 2.14% |
| Other transit and ground passenger transportation and scenic and sightseeing transportation | \$136,269 | \$82,301 | 1.71% |
| Amusement and recreation industries | \$97,259 | \$58,741 | 1.22% |
| Health and personal care stores | \$81,737 | \$49,366 | 1.02% |
| Performing arts, spectator sports and related industries, and heritage institutions | \$80,644 | \$48,706 | 1.01% |
| Gambling industries | \$67,538 | \$40,790 | 0.85% |
| Travel arrangement and reservation services | \$63,034 | \$38,070 | 0.79% |
| Miscellaneous store retailers | \$62,444 | \$37,713 | 0.78% |
| Community colleges and C.E.G.E.P.s | \$58,342 | \$35,237 | 0.73% |
| Sporting goods, hobby, book and music stores | \$47,595 | \$28,746 | 0.60% |
| Automotive repair and maintenance | \$32,914 | \$19,879 | 0.41% |
| Taxi and limousine service | \$31,056 | \$18,757 | 0.39% |
| Hospitals plus nursing and residential care facilities | \$30,066 | \$18,159 | 0.38% |
| Personal care services and other personal services | \$29,431 | \$17,775 | 0.37% |
| Motor vehicle and parts dealers | \$19,348 | \$11,685 | 0.24% |
| Electric power generation, transmission and distribution | \$11,623 | \$7,020 | 0.15% |
| Grant-making, civic, and professional and similar organizations, plus other non-profit institutions serving households | \$6,718 | \$4,057 | 0.08% |
| Offices of dentists | \$6,068 | \$3,665 | 0.08% |
| Miscellaneous ambulatory health care services | \$5,700 | \$3,443 | 0.07% |
| Telecommunications | \$4,209 | \$2,542 | 0.05% |
| General merchandise stores | \$936 | \$565 | 0.01% |

Table 16 Channel-Port aux Basques Diverted Employment Estimates for Fixed Link Route

| Industry | Estimate for Diverted Employment | | |
|---|----------------------------------|--------|------|
| | Low | Medium | High |
| Food services and drinking places | 11 | 17 | 23 |
| Traveller accommodation | 9 | 15 | 22 |
| Clothing and clothing accessories stores | 3 | 5 | 7 |
| RV (recreational vehicle) parks, recreational camps, and rooming and boarding houses | 2 | 4 | 6 |
| Gasoline stations | 3 | 4 | 6 |
| Amusement and recreation industries | 3 | 4 | 5 |
| Food and beverage stores | 2 | 3 | 4 |
| Other transit and ground passenger transportation and scenic and sightseeing transportation | 1 | 2 | 2 |
| Lessors of real estate | 1 | 2 | 2 |
| Miscellaneous store retailers | 1 | 1 | 2 |
| Health and personal care stores | 0 | 1 | 1 |
| Sporting goods, hobby, book and music stores | 1 | 1 | 1 |
| Water transportation | 0 | 0 | 1 |

Note: Because of rounding employment numbers will not add up to totals presented in Table 12

The preceding discussion has focused on the amount of passenger expenditures diverted away from the Channel-Port aux Basques region and has not addressed where these impacts would be redistributed geographically. Table 16 presents the estimates produced by the GDP gravity model for redistribution of expenditures by Economic Zone.

Under the assumptions of the gravity model, the Labrador Straits Zone receives \$0.641 to \$0.997 million or 34.25% of the diverted revenue while the Gros Morne Zone is allocated \$0.769 to \$1.195 million or 41.05% of the revenue. The Viking Trail and St. Anthony Zone share ranges from \$0.257 to \$0.399 million of 13.70% (Table 15). Proximity to the tunnel entrance is a comparative advantage for the Labrador Straits and the Viking Trail/St. Anthony Zones, while the Gros Morne region gains an advantage because it is still somewhat proximate to the exit point for the tunnel in the St. Barbe area and the GDP for the Gros Morne area reflects its preferred destination status for vacationers on the Northern Peninsula. To determine impacts by industry or the capacity of the industries to meet the demand and include potential growth factors for each of the regions requires further analysis and is not within the scope of this study.

Table 16 GDP Gravity Model Results for Redistribution of Diverted Passenger Expenditures Via the Fixed Link Route

| Economic Zones | GDP Gravity Model % Diversion Factor | Vacationer Only - Revenue Diverted | All Passenger - Revenue Diverted |
|---|---|---|---|
| Zone 5: Labrador Straits (L'Anse Au Clair to Red Bay) | 34.25% | \$641,428 | \$996,922 |
| Zone 6: Viking Trail, St. Anthony South West to Plum Point, East to Roddington/Englee | 13.70% | \$256,618 | \$398,841 |
| Zone 7: Gros Morne Area, Viking Trail North to and including Plum Point | 41.05% | \$768,736 | \$1,194,787 |
| Zone 8: Deer Lake/Humber Area Corner Brook/Massey Drive | 9.88% | \$185,047 | \$287,605 |
| Zone 9: Stephenville/Port-au-Port/Burgeo | 1.11% | \$20,844 | \$32,396 |
| Totals | 100% | \$1,872,673 | \$2,910,550 |

Summary

Overall, there is an expected diversion of between \$1.9 million and \$2.9 million in total (direct plus indirect) economic activity from the Channel-Port aux Basques region north to the Fixed Link regions due to the diversion of non-resident tourist expenditures. This represents a median estimate of between 40 and 62 jobs being redistributed. This spending affects some industries more than others, so expenditure and employment in industries such as restaurants, accommodations and gas stations would be affected the most.

Economic Zones 5 and 7 could reasonably be expected to benefit the most from any transfers of tourism expenditure north, due to their existing high concentration of the industries demanded and proximity to the potential Fixed Link, respectively.

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Leslie Harris Centre of Regional Policy and Development

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**Proposal to
The Government of Newfoundland and Labrador
and
The Atlantic Canada Opportunities Agency**

**To Undertake An Update of the 2004 Study on the
Pre-Feasibility Study for a Fixed Link Between
Labrador and Newfoundland**

**Submitted by the Harris Centre
Memorial University of Newfoundland**

March 8, 2017

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Proposal

Memorial University of Newfoundland's Leslie Harris Centre of Regional Policy and Development (the Harris Centre) submits this proposal to the Government of Newfoundland and Labrador to undertake a study of a possible fixed link between Labrador and the Island of Newfoundland.

This study is intended as a follow-up to a study already undertaken by the Harris Centre back in 2004, which determined that the best option for a fixed link was a tunnel dug 10 meters under the sea bottom using a tunnel boring machine. Vehicles would be transported through the tunnel using an electric train. The total development cost, including interest during construction and escalation, was estimated at around \$1.7 billion. The project was estimated to take 5 years to plan and 5.7 years to construct, for a total project implementation time of 10.7 years.

The purpose of this new study is to determine to what extent new geological research, innovations in tunneling technology, labour costs, inflation or other factors may have an impact (positive or negative) on the original cost and time estimates. Secondly, the study will aim to measure some possible impacts on the economy of the province should the fixed link be constructed and traffic subsequently diverted from the Gulf ferry service.

The engineering partner in the original study was the international consulting firm Hatch Mott MacDonald. This firm has now split into two separate firms, Hatch and Mott MacDonald, with the former having the expertise in tunneling technology. In order not to begin the research from scratch and to build on the existing expertise, it is proposed that Hatch be retained as the engineering consultant for the project.

The regional development portion of the study will be undertaken by RAnLab, a program of the Harris Centre in collaboration with the Department of Geography at Memorial University.

The Harris Centre will act as the proponent for this study, with Hatch and RAnLab as subcontractors. The Harris Centre will be responsible for project management, monitoring progress, contract management, payments to subcontractors, accountability to the funder, etc.

Proponent

The Leslie Harris Centre of Regional Policy and Development is a unit of Memorial University of Newfoundland. It was named in honour of scholar and former Memorial University president, Dr. Leslie Harris. Dr. Harris was known for his integrity and independence while making a practical contribution to Newfoundland and Labrador. Established in 2004, the Harris Centre continues this commitment by coordinating and facilitating Memorial's educational, research and outreach activities in the areas of regional policy and development.

The final product offered to the community, whether in the form of research, teaching or outreach, is based upon the independence and integrity of Memorial's faculty, staff and students in applying their professional expertise in contributing to regional policy and development in Newfoundland and Labrador.

Scope of Work

Engineering and Geotechnical

This component of the work is described in detail in the appendix.

Regional Development

This component of the work will determine to what extent the proposed fixed link would impact:

- The province's export sectors (tourism, fishery, forestry, manufacturing, etc.) related to such factors as: on-time delivery, quality of product, cost, etc.
- The province's imports (perishables, other foods, retail, vehicles, etc.) related to such factors as: on-time delivery, quality of products, cost, etc.
- Different regions of the province, in particular: Southwestern Newfoundland, Bay St. George and Corner Brook; and Labrador West, Central Labrador and the Northern Peninsula.

The analysis will identify the key industry stakeholders, their associated geo-spatial supply chain networks and how costs will vary by mode of transport. It will look at the dependency of various industries on the transportation sector and how disruptions could potentially impact transportation supply chains. The analysis will permit the construction of transportation and industry supply chains that identify not only the geographical linkages and industry dependencies but also the value of those links in terms of dollars and value to the province's sub-regional economies.

The final products for this analysis will be a written report that includes:

- Estimates of potential changes to commodity prices and production costs by domestic industries due to changes in transportation costs.
- Estimated changes in travel times for interprovincial commodity flows by mode of transportation, and the impact of these changes on the economy.

- The impact on the costs of imports and exports associated with a change in the mode of transportation and how these changes impact the provincial economy overall.
- An estimate of the economic impact (in dollars and jobs) on the Southwestern Newfoundland, Bay St. George and Corner Brook regions, along with an estimate of the impacts of increased traffic on the relevant regions in Labrador and the Northern Peninsula.

Project Staff

The project will be managed by **Mike Clair, Associate Director (Public Policy)** at the Harris Centre. Mike is an experienced project manager with 23 years' experience with the Government of Newfoundland and Labrador, and another 12 years in academia. While in government, he led the process that resulted in the creation of The Rooms, the largest investment in heritage in the province in a hundred years. He has experience in tourism, culture, regional development, public policy, governance and post-secondary education administration.

He will be assisted at the Harris Centre by **Kim Crosbie, Operations Manager**. Kim is a recent hire, arriving at the Harris Centre after over 20 years of business consulting experience. She has extensive experience in private/public collaborations and structuring multi-partner agreements.

Alvin Simms, PhD, will lead the component of the project dealing with regional development. He is an Associate Professor in the Department of Geography at Memorial University and an Adjunct Professor in the Department of Agricultural and Rural Economics at the University of Kentucky. He has acted as consultant for municipal, provincial and federal governments as well as industry. His research focuses on the integration of regional economic and spatial analysis methods to develop dynamic models to aid in the assessment of the sustainability of places for the purpose of informing evidence-based policy and strategies for regional development. Current and past research projects include studies on assessing the value and impact of intra- and inter-regional industry-to-industry and occupation linkages on economies, analyzing the impact of transportation supply chains, as well as the importance of small and medium-sized enterprises in rural economies, resilient communities, rural-urban interaction, functional economic regions, sustainability and capacity issues in rural communities, as well as forecasting population shifts and demands for public and private services.

Jamie Ward, MSc will assist Dr. Simms with the regional development research. Mr. Ward was a member of the team that developed the Regional Economic Capacity Index (RECI). He has spent the past several years working closely with a wide group of collaborators and community partners to help make the often-complex world of statistical comparison more accessible to regional planners and developers. Jamie's research has involved using regional science methods to model

and gain a better understanding of some of the determinants of economic growth in Newfoundland and Labrador labour markets. Specifically, this research attempts to address questions relating to the potential applicability of neoclassical growth models to provincial labour markets, and the magnitude of potential negative effects of outmigration on rural labour markets.

The personnel from Hatch assigned to work on this project are listed in the appendix.

Budget

| | |
|--------------------------------------|------------------|
| Engineering and geotechnical (Hatch) | \$150,000 |
| HST (15%) | 22,500 |
| Student research (RAnLab) | 25,000 |
| Project management (Harris Centre) | 25,000 |
| TOTAL | \$222,500 |

Timeline

The engineering and geotechnical component of this project is estimated to take 4 months to complete. At that point, a draft final report will be presented to Government for review. The schedule allocates one month for Government review and comments.

The regional development component of the project is estimated to take 3 to 4 months to complete, depending upon the availability and the quality of the data.

Steering Committee

The project will be overseen by a steering committee composed of representatives from the Government of Newfoundland and Labrador, Hatch, and Memorial University of Newfoundland.

It is anticipated that this committee will meet at least three times:

1. At the beginning of the project to confirm the scope of work, timelines and budget.
2. Mid-way through the work to provide an update on progress.
3. Near the end of the project, when the final draft report is ready for review.

Meetings of the committee will be convened and chaired by the Harris Centre.

Appendix: Engineering and Geotechnical

March 6, 2017

Michael Clair
Leslie Harris Centre of Regional Policy and Development
Memorial University of Newfoundland
St. John's, NL
A1C 5S7

Dear Mr. Clair:

Subject: Proposal for Updated Pre-Feasibility Study for Newfoundland and Labrador Fixed Link

Hatch is please to submit the attached Offer for Engineering and Consultancy Services which outlines the scope, the approach that will be used to complete the study, the deliverables and our commercial offer.

Hatch will perform the work outlined in this Offer for Engineering and Consultancy Services to update the previous study that was carried out in 2004. This letter, the Statement of Work and Hatch Standard Terms and Conditions form the whole agreement between Leslie Harris Centre of Regional Policy and Development and Hatch.

The overall cost is \$150,000 plus HST on a lump sum cost basis.

If this offer is acceptable to Leslie Harris Centre of Regional Policy and Development, please sign the attached Acceptance and we can mobilize the team to start to undertake this work in time manner. If you would like to meet with me to clarify and further discuss any aspect of this offer, please call me at 709 701 0075.

Regards,

Mark Cumby, P. Eng.

MC:sr

cc: Steve Routledge, P. Eng.
Jean Habimana, P. Eng.

Michael Clair
Leslie Harris Centre of Regional
Policy and Development
March 6, 2017

OFFER FOR ENGINEERING AND CONSULTANCY SERVICES for Proposal for Updated Pre-Feasibility Study for Newfoundland and Labrador Fixed Link

March 6, 2017

| | |
|------------------------------|--|
| Client Name: | Leslie Harris Centre of Regional Policy and Development |
| Project Name: | Proposal for Updated Pre-Feasibility Study for Newfoundland and Labrador Fixed Link |
| Client Contact: | Michael Clair |
| Hatch Contact: | Mark Cumby, P. Eng. mark.cumby@hatch.com Phone: 709 701 0075 |
| Proposal Number: | 17-1033, Rev. A |
| Estimated Start Date: | Apr 1, 2017 |
| Cost Basis: | Lump Sum Cost Basis |
| Project Estimate: | \$150,000 + HST |

Introduction

In 2004, Hatch Mott MacDonald (now Hatch) performed a pre-feasibility study of fixed link concepts between the island of Newfoundland and mainland Labrador for the Government of Newfoundland and Labrador. The Government is now looking to update that pre-feasibility study to account for new available investigation and survey data of the tunnelling options as well as validate or update the conclusions reached during the pre-feasibility phase.

With experts in the tunnelling industry worldwide, some of whom were a part of the pre-feasibility study, Hatch is well positioned to provide a clear and concise summary report that

Michael Clair
Leslie Harris Centre of Regional
Policy and Development
March 6, 2017

brings the pre-feasibility of a fixed link to current state of the practice as well as socio-economic realities.

Scope of Work

The goal of this study is to update the previous study to account for new information collected as well as current realities by utilizing experience gained worldwide for similar projects. As part of this, Hatch's scope of work will consist of:

1. Update based on any new background information available relating to the following
 - a. Geotechnical information
 - b. Survey data
 - c. Environmental information
 - d. Regulatory considerations
2. Validation of previous assumptions related to the following and updating as required
 - a. Ridership
 - b. Tunnel utilization
 - c. Design criteria
 - d. Alignment Update
3. Update of experience gained for fixed links worldwide:
 - a. Similar projects
 - b. Gained technological experience
4. Update of alternatives comparison for the fixed link and recommendation of preferred option
5. Update of cost estimate
6. Update of schedule

It should be noted that aspects listed under Item 1 above relate to review of existing information that has become available since the pre-feasibility study was completed and that performing a new geotechnical investigation, topographic survey, or environmental assessment is outside Hatch's scope of work.

Schedule

| <u>Tasks</u> | <u>Duration After Award</u> |
|--------------------------------------|---|
| 1. Update background information | 1 st Month |
| 2. Validate previous assumptions | 2 nd Month – 3 rd Month |
| 3. Update experience for fixed links | 2 nd Month 3 rd Month |
| 4. Update comparison alternatives | 3 rd Month – 4 th Month |
| 5. Update cost estimate | 4 th Month |
| 6. Update schedule | 4 th Month |
| 7. Preliminary Report | End of 4 th Month |
| 8. Client Comment | 5 th Month |
| 9. Final Report | End of 6 th Month |

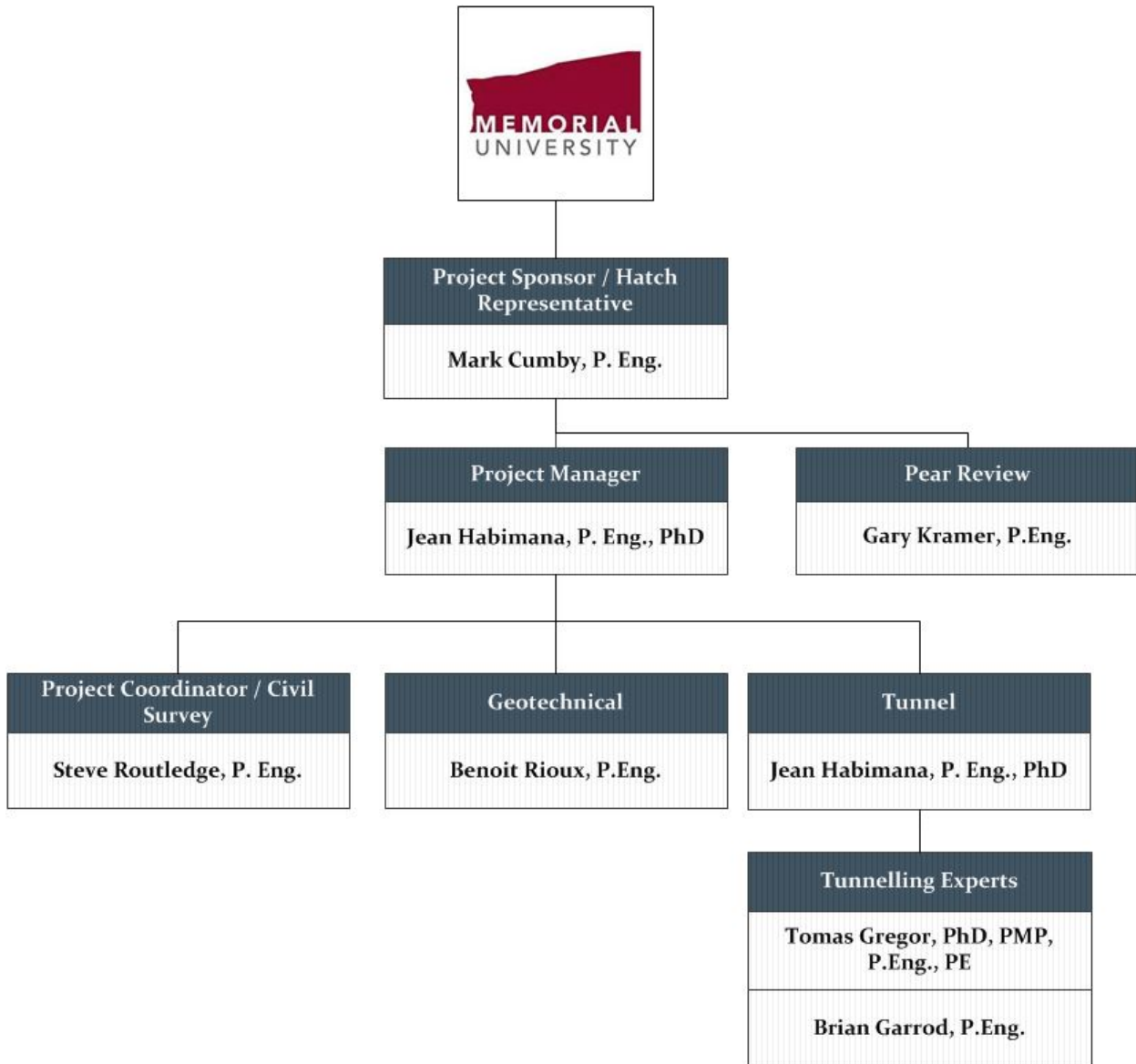
Execution Plan

Upon award, Hatch will commence with research on new tunnelling technologies that have been introduced since 2004 as well as any experience gained through similar projects worldwide. In conjunction with this, we will determine what factors have changed since 2004 that could impact the feasibility of the proposed tunnelling options. Hatch understands the client will make available supplemental geological data upon award of this contract.

The final deliverable will be a summary report that brings the previous report into today's realities and standards to determine the viability of a fixed link between Newfoundland and Labrador.

The anticipated timeline would see a draft of the summary report submitted for review 4 months after award and a final summary report would be submitted 1 month after a client review period. Our schedule allocates one month for client comments.

Hatch Project Team



MARK CUMBY, P. Eng. - Project Sponsor/Hatch Representative

Project Director, Hatch - Accomplished in project management, engineering management, and general business management, Mr. Cumby offers over 22 years of progressive and comprehensive experience in a variety of oil & gas projects across the globe. Through work with Tier 1 producers and EPC firms, he has demonstrated a "customer first" focus, with a keen ability to get deliverables driven to closure, and effective problem solving skills recognized by

Michael Clair
Leslie Harris Centre of Regional
Policy and Development
March 6, 2017

colleagues and clients. In addition, he has extensive experience in pressure equipment and modular skid design and fabrication with emphasis on: produced water treating equipment (hydrocyclones, traditional separators, water polishing); hydrocarbon filtration; gas /oil separation.

JEAN HABIMANA, P. Eng., PhD - Project Manager

Director of Tunnel Practice Lead, East North American, Hatch - Mr. Habimana is experienced in a variety of aspects of geotechnical and tunnel engineering for both soft ground and hard rock tunneling. He has over 20 years of experience and has worked on design and construction of major projects including tunnels, caverns, shafts, open-cut, cut-and-cover excavations, highways, and bridge foundation. His experience ranges from conceptual design to preparation of engineering final design documents including Geotechnical Baseline Reports, drawings and specifications and providing engineering services during construction. Mr. Habimana has an extensive experience in static and seismic soil-structure interaction analysis and seismic design of tunnels and underground facilities. He notably managed tunnel design and construction projects, and he is skilled in the coordination of different disciplines, the management of client inspections, and completing projects on time and on budget. He experience include projects in Europe, China, Australia, United States and Canada. Most notably he has worked on the Swiss Lotschberg Tunnel Project, the SR99 Alaskan Way Replacement Project in Seattle, Washington, the San Francisco Bay Area Caldecott Tunnel, The Toronto Eglinton Crosstown Light Rail Project, the Montreal Blue Line Extension Project, and is currently working on the Montreal Light Rail Project.

BRIAN GARROD, P. Eng. – Tunneling Expert:

Director Tunnels, Australasia, Hatch - Mr. Garrod is a Director - Tunnelling with Hatch and a world-recognized expert on tunnels and tunnel technology. With multiple publications, he is a sought-after speaker at technical conferences. He has 46 years of general civil engineering experience, the last 36 years devoted solely to tunnelling assignments.

Brian's technical expertise covers a broad range of tunnel techniques for high and low-strength rocks and soft ground using excavation methods such as earth pressure balance tunnel boring machines (EPB TBM) and the New Austrian Tunnel Method (NATM). Moreover, he specializes in the design of pressurized-face TBM-driven tunnels, one-pass segmental tunnel linings, risk analysis, productivity analyses and cost estimates for underground projects. Brian has also developed Hatch's in-house tunnels estimating system that enables his team to produce highly accurate project cost estimates.

With outstanding tunnelling qualifications, Brian has been engaged on significant projects around the world such as: the Channel Tunnel; the Boston Outfall; Dulles Airport tunnels in Washington DC; Beacon Hill tunnels in Seattle; BART to San Jose tunnels; Tel Aviv metro tunnels; Los Angeles Metro Red Line, Sewer Tunnel in Columbus OH, Seattle Combined Sewer Outflow (CSO) and sewer tunnels; river and strait crossings in Sarnia, New Jersey, Sacramento, Detroit and Newfoundland and road tunnels in Brazil. At home in Canada, Brian's expert

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knowledge has been applied on major projects to expand transit facilities, e.g. Toronto's new Eglinton Crosstown LRT and the Spadina Subway Extension and Sheppard Subway (among many others for the Toronto Transit Commission), in addition to Vancouver's Skytrain and the new Evergreen Line, Edmonton LRT and the Green Line LRT in Calgary. He also has significant expertise with water conveyance tunnels through various roles on such projects as the new 10 km diversion tunnel at Niagara Falls to serve the Sir Adam Beck hydro power station, the new 15 km Southeast Collector sewer tunnel in York and Durham Regions and for others in Vancouver and Toronto.

He has significant construction management experience, which includes the Detroit River Tunnel, oversight for Seymour-Capilano twin tunnels, the 19th Avenue Tunnel in York Region, the Toronto Coxwell Bypass Tunnel, and engineering support during construction of the Sheppard Subway, Spadina Subway Extension, Eglinton CrossTown LRT, Vancouver ALRT, Dulles Airport tunnels, Sacramento River crossings and the Beacon Hill tunnels.

Mr. Garrod was involved in the preparation of the 2004 pre-feasibility study.

TOMAS GREGOR, PhD, PMP, P. Eng. – Tunneling Expert

Sub Division Manager, Underground Infrastructure, Hatch – Mr. Gregor is a Civil and Structural Engineer as well as an internationally known tunnelling specialist. He has extensive experience in the planning, design and project management of transportation and heavy civil engineering projects. Over the past years, Mr. Gregor has used his technical knowledge as the basis for his work on the management of tunnel design for the Scarborough Subway Extension, participation in the program management of the Toronto-York Spadina Subway Extension, and he was the Hatch Project Manager on the design of the Eglinton Scarborough Crosstown Twin Tunnels and the Pre-AFP Reference Design (six stations and at-grade section) for the LRT line. On the analytical side, Tomas was the Lead Structural Engineer for the Sheppard Subway Twin Tunnels in Toronto and the Beacon Hill Station and Tunnels project in Seattle, WA. Mr. Gregor has been responsible for the design of over 25 precast tunnel linings for various tunnels in Canada, the US and Russia. He also performed detail design and design management for a number of significant bridge projects earlier in his career. Mr. Gregor's broad range of project work has given him an understanding of all aspects of transportation projects, including rehabilitation, design management, analysis and design of concrete (reinforced, pre-stressed and fiber reinforced), steel structures, foundation design, tunnel design, cost estimates, client and contractor communications, and knowledge of numerical analysis and design computer software.

Mr. Gregor was involved in the preparation of the 2004 pre-feasibility study.

GARY KRAMER, P. Eng. – Peer Review

Global Tunnel Practice Lead, Hatch - Mr. Kramer has over 30 years of experience (27 years in consulting engineering with Hatch) in project, design and construction management on multi-disciplinary large tunnel projects for transit, highway, sewage, rail, earthwork, marine (intake

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and outfall) and hydroelectric works. He is Hatch's Practice Lead for Tunnels responsible for technical leadership and quality excellence on tunnel, shaft and trenchless projects.

Throughout his tenure at Hatch, he has been directly responsible for coordination of civil design disciplines - structural, geotechnical and end use functionality (hydraulic, transit, storage and energy) and construction management associated with tunnelling, shaft and marine related work. The last 24 years has been virtually exclusively in tunnel related projects during which he has held major roles (PM, design manager, lead tunnel and/or resident engineer) on over 130 km of constructed tunnels in soft ground and rock with 71 km of those 5 metres or more in diameter.

He received the Bickel Award for tunnelling from the American Society of Engineers in 1998 and was recently invited by the British Tunnelling Society to give a presentation on tunnel lining systems in seismic areas. As a result, he was invited by the International Tunnelling Association to act on their Seismic Committee. He is actively involved with the Tunnelling Association of Canada, sitting on the executive of the Ontario Chapter.

STEVE ROUTLEDGE, P. Eng. – Project Coordinator / Civil Survey

Structural Engineer, Hatch - Mr. Routledge has over 10 years of experience in the engineering field. His primary experience lies in the design and construction of steel, concrete, and wood framed buildings and other structures as well as coordinating lift studies for moving heavy equipment. He is knowledgeable in many aspects of this process, including inspection and surveying, static analysis and design, and finite element analysis. He also has project management and project coordination experience on a number of projects ranging from building rehabilitation and structural steel frames to large span bridge proposals. He is fluent in both English and French.

BENOIT RIOUX, P. Eng. – Geotechnical

Senior Project Engineer – Tunnels, Hatch - Mr. Rioux has a strong experience as a Geological Engineer specialized in rock mechanics and geotechnics. From 2004 to 2009, Mr. Rioux has worked as a Geotechnical Project Manager and as an Exploration Geologist as part of five geotechnical investigation campaigns on the Rupert-Eastmain-1A Hydroelectric Projects. Mr. Rioux also worked as a Site Geological Engineer for the construction of the Keeyask Hydroelectric Project from 2015 to 2016 and Eastmain-1A Hydroelectric Project from 2009 to 2010. From 2010 to 2015, he worked as a Geotechnical and a Rock Mechanics Project Manager. He managed the geotechnical exploration campaigns for a major underground retention basin and a subway extension project in Montreal. Mr. Rioux was also V.P. of the Canadian Geotechnical Society for the Western Quebec Chapter from 2006 to 2009 and he was the Technical Short Courses Manager for GeoMontreal 2013.

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Payment Schedule

The overall cost is \$150,000 plus HST to be paid on a lump sum cost basis.

| | |
|---|----------|
| 1 st Month | \$30,000 |
| 2 nd Month | \$30,000 |
| 3 rd Month | \$30,000 |
| 4 th Month – Preliminary Report Submission | \$30,000 |
| 6 th Month – Final Report Submission | \$30,000 |

Hatch will perform the work outlined in this Offer for Engineering and Consultancy Services in accordance with the attached Professional Services Terms and Conditions. This letter, the Statement of Work, and Hatch Standard Terms and Conditions form the whole agreement between Leslie Harris Centre of Regional Policy and Development and Hatch.

This offer remains valid for a period of 30 days from the date of this letter.

Acceptance of Offer

Leslie Harris Centre of Regional Policy and Development accepts this proposal and requests Hatch to undertake the assignment as detailed above.

Signed on behalf of Hatch by:

Signed on behalf of Leslie Harris Centre of
Regional Policy and Development by:

Name: Nancy Fréchette

Name: _____

Title: VP Infrastructure, Hatch

Title: _____

Date: _____

Date: _____

Michael Clair
Leslie Harris Centre of Regional
Policy and Development
March 6, 2017

Attachment A – Terms and Conditions

CLAUSE 1 AGREEMENT

1.1 Unless a written agreement is entered into, Client's acceptance of a proposal, whether written or oral (the "Proposal"), from Hatch Corporation ("or Consultant") or a request by Client for some or all of the services included in the Proposal, constitutes a binding contract between Client and Consultant (the "Agreement"). The Agreement incorporates and is subject to these Terms and Conditions and the terms and conditions included in the Proposal, including the description of the services to be provided by Consultant (the "Services"). If there is any conflict between the Proposal and these Terms and Conditions, the Terms and Conditions will govern. Any terms appearing on any orders or other documents produced by or on behalf of Client are excluded.

CLAUSE 2 CONSULTANT SERVICES AND RESPONSIBILITIES

2.1 Consultant will (a) perform the Services with due care, skill and diligence in accordance with the standard of care normally exercised by professionals providing similar services under similar circumstances, and (b) re-perform any Services that fail to comply with this standard of care if Client gives Consultant notice of such failure within 12 months of performance of such Services. Consultant may subcontract the performance of any of the Services to an affiliate.

2.2 Consultant is not liable or responsible for (a) the work or products of any third party contractors or suppliers engaged by or on behalf of Client, including any means, methods, sequences, control, procedures or techniques used by construction contractors, (b) any goods, equipment or materials procured on behalf of Client, provided that Consultant will use reasonable efforts to obtain appropriate warranties from the suppliers of such goods, equipment or materials for Client's benefit, or (c) the safety and security at any Client premises or the project site, provided that Consultant will comply with all relevant laws and those site requirements relating to safety and security that have been notified to Consultant.

CLAUSE 3 CLIENT RESPONSIBILITIES

3.1 Client will (a) make available to Consultant all information, documents and assistance necessary or reasonably requested by Consultant in order to enable it to perform the Services in a timely manner, (b) make decisions, provide approvals and obtain all necessary authorisations, licences and permits required in order to permit the timely performance of the Services, (c) notify Consultant if it becomes aware of any matter that may change the scope, timing, order or complexity of the Services, and (d) act reasonably and in good faith in all respects in connection with the Agreement.

CLAUSE 4 INVOICING, PAYMENT AND TAXES

4.1 Unless otherwise provided in the Proposal, (a) Services (including any additional services provided at the request of Client or pursuant to Clause 4.4) and all costs incurred by Consultant in connection with the Services, will be charged to Client in accordance with Consultant's schedule of rates, (b) amounts invoiced to Client by Consultant are due and payable within the period stated in Consultant's schedule of rates or, if not so stated, within 30 days of receipt of invoice by Client, and (c) interest will be paid on past due amounts at the rate stated in Consultant's schedule of rates or, if not so stated, at the prime rate quoted by Consultant's main bank in the Jurisdiction plus 3%.

4.2 Consultant's rates are exclusive of all taxes, duties, royalties, levies and other governmental or regulatory charges, other than taxes on payroll and Consultant's net income. If any such taxes, duties, royalties, levies or charges are levied on or applicable to amounts payable to Consultant, they will be borne by Client and (a) if Consultant is required to pay any such taxes, duties, royalties, levies or charges, the amount of such payments will be reimbursed to Consultant by Client, and (b) if they are required to be withheld or deducted from amounts payable to Consultant, the amounts payable will be grossed up so that Consultant receives the entire amount that is due pursuant to the terms of the Agreement.

4.3 If Client disputes any portion of an invoice, it will pay those amounts that are not in dispute and notify Consultant in writing of the reasons for the dispute within 10 days of receiving the invoice. Failure to notify Consultant of the dispute within the required time will be deemed as acceptance of the invoice. If it is determined that any amounts in dispute should have been paid at the time it was invoiced, then Client will promptly pay such amount, together with interest at the rate set out in Clause 4.1.

4.4 If, (a) Consultant is required to perform Services in circumstances other than those expressly or reasonably assumed and normally pertaining to services of a similar nature (b) Consultant incurs costs arising or resulting in whole or in part from any site conditions existing on or after the date of the Agreement, (c) there is a change in the scope, timing, order or complexity of the Services or claims filed by Clients subcontractors, (d) there is a force majeure event which impacts Consultant, or (e) additional costs are incurred as a result of a change in any laws, regulations or rules or the interpretation thereof then any resulting costs will be borne by Client and Consultant will be entitled to such amendments to the Agreement that are fair and reasonable.

CLAUSE 5 LIMITATION OF LIABILITY

5.1 To the maximum extent permitted by law and notwithstanding and superseding anything to the contrary in the Agreement:

- (a) subject to Clause 5.2, the aggregate liability of Consultant arising out of the performance or non-performance of the Services or otherwise in connection with the Agreement is limited to the sum of (i) the amount of the professional fees paid to Consultant pursuant to the Agreement up to \$100,000 and (ii) 10% of such fees paid in excess of \$100,000, provided that in no event will Consultant's aggregate liability exceed \$1,000,000;
- (b) in no event will Consultant be liable to Client or its directors, officers, employees, agents, contractors, subcontractors, parent or affiliated corporations, vendors or materialmen for any claim, action, proceeding, loss, damage or cost that (i) in any manner relates to a loss of revenue, profits, opportunity or production, loss or denial of use of any equipment or facility, increased expense of construction, operation or maintenance, economic loss, loss of goodwill or reputation, delay, business interruption or the cost of repair to or replacement of equipment, facilities or goods and related third party services, (ii) in any manner can be construed as indirect, incidental, special, punitive or consequential losses or damages, or (iii) is not a direct result of a material breach by Consultant of the Agreement; and
- (c) Consultant does not guarantee any specific outcomes or results, including fit for purpose, project costs or quantities or the ability of any process, technology, equipment or facility to meet specific performance specifications.

5.2 Consultant's liability for claims or losses covered by the insurance policies referred to in Clauses 7.1(b) and (c) shall not be subject to Clause 5.1 and shall instead be limited to the proceeds of the types and specific amounts of insurance up to the amounts specified in Clause 7.1.

5.3 All statutory, express or implied warranties (including those in any relevant trade practices or sale of goods laws relating to the quality or fitness for purpose of the Services or any goods, equipment or materials supplied by Consultant in connection with the Services) are excluded or limited to the maximum extent permitted by law.

5.4 Any action or claim against Consultant in connection with the Agreement or the performance or non-performance of Services, whether in contract, tort, equity, statute or otherwise, must be made within 12 months of the date of the performance or non-performance of the relevant Services.

5.5 Client indemnifies, defends and holds harmless Consultant for any claims, actions, proceedings, liabilities, losses, damages or costs: (a) it suffers or incurs in connection with the Services and which result other than from a material breach of the Agreement by Consultant or (b) that result from any material breach of the Agreement by Client (c) that result from the site conditions existing prior to or after the date of the Agreement.

CLAUSE 6 USE AND OWNERSHIP OF INFORMATION

6.1 Each party retains title to all intellectual property (including all patents, trademarks, copyright, trade secrets and know how) owned or possessed by it or any of its affiliates and used by it in fulfilling its obligations under the Agreement, including any modifications or improvements made thereto ("Background IP"). All new and original intellectual property created by Consultant during the course of performing the Services ("Project IP") is the property of Consultant. Consultant grants Client a non-exclusive, non-transferable and, unless otherwise agreed, royalty-free license to use (a) any Consultant Background IP used in the performance of the Services but only to the extent required to use any deliverables provided by Consultant for the purpose for which they have been provided (excluding any software source code), and (b) Project IP for any purpose whatsoever.

6.2 Upon receipt of full payment for the related Services and subject to Clause 6.1, all reports, drawings and other deliverables provided to Client by Consultant will become the property of Client.

6.3 Any information or deliverable provided by Consultant to Client in connection with the Services are provided solely for Client's own use and for the specific purpose for which the Services were engaged. In no case will any such information or deliverable be used in connection with any public or private stock, bond or other financial offering, any investment decision, the sale of securities or any other financing transaction or otherwise be made available to the public generally. Consultant makes no warranty or representation and assumes no liability in respect of and Client shall bear all losses or damages resulting from (a) the wrongful or unauthorised use of information or deliverable by Client or third parties, and (b) the accuracy or completeness of information based on data gathered from Client or provided by third parties on behalf of or at the instruction of Client (and Consultant is able to rely on such information without verification in the performance of the Services).

6.4 Each party will keep confidential all Confidential Information disclosed to it by the other party; provided that (a) Consultant is able to disclose Client's Confidential Information to those persons who need to know such information for purposes that relate to the performance of the Services, (b) Client is able to disclose Consultant's Confidential Information to the extent required in connection with the purpose for which the information was disclosed, and (c) either party is able to disclose Confidential Information required to be disclosed by law, provided that the receiving party immediately notified the disclosing party of the requirement to disclose and took all reasonable steps to lawfully resist or narrow the requirement to

disclose the Confidential Information. Except as specifically provided herein, neither party will acquire any right, title or interest in or to the Confidential Information of the other party.

6.5 "Confidential Information" means any information in any form disclosed by or on behalf of one party to the other party at any time before or after the execution of the Agreement in connection with the Services; excluding only information which (a) was at the time of disclosure or thereafter became part of the public domain through no act or omission of the receiving party, (b) became available to the receiving party from a third party who did not acquire such confidential information under an obligation of confidentiality either directly or indirectly to the disclosing party, or (c) was known to the receiving party at the time of disclosure by the disclosing party and such knowledge can be demonstrated by written records that were in existence at the time of disclosure.

CLAUSE 7 INSURANCE

7.1 Consultant will have in effect for the duration of the Services the following insurance (a) workers compensation, in accordance with statutory requirements, (b) comprehensive general (or public) liability (\$5,000,000 per occurrence/aggregate); (c) automobile liability (\$5,000,000 per occurrence/aggregate), and (d) professional indemnity (E&O) liability (\$1,000,000 per claim/aggregate on a claims made basis).

7.2 During the period in which the Services are being performed, Client will, at its own expense, maintain insurance to limits which are normal and customary in the circumstances. Client, on behalf of itself and its insurers, waives all rights of subrogation against Consultant for, and releases Consultant from any liability for damage to Client's property howsoever caused to the extent that Client is compensated for such damage under an insurance policy.

CLAUSE 8 TERMINATION AND SUSPENSION

8.1 Client may suspend the Services or terminate the Agreement for its convenience on 30 days prior written notice to Consultant; provided that, if the aggregate duration of all suspensions under the Agreement exceeds 60 days, Consultant will have the right to terminate the Agreement.

8.2 Either party may terminate the Agreement immediately if anything happens to the other party that reasonably indicates that there is a significant risk that the other party is or will become unable to pay its debts generally as they come due.

8.3 Either party is entitled to terminate the Agreement on 30 days prior written notice to the other party in the event that the other party is in substantial default under the Agreement and such default has not been corrected or reasonably commenced to be corrected within 15 days following notice of such default. Consultant may, by providing 5 days prior notice to Client, suspend Services if Client is in breach of Clauses 3 or 4.

8.4 In the case of any suspension or termination of the Agreement, Client will pay Consultant for all Services provided and costs incurred up to the effective date of suspension or termination. In the event of any suspension or termination pursuant to Clause 8.1 or any suspension or termination by Consultant pursuant to Clauses 8.2 or 8.3, Client will also pay Consultant for any Services provided or costs incurred that are necessary or incidental to suspension or termination, including demobilization costs.

CLAUSE 9 NON-SOLICITATION

9.1 Client will not, during the term of the Agreement or for 12 months thereafter, either directly or indirectly on its own behalf or jointly with or on behalf of any other person, solicit, engage or employ any employee or independent contractor of the Consultant (or any of its affiliates) that has been involved in the provision of Services or with whom the Client has otherwise had contact in connection with the Agreement.

CLAUSE 10 DEFINITIONS AND INTERPRETATION

10.1 Reference to (a) "*affiliate*" means with respect to a party, one or more entities that control, are controlled by, or are under common control with, the party, (b) "*costs*" means any and all costs and expenses, including reasonable legal fees, (c) "*force majeure*" means acts of God, strikes, lockout, other industrial action, war or civil disturbance, terrorism, unusually inclement weather, storm, flood, earthquake, lightning, fire, explosion, nuclear or radioactive contamination, epidemics or pandemics, governmental action or inaction, change in law, extraordinary market conditions affecting the availability of labour, late or inadequate execution of work or supply of goods by third parties and any other event beyond the reasonable control of the affected party, (d) "*Consultant's schedule of rates*" means Consultant's standard hourly rates and reimbursable charges as notified by Consultant from time to time, provided that any changes to the schedule of rates will be communicated to Client before they take effect and will not occur more than once every six months, (e) "*liability*" includes any and all liability whatsoever, whether arising under the law of contract, tort (including negligence), equity, statute or otherwise, whether arising in connection with the performance or non-performance of the Services or otherwise in connection with the Agreement and whether to Client or other persons, and "*liable*" has a corresponding meaning, (f) "*site conditions*" means any conditions in, on, under or around the project site that affect the project or the performance of Services, including any plant and subsurface conditions and any hazardous, radioactive, special, toxic, residual or regulated substances, waste

or materials, (g) "*Jurisdiction*" means the Province of Canada where Consultant's office providing the Services is located or if the Services are being provided in multiple offices, then the laws of Ontario, Canada, and (h) "\$" means the currency of Canada unless specified otherwise.

10.2 Headings are for convenience only and will not be taken into account in interpretation and words importing the singular include the plural and vice versa. If any provision of the Agreement is held to be void, illegal or unenforceable, then: (a) it is severed and the rest of the Agreement remains in force, and (b) the parties will replace the provision with one that is in accordance with applicable law and as close as possible to the parties' original intent. Any rules of contract interpretation that result in the Agreement being construed contrary to the interests of Consultant do not apply in the interpretation of the Agreement.

CLAUSE 11 GENERAL

11.1 The Agreement will be governed by and construed in accordance with the laws of the Jurisdiction, without giving effect to conflict of law considerations. All disputes will be submitted to senior management for discussion. If the parties are unable to resolve a dispute through such discussions, either party may submit the dispute to arbitration. The arbitration will be held in English and at the location of Consultant's contracting office. The arbitration panel will consist of one arbitrator. Any arbitration award will be final and binding on the parties without any right of appeal. Each party will bear the costs of arbitration. No legal proceedings may be commenced by either party in connection with the Agreement or the Services other than in accordance with this Clause 11.1; provided that either party may apply to a court of competent jurisdiction for interlocutory relief during the course of such proceedings or to enforce any order or award obtained in accordance with this Clause 11.1.

11.2 The Agreement represents the entire agreement between the parties regarding the subject matter hereof and supersedes all prior representations, understandings or agreements, whether written or oral and whether express or implied; provided that, if the parties have previously entered into a confidentiality (or similar) agreement regarding the subject matter hereof, such agreement will survive and Clauses 6.4 and 6.5 will be of no force and effect. Amendments to the Agreement are effective only if executed in writing by authorized representatives of both parties.

11.3 Neither party may assign (other than to its affiliate) the Agreement or any interest therein, in whole or part, without the prior consent of the other party. The Agreement will enure to the benefit of and be binding upon the parties and their respective successors and permitted assigns.

11.4 Neither party will be considered to be in breach of its obligations under the Agreement, except obligations to make payment, to the extent that performance is prevented or delayed by force majeure. Each party will use best efforts to overcome any force majeure as soon as possible.

11.5 The limitations and exclusions on liability expressed in this Agreement will apply even in the case of the fault, negligence or strict liability of the party who is the beneficiary of the clause, and will extend to the officers, directors, employees, agents, representatives, subconsultants and affiliates of such party.

11.6 Any notice, consent or other communication given hereunder will only be deemed to have been given if it is in English, in writing and is sent to the recipient's authorized representative at the usual business address of the recipient by (a) registered mail, (b) fax, (c) e-mail (but only when receipt is confirmed in writing by reply e-mail or otherwise) or (d) personal delivery for which a receipt is obtained. Notice given by fax, personal delivery or e-mail will be deemed to have been given on the business day following delivery. Notice given by mail will be deemed to have been given on the fifth business day after mailing.

11.7 No waiver by either party of any breach of the Agreement will be binding unless made in writing and any such waiver will extend only to the specific breach waived and not to any future breach.

11.8 Consultant is an independent contractor in performing the Services. Nothing in the Agreement will create or will be construed so as to create the relationship of principal and agent between Client and Consultant.

11.9 Client and Consultant shall strictly comply with all applicable laws and regulations prohibiting illegal activity of any kind, shall act in an ethical manner with all professional levels of integrity and shall not engage in any acts of corruption, bribery or do anything to improperly influence decision makers.

11.10 Electronic files provided to Client are for reference only and Consultant makes no warranties as to the correctness of information contained in the same after transmittal to Client and nothing therein shall serve to modify Clauses 2 and 5 respectively. In the event of a conflict between electronic files and non-electronic documents, the non-electronic documents shall control and Consultant retains all ownership rights in the electronic files per the terms of this document.

11.11 The provisions of Clauses 1, 2.2, 4, 5, 6, 8, 8.4, 9, 10 and 11 survive the termination of the Agreement.





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